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**WEST QUESNEL LAND STABILITY STUDY
VOLUME 2 OF 2
APPENDICES A TO I**

DRAFT

Submitted To:

City of Quesnel
410 Kinchant Street
Quesnel, BC
V2J 7J5

By

**AMEC Earth & Environmental
A division of AMEC Americas Limited**

KX04397

May 2007



FILE COPY

**WEST QUESNEL
INSAR DEFORMATION ASSESSMENT
QUESNEL, BRITISH COLUMBIA**

Submitted To:

**CITY OF QUESNEL
BRITISH COLUMBIA, CANADA**

Submitted By:

**AMEC EARTH & ENVIRONMENTAL
EDMONTON, ALBERTA**

22 November 2004

File No. KX0439702

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APPENDICES

Appendix A

1.0 INTRODUCTION

AMEC Earth & Environmental (AMEC), a division of AMEC Americas Limited, was retained by the City of Quesnel to undertake an assessment of ongoing slope movements for the area of West Quesnel using Synthetic Aperture Radar Interferometry (InSAR). The main purposes of the assessment were to:

1. Identify and/or confirm the boundaries of the main slide mass that is currently being monitored using other methods, and;
2. Identify zones of differential movement within the slide mass in order to delineate more active zones or zones of differential movement, if any.

The proposed methodology for conducting the InSAR assessment was contained within AMEC's letter to the City of Quesnel dated 5 July 2004. Authorization to proceed was provided by Mr. Jack Marsh of the City of Quesnel in a letter dated 21 July 2004.

This report does not provide a detailed discussion of the background of the study area in relation to the geology, landslide mechanisms and review of historical data trends. Further more detailed information has been outlined previously in other reports by AMEC for the City of Quesnel.

2.0 BACKGROUND

Since 1997, the presence of large-scale landslide features in a suburban area of West Quesnel has been postulated by various geotechnical assessments. These assessments were largely based on airphoto interpretation, surface reconnaissance, and a review of reported utility breaks. Due largely to the absence of detailed subsurface information, reviews of past work had been unsuccessful at documenting the clear existence of a deep-seated landslide. Subsequently, lateral movements of up to 250 mm over an area at least 1.5 km by 1.5 km square were reported at Global Positioning System (GPS) ground movement monitoring hubs installed in West Quesnel by BC Gas (now Terasen).

Between September 2000 and July 2002, AMEC conducted a sub-surface geotechnical investigation in a portion of the affected area of West Quesnel. The findings of the investigation were presented in a report to the City of Quesnel entitled *West Quesnel Land Stability Study, Quesnel, B.C.*, dated 25 October 2002. Based on subsurface information collected from seven slope indicator installations, AMEC concluded that a large, deep-seated, active but slow moving landslide underlies part of the West Quesnel area. Relatively high (in some places artesian) groundwater levels were evident in the slide area. AMEC's report concluded that potentially the most practical and cost effective way of improving the stability of the area (reducing movement rates to manageable levels or possibly stopping further movement) would likely be via extensive surface and sub-surface drainage measures focused on dewatering and/or reduction of groundwater pressures within the slide area.

From July 2003 to March 2004, AMEC conducted a preliminary dewatering well test in an isolated section of the study area in order determine the feasibility of using pumping wells to lower water pressures within the slide mass. The results of the dewatering test were provided in a report to the City of Quesnel entitled *Pilot Dewatering Well Test, West Quesnel Land Stability Study*, Quesnel B.C., dated 28 May 2004. Over the same time period, AMEC continued to monitor slope inclinometers, groundwater levels, and gather GPS movement hub data provided by Terasen. This information was last reported in an AMEC report to the City of Quesnel entitled *On-going Monitoring, West Quesnel Land Stability Study*, dated 20 January 2004.

During AMEC's previous work it became apparent that additional information regarding geological and groundwater conditions was needed over a wider area of the study area than had been previously investigated. In order to optimize location of future drill and instrumentation locations, a number of recommendations for additional study were made, including continued monitoring of ground movements via GPS surveys, installation of additional slope indicator casings, and the possible use of innovative satellite based remote sensing techniques (e.g. InSAR).

3.0 INSAR ASSESSMENT

3.1 THEORY

Synthetic aperture radar (SAR) is an active sensor that can be used to measure the distance between the sensor and a point on the earth's surface. A SAR satellite typically orbits the earth at an altitude of approximately 800 km. The satellite constantly emits electromagnetic radiation to the earth's surface in the form of a sine wave. The electromagnetic wave reflects off the earth's surface and returns back to the satellite. This reflected signal is used to create a SAR satellite image (a black and white representation of the ground reflectivity).

SAR images are made up of pixels, which represent an area of ground surface 20 m across for the satellite data under discussion. Each pixel has a specific size determined by the SAR satellite resolution, the higher the resolution the smaller the pixel size. The ground reflection is averaged over the pixel area. For this current assessment, the ERS data utilized is uses a 20 m x 20 m pixel and therefore each pixel represents average deformations within this area.

To measure differential ground movements over a specified time period, InSAR requires two SAR images of the same area taken from the same flight path from positions within 500 m apart. InSAR compares the phase of the echoed signal to the reference signal on a pixel by pixel basis. Thus, the differences in phase between the two SAR images can be used to determine ground movement in the line of sight of the SAR satellite for each pixel.

The wavelength of the electromagnetic signals emitted by the SAR sensor is typically in the order of a few centimeters. Because InSAR measures the phase difference resulting from the path length change between the sensor and a point on the earth's surface, the magnitude of ground movement between the two satellite passes can be measured to millimeter accuracy.

InSAR has proven highly successful in detecting ground movements in several locations and applications around the world. For example, one of the early users of the technology was the oil sector in the United States, which used spaceborne InSAR to detect very small surface movements (settlements) above deeply buried reservoirs during hydrocarbon production (Van der Kooij, 1997). More recently, examples of successful detection of ground motion for landslides in Canada have been presented by Kosar et al (2003a, b).

Currently there are two agencies operating SAR satellites in the civilian sector, the Canadian Space Agency (CSA) and the European Space Agency (ESA). CSA has had one SAR satellite, RADARSAT-1 in orbit since 1996. ESA recently launched its third SAR satellite, Envisat, in 2002. Its predecessors, ERS-1 and ERS-2 collected SAR images from 1992 to 2000 and from 1995 to 2001, respectively.

The main considerations in choosing and collecting data for the interferometry assessment are as follows:

1. Satellite Path/Trajectory: One of the main considerations for mapping slope deformations is the satellite trajectory. A satellite will pass over the same position on the earth on both an ascending and descending orbit due to the rotation of the earth and the orbit of the satellite. On the ascending orbit, data the satellite sensors are facing approximately to the ENE while the descending orbit will face approximately to the WNW. The terms "ascending" and "descending" refer to the apparent directions of the orbits when viewing the earth from space as a globe. For example, an "ascending" orbit is going from the south toward the north. The sensors always point to the right looking in the direction of the satellite orbit and so point in opposite directions relative to the same point on the ground for ascending and descending passes.

For Central British Columbia the trajectory of the ascending orbit, for both ERS and RADARSAT, is 344° with a look direction (line of sight) of 074°. The descending orbit has a trajectory of 195° with a look direction of 285°. In order to have the best possible chance for success, it is desirable to choose a satellite look angle that is approximately along and parallel to the movement vector of the slope. For the slopes of West Quesnel, in general the slope is east facing and is relatively flat, either an ascending or descending trajectory would be considered to be acceptable for mapping deformation and therefore the choice of trajectory was made based on data availability.

2. Incidence Angle/Slope Geometry: Another important consideration for detecting slope movements is the direction of movement in relation to the angle off vertical of the satellite look direction down the slope. This is termed the Incidence Angle. The best case for detecting movements using InSAR is when movements are completely parallel to the beam path, and the worst case being that where movements are completely perpendicular to the beam path. This is more of an issue for steep mountainous slopes and less so for the relatively flat slopes present in the landslide topography along the Fraser River. For relatively flat slopes, the best case is to have the highest incidence angle possible as this correlates with the flattest angle at which the satellite views the earth's surface. The incidence angle for the data utilized for the West Quesnel project is 23.3°.
3. Vegetation/Snow Cover: For Quesnel, as there may be snow on the ground between the end of October and mid-April, scenes were chosen outside this time frame. Another factor considered is the vegetation cover on the ground. For heavily vegetated sites, the leaves/branches on the trees will scatter the electromagnetic waves emitted from the satellite and not provide a coherent return and are therefore not suitable for deformation measurement. As much of West Quesnel is a developed urban/suburban area and there are large non-vegetated patches, this produces ideal conditions for deformation monitoring and suitable for obtaining deformation results over longer time frames (a year or two vs. the few months that are possible in more vegetated areas).

4. **Range of Motion:** Typically motion ranging between a fraction of a wavelength and a full wavelength can be recorded using InSAR. For C-Band satellites (ERS and Radarsat) a 5.6 cm wavelength is utilized. For this assessment a lower bound cut off of 5 mm was utilized to filter out potential atmospheric effects and poor data with a practical upper bound for deformation mapping of 6 cm. In choosing the most suitable time period for deformation monitoring using InSAR for West Quesnel, the aim was to choose a time period that would capture the largest range of motion based on the historical GPS monitoring data. This is discussed in more detail in Section 5.2.

A more detailed discussion of the application of InSAR to landslide movement detection is provided by Froese et al. (2004).

3.2 METHODOLOGY

In order to attempt to map deformations over West Quesnel using InSAR the following information was evaluated:

- Existing GPS and geotechnical instrumentation ground monitoring data
- Lists of archival ERS and RADARSAT-1 satellite data available since 1992.

The results of the historical monitoring up to the spring of 2003 were compiled by AMEC (2004) and reviewed to assess the optimum time frame for selection of satellite data sets. Differential GPS ground movement monitoring hubs were first installed in West Quesnel in the fall of 1998 by Terasen in order to assess movement potentially impacting on their existing gas pipeline network. A second set of 10 hubs were installed in 2001 by Terasen. In the fall of 2001 AMEC installed a series of seven slope inclinometers. Based on this available data, the overall movement trends suggested a period of larger movements occurred up to the fall of 2002 with rates decreasing significantly since that time. This change in rate of movement has been attributed to significantly lower precipitation levels since the fall of 2002. This is discussed in more detail by AMEC (2004).

Based on these movement trends, the optimum period for InSAR deformation assessment was determined to be between September 1998 and fall 2002. This time frame captured the maximum rate of movements detected by ground based instrumentation. Based on this time frame, a search of the available ERS and RADARSAT ascending and descending data was undertaken. From the search, there were found to be a large number of images available at suitable times over this time period. The following ascending or descending scenes were chosen for generation of preliminary interferograms that would allow a preview of the data quality and to provide an initial indication of deformation patterns:

- July 26, 1997
- October 24, 1998
- July 31, 1999
- November 13, 1999
- September 23, 2000

Based on the preliminary review of the interferograms, there appeared to be good quality data available for the October 24, 1998 to July 31, 1999 time frame that also spanned a period during which the Terasen GPS movement hubs detected movements up to 7 cm. It was expected that this range would allow the best chance for the differentiation of zones of movement within the landslide mass and determination of the boundaries of the landslide.

Once the choice of data was made, more detailed processing was undertaken by Atlantis Scientific Inc. (ASI) and deformation maps were generated. A more detailed discussion of the methodology utilized in the generation of the deformation maps is provided in ASI's report (ASI, 2004), which is appended to this report. Based on the more detailed processing, the deformation data was orthorectified and overlain on an orthophoto base of West Quesnel. Two types of deformation products were provided to AMEC: slant range and flow line deformation maps. Each of these products allowed AMEC to gather different types of deformation data and these two products are discussed in more detail in the following paragraphs:

- **Slant Range Maps :** These maps depict the deformations as they occur either away from or towards the satellite. Positive values represent for away from the satellite (lengthening of wave travel distance) and negative values represent movements towards the satellite (shortening of wave travel distance). On the attached Figure 1, the slant range data has been presented overlain on the orthophoto data. The slant range data provides important information as to the style of movement and an indication of data decorrelation, which may be indicative of either ground movements that exceed a wavelength (~6 cm) or where the ground surface has changed for other reasons over this time period (e.g. agricultural activity, site regrading, heavy vegetation).
- **Horizontal Flow Line Map:** This map (Figure 2) take the slant range data and, for simplicity, resolves it into the direction of the slope surface, assuming that movements are in the direction of the fall line of the slope. These movements are then represented as absolute downslope deformation values. As most movements are not purely in the maximum a downslope gradient direction, this assumption will never be totally correct but provides a reasonable depiction to be used to overall zonation of landslide movements and as a reasonable indication of the magnitude of the deformation over the specified time period. Caution must be taken in the interpretation of the flow line results as:

- o The movement direction may not necessarily be parallel to the slope and therefore the results may be misleading. For deep seated landslides, movements are often near horizontal and the projection of movement along the slope may be slightly greater than actually exists, and
- o If there are subtle changes in the topography along the slope, the projections along the slope may yield pockets of deformation that are not real but rather created due to geometric corrections based on the DEM that is utilized.

4.0 RESULTS

InSAR deformation results were obtained over the West Quesnel study area for the time period between October 24, 1998 and July 31, 1999. The following provides a discussion of the features observed in the slant range and flow line maps on Figures 1 and 2, respectively, in relation to the overall correlation with the existing ground movement instrumentation, the boundaries of the slide mass and zones of differential movement within the slide mass:

Comparison with Existing Instrumentation: The deformation vectors from previous GPS ground movement hubs have been overlain on both the slant range and flow line data on Figures 1 and 2. As GPS readings were taken in September 1998, December 1998, April 1999 and December 1999, the results were graphed to look at overall movement trends over that time. As the overall movement trends appeared to be relatively linear over that time frame, the specific cumulative movement vectors for the GPS movement hubs over the October 24, 1998 to July 31, 1999 time frame were interpolated and have been provided on the flow line map on Figure 2. As can be seen the results of the flow line InSAR and the GPS hubs agree fairly well in terms of the magnitude of deformations observed, especially when comparing movements within the more active zone to those outside to the east. Overall movements on the GPS hubs over this time frame range from 28 to 70 mm, while deformations up to 80 mm are observed in the InSAR assessment. As the InSAR assessment is looking at strictly two different scenes and movements are averaged within a 20 m x 20 m pixel, it would not be expected that the results would be exactly the same but rather show similar trends to those obtained from the GPS hubs. There are also potential inherent errors in the GPS monitoring that could lead to variations (+/- 25mm). In the northern portion of the slide, where movements have a bit of a northerly component, the movement direction is at more of an oblique angle to the satellite look direction. The associated geometric errors have led to noise in the data that has been filtered out of the flow line data and therefore movements in these areas have not been quantified by InSAR. Also, as indicated in the flow line discussion, the deformations shown on the flow line map have been resolved along the slope surface, instead of in a purely horizontal component, which is the plane in which the GPS hubs are monitored.

Slide Boundaries: There are two boundaries of the main slide mass that appear to be defined by the InSAR: the east and northern boundaries. The breaks in these boundaries are most apparent on the slant range data. On the northern edge of the site there is colour shift between

the blue and yellow zones that appears to be the break between slopes that are moving to the east, where Baker Creek flows northwest to southeast, and between slopes that are moving towards the northeast, where Baker Creek wraps around to run east-west. This is not a distinct boundary but an inferred boundary has been sketched on Figure 1. On the flow line deformation map, these are portions of the study area that are moving with more of a northerly component (more of an oblique angle to the look direction of the satellite) and therefore there is more noise (error) in the data and it is not well quantified in the flow line data.

Along the eastern edge of the study area, there appears to be a distinct change between the blue and yellow tones that agrees well with the GPS monitoring hub data and previous AMEC mapping of the suspected toe of the slide mass. Although this represents a very distinct and clear change in levels of slope movement activity, this boundary likely has an error range of +/- 50 metres due to averaging of data in this area. This boundary also appears distinct as it wraps around to the south east portion of the study area.

Along the southern edge of the study area, with the exception of the area in the southeast that appears to be well defined, there is a general loss of coherence of data in the middle portion and a significant loss of coherence in the vegetated areas to the west that does not allow for a reasonable delineation of the boundaries of the slide mass. While it is not possible to assess the boundaries in the southern and western portions of the study area, there is some movement data available upslope, to the west that suggests movements with a similar range of motion (up to and exceeding 6 cm) as seen in the urban area. Based on AMEC's experience in the area, it is not clear whether a southern boundary would be expected as the similar processes as are occurring at West Quesnel may be occurring to the south.

Differential Movements: The slant range data (Figure 1) appears to show zones of differing movement magnitudes within the urban area that may correspond to differential movement within the slide mass. These data trends are the same as shown in the flow line data and appear to vary between 4 cm to >6 cm over the Oct 98 to July 99 time frame considered. As there is some level of noise (error) in the data, and while the trends appear to be real, definitive zones of movement within the slide cannot be determined based on only two scenes of data. A method of providing a more accurate delineation of movements within the mass would be to use an InSAR technique called Coherent Target Monitoring (CTM) over this site. CTM was not originally provided as an option as the overall costs would likely be three to four times that of the current assessment. This technique utilizes more frequent ERS or RADARSAT scenes, identifiable hard targets (houses), and has the potential to delineate very fine movements (within +/- 5 mm) for individual areas that approximate the pixel resolution area of the SAR sensor. For West Quesnel, this technique would likely be able to collect data from hundreds of houses to provide refined delineation when compared to the small number of existing GPS hubs. This method has been shown to be financially competitive with equivalent density differential GPS surveys on other sites.

5.0 CONCLUSIONS AND SUMMARY

The discussion provided above has been based on a review of the available flowline and slant range interferometry data, coupled with the ongoing work at West Quesnel being undertaken by AMEC and others.

Overall, InSAR appears to have been able to provide reasonable delineation of the eastern and southeastern boundaries of movement within the developed area of West Quesnel where terrain is not heavily vegetated. In addition, based on the interpretation of the slant range data, a poorly developed boundary appears along the northern edge which likely represents the break between slopes that are moving with more of a northerly component versus the overall West Quesnel slide which moves to the east. In many of the areas that have been discussed in this report, there have been ongoing visual observations of deformation, and in some cases, actual deformation data available. In many cases, the InSAR may be detecting centimeter level movement over the time periods assessed that may not be detectable using visual methods. Kosar et al (2003) have documented the use of InSAR to detect movements within Central British Columbia along the Fraser River in which deformations associated with a highway and railway alignment that were initially not observed visually prior to InSAR analysis were subsequently confirmed after completing a follow up field check. It is recommended that more detailed field visits be undertaken for the sites highlighted in this report to initiate examination of slide areas that have not been previously monitored or field checked.

This project has involved the application of InSAR as a tool for detection of ground movements within West Quesnel in order to provide additional information on which to plan further field assessment and investigation as part of an overall risk management framework for mitigating the impacts of large scale landsliding on this area. Caution should be used in utilizing InSAR as a specific monitoring tool to replicate and replace existing slope instrumentation, but rather it should be utilized to supplement existing information.

City of Quesnel
West Quesnel
Quesnel, British Columbia
22 November 2004



6.0 CLOSURE

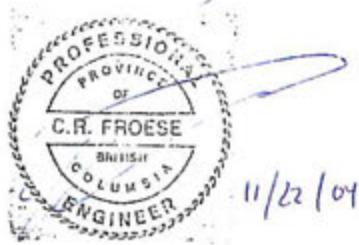
The information provided in this report is for the exclusive use of the City of Quesnel in conjunction with ongoing West Quesnel land stability studies being carried out by AMEC. The user is cautioned that due to the nature of InSAR measurements a complete movement vector is not provided and some movements may not be detected. The information in this report should be reviewed together with other information before finalizing actions or conclusions relative to ground movements in or near the study area. This study has been undertaken utilizing the InSAR and available published data and no field truthing by the authors has been completed as part of this study. Use of the data provided is subject to the Atlantis Scientific Inc. licensing agreement that is provided with the submission of the electronic data to the City of Quesnel. Any use or further interpretation of this data by others is subject to the licensing agreement and neither AMEC nor Atlantis Scientific Inc. will be held responsible for interpretations of the data provided made by other parties.

If you have any questions or require any further information, please contact Nick Polysou P.Eng. at (250) 564-3243 or nick.polysou@amec.com.

We have enjoyed working with the City of Quesnel on this interesting project and look forward to continued involvement in the future.

Respectfully submitted,

AMEC Earth & Environmental



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Reviewed by:

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Principal Geological Engineer

7.0 REFERENCES

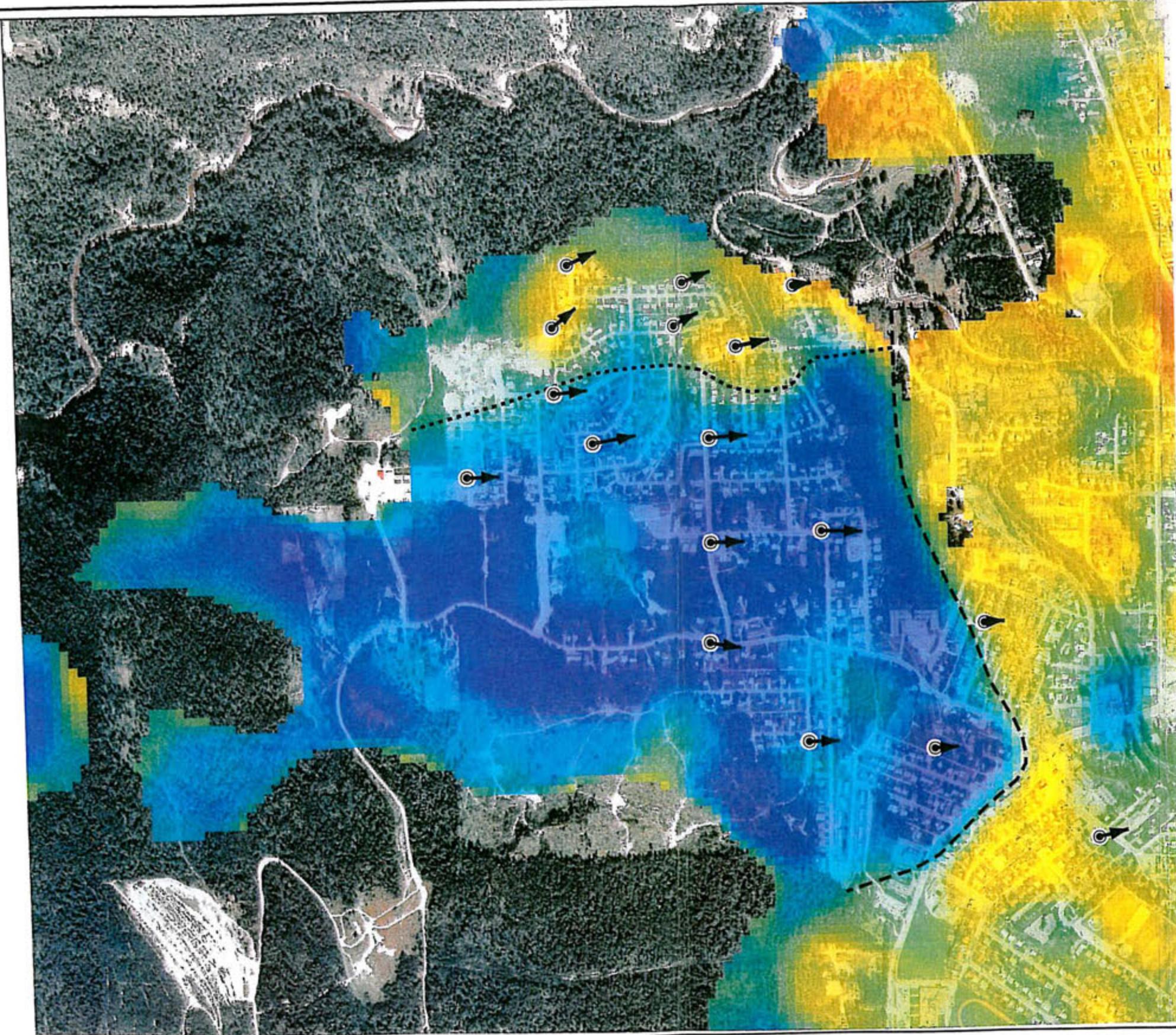
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LIST OF FIGURES

**FIGURE 1 WEST QUESNEL INSAR ASSESSMENT SLANT RANGE DEFORMATION
OCTOBER 24, 1998 TO JULY 31, 1999**

**FIGURE 2: WEST QUESNEL INSAR ASSESSMENT FLOW LINE DEFORMATON
OCTOBER 24, 1998 TO JULY 31, 1999**

- NOTES:**
1. Orthophoto provided by the City of Quesnel.
 2. Terasen gas movement hub data has been interpolated to reflect the same time frame as InSAR measurements.
 3. Slant Range deformation data represents movement in the line of site of the satellite and does not represent absolute values of movement in the direction of movement of the landslide.
 4. Data obtained from ERS-2 descending trajectory with a look direction azimuth bearing of 285° and incidence angle (angle between beam and vertical) of 23.3°.
 5. Areas where there is no colour do not necessarily indicate that there are not deformations occurring but rather that the data quality is insufficient to detect deformations due to data decorrelation or movement directions in relations to the satellite.



Deformation Results Provided By

ATLANTIS

Orthophoto: Scanned BCC97136 No. 73 (1997)

Client:
City of Quesnel

AMEC Earth & Environmental

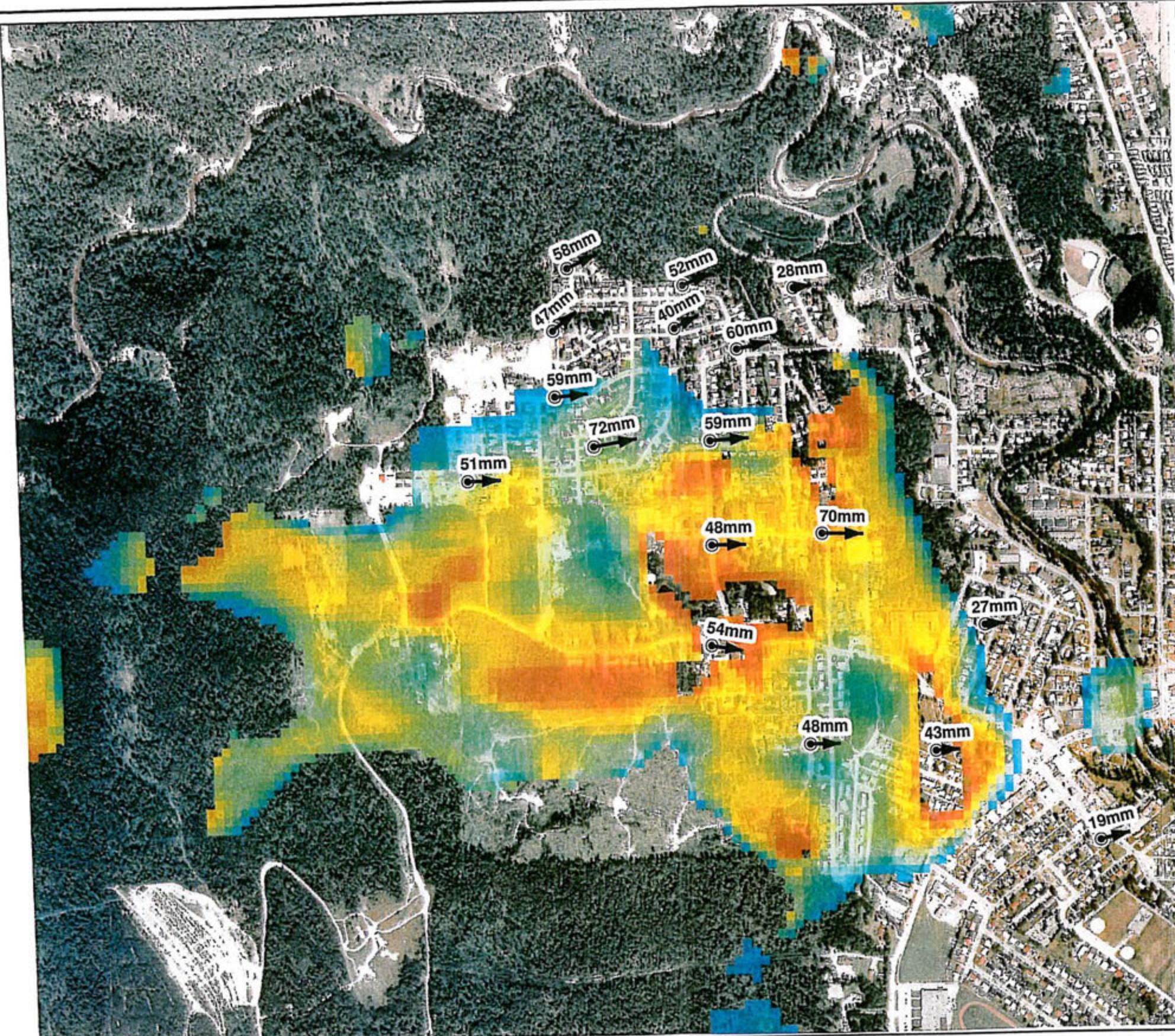
Projection:	UTM Zone 10
Datum:	NAD83
Scale:	1:10 000
Date:	November 2004

**WEST QUESNEL
INSAR ASSESSMENT
SLANT RANGE DEFORMATION
OCTOBER 24, 1998 TO
JULY 31, 1999**

AMEC Project:
KX0439702
File:
WQuesnel_Slant.mxd
Figure 1

NOTES:

1. Orthophoto provided by the City of Quesnel.
2. Flow line deformation data provides the deformations observed from the satellite (slant range) that have been resolved in the direction of the ground surface. As movements for deep seated landslides are often not parallel to the slope surface, these movements will vary from actual deformations but have provided a reasonable estimation of the magnitude of the deformation over this time period. Caution must be used in interpreting the values shown on this map as minor changes in ground topography could lead to changes of deformation magnitude within the overall slide mass that are not actually present.
3. Data obtained from ERS-2 descending trajectory with a look direction azimuth bearing of 285° and incidence angle (angle between beam and vertical) of 23.3°.
4. Areas where there is no colour do not necessarily indicate that there are not deformations occurring but rather that the data quality is insufficient to detect deformations due to data decorrelation or movement directions in relation to the satellite.
5. The deformation values provided for the GPS monitoring hubs represent an interpolation of the Terasen data to correspond to the same time period as considered for the InSAR assessment in order to provide a gross comparison of deformation rates over this time period.



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Movement Vectors at the
Terasen Gas GPS Monitoring Hubs

Deformation Results Provided By

ATLANTIS

Orthophoto: Scanned BCC97136 No. 73 (1997)

Client:
City of Quesnel
AMEC Earth & Environmental

Projection: UTM Zone 10
Datum: NAD83
Scale: 1:10 000
Date: November 2004

**WEST QUESNEL
INSAR ASSESSMENT
FLOW LINE DEFORMATION
OCTOBER 24, 1998 TO
JULY 31, 1999**

AMEC Project:
KX0439702
File:
WQuesnel_Flow.mxd
Figure 2

APPENDIX A
INSAR SUBSIDENCE MONITORING: DEFORMATION MAPPING WEST QUESNEL

InSAR Subsidence Monitoring:
Deformation Mapping
West Quesnel

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October 15, 2004

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1. Introduction

Atlantis Scientific has executed approximately 150 projects using InSAR technology since 1997. The objective of these projects was to create very accurate maps of displacement using spaceborne radar technology. These projects have resulted in a number of operational monitoring projects in North America and the Middle East.

This report describes the generation of deformation maps of the West Quesnel site using SAR interferometry for the time period October 24, 1998 to July 31, 1999 using ERS descending data. The report will give a short introduction in InSAR deformation mapping technology and will provide details of the processing including intermediate and final results. The objective is to create suitable format, projection and visualization of the final results.

Figure 1 indicates the West Quesnel site. The co-ordinates for site are given by a box with coordinates 53° 00' 21" N 122° 34' 26" W to 52° 56' 40" N 122° 28' 06" W. The extent is approximately 6.8km x 7.2km.

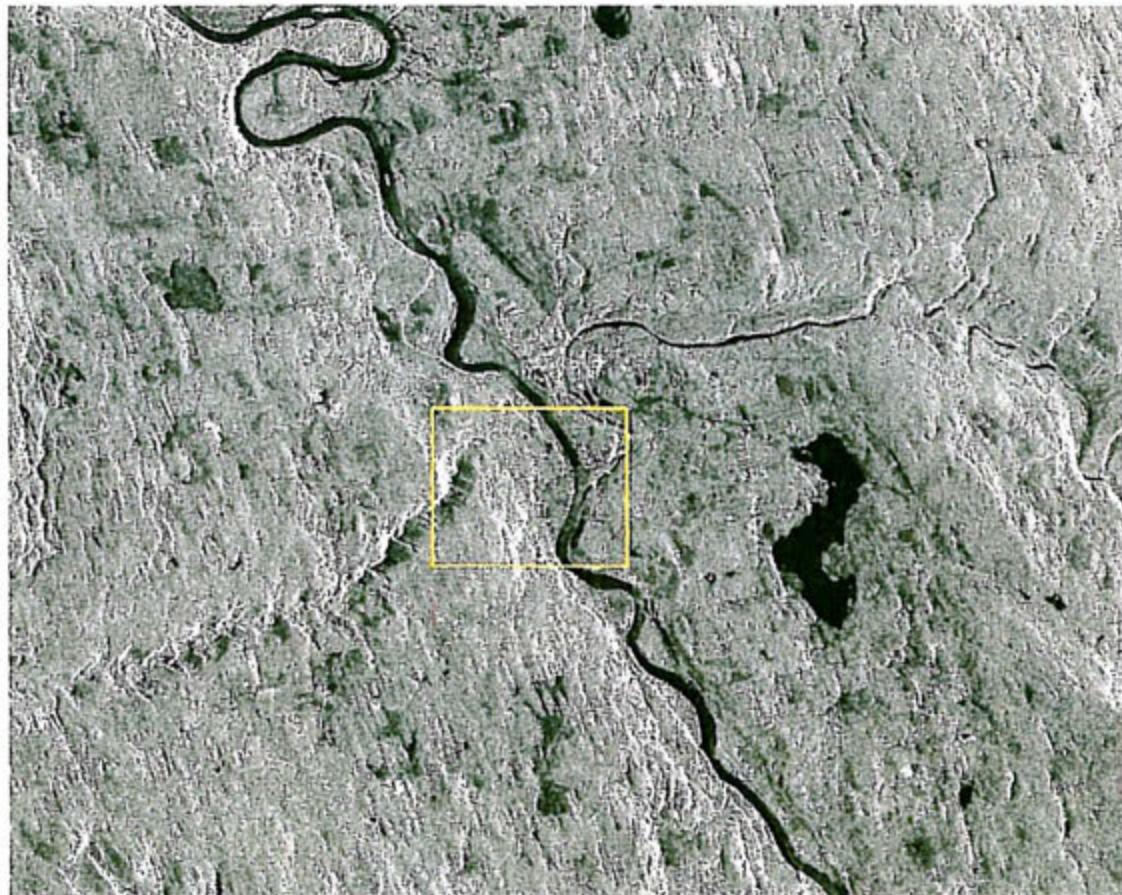


Figure 1: West Quesnel Site

2. Deformation Mapping Using InSAR

SAR systems measure both the magnitude and the phase of the transmitted electromagnetic signal that is backscattered from the earth's surface. The magnitude is what we usually associate with a remotely sensed image. It indicates the "brightness" of the surface as detected by the SAR. The phase represents the combination of two factors; the first is a quantity related to the distance from the SAR to the surface, and the second is the surface scattering effect on the incident electromagnetic wave. Given that this scattering effect is independent from point to point on the ground, the phase of a SAR image is in general quite random and of no practical purpose on its own.

If a second SAR data set is collected from nearly the same location as the first then upon subtracting the phase of the second image from the first an interferogram is formed. If the distance (called the baseline) between the two locations of the SAR platform is small then the surface scattering effects will be the same for the two images and are thus canceled in the formation of the interferogram. Therefore the interferogram exhibits information related to the difference in distances from the surface to the two SAR locations. Knowing the geometry involved, topography can be derived from this information. It is shown that the sensitivity of the phase to height is proportional to the baseline. Heights can typically be measured to an accuracy of 1 to 10 metres.

If surface motion occurs between the acquisition times of the two imaging passes of the SAR platform then the interferogram phase represents a combination of topographic information and surface change. If the topography for the area before the change is known it can be used to remove the topographic phase information from the interferogram leaving simply the surface change phase. The sensitivity of the phase to surface change is proportional to the wavelength of the transmitted electromagnetic wave. Thus surface change can potentially be measured with accuracy at the centimetre level or better.

The steps to generating a deformation map using InSAR are

- **SAR data set selection**
- **SAR raw data processing.** Both scenes are processed to a product that include intensity information plus phase information.
- **Image coregistration.** Both scenes are very accurately co-registered (lined up) to sub-pixel accuracy.
- **Interferogram generation.** The interferogram is an image of the phase difference between corresponding pixels of the 2 images.
- **Topographic phase removal.** An external Digital Elevation Model (DEM) of moderate accuracy is used to remove effects of stationary topography (see more in next section).
- **Phase unwrapping.** For the generation of a quantitative deformation product it is crucial to create a map of "absolute phase" from "wrapped phase" values. This process is called "phase unwrapping".
- **Terrain distortion correction (orthorectification).** This process removes specific distortions due to the radar geometry.
- **Ground control point collection and position refinement.** This process is required as the geographical location of the SAR data is not accurate enough. Ground control points from standard topographical maps are generally sufficient to create horizontal accuracies of ~25-50 m.
- **Deformation map generation in requested format and projection/datum**

These steps are described in the following subsections, as they pertain to the data processed in this project.

3. Final Data Selection

In general, for final selection of the InSAR pair of interest a number of criteria is used:

1. Proper geometry for a pair of interest. This means that the so-called “baseline distance” between the repeated satellite passes is sufficiently small to allow interferometry. The maximum distance is in the order of 1000 m. Typically most of the Radarsat pairs of interest have a baseline within that range
2. The surface conditions are such that a satisfactory phase coherence level is to be expected. Poor conditions could include: (wet) vegetation, snow cover. Phase noise could also be high over areas that experience farmer activities such as plowing. In order to optimize the phase coherence Atlantis utilizes weather data and has the capability to assess phase coherence levels before committing to the purchase of a specific pair.
3. Continuity with previous acquisitions. It is attempted to create continuity with previous InSAR surveys. In some cases it is possible that there is an overlap with previous surveys or perhaps a small temporal gap might occur.

After assessing the geometry and phase coherence levels it was decided to commit to two differential pairs of descending data and ascending data. Table 1 lists the RADARSAT data purchased and Table 2 provides the interferometric pairs processed and Table 3 provides the final pair used for interferometric analysis.

In order to create a so-called differential interferogram it is required to use a Digital Elevation Model (DEM) to remove “Elevation Phase Effects”. A height accuracy of ~ 10 m is required to remove these elevation effects in order to focus on the extraction of highly accurate deformation phase information. Perhaps the most confusing aspect of SAR interferometry for new users of this technology is the fact that it is possible to use a relatively low quality digital elevation model for this purpose while still being able to estimate surface deformation at sub-cm level. The key is to recognize that an InSAR deformation map is created from a pair of SAR data and that only changes in distance between the ground and the satellite can be measured and not the absolute height at sub-cm level accuracy. Figure 2 shows a colour representation of the SRTM data that was used for the removal of topography. Area of interest is outlined with a black box.

SAR ID #	Sensor	Dates	Look	Orbit
01	ERS-2	1997-07-26	desc	11848
02	ERS-2	1998-10-24	desc	18361
03	ERS-2	1999-07-31	desc	22369
04	ERS-2	1999-09-04	desc	22870
05	ERS-2	1999-11-13	desc	23872
06	ERS-2	2000-09-23	desc	28381
07	ERS-2	2000-12-02	desc	29383

Table 1: List of SAR data

Pair	Date	Baseline
01-02	1997-07-26 to 1998-10-24	146
01-03	1997-07-26 to 1997-07-31	294
03-05	1999-07-31 to 1999-11-13	0.661
03-06	1999-07-31 to 2000-09-23	73
05-06	1999-11-13 to 2000-09-23	72
06-07	2000-09-23 to 2000-12-02	-3

Table 2: List of possible Interferometric pairs for analysis

Pair	Date	Baseline
02-03	1998-10-24 to 1999-07-31	148 m

Table 3: Interferometric pairs used for analysis

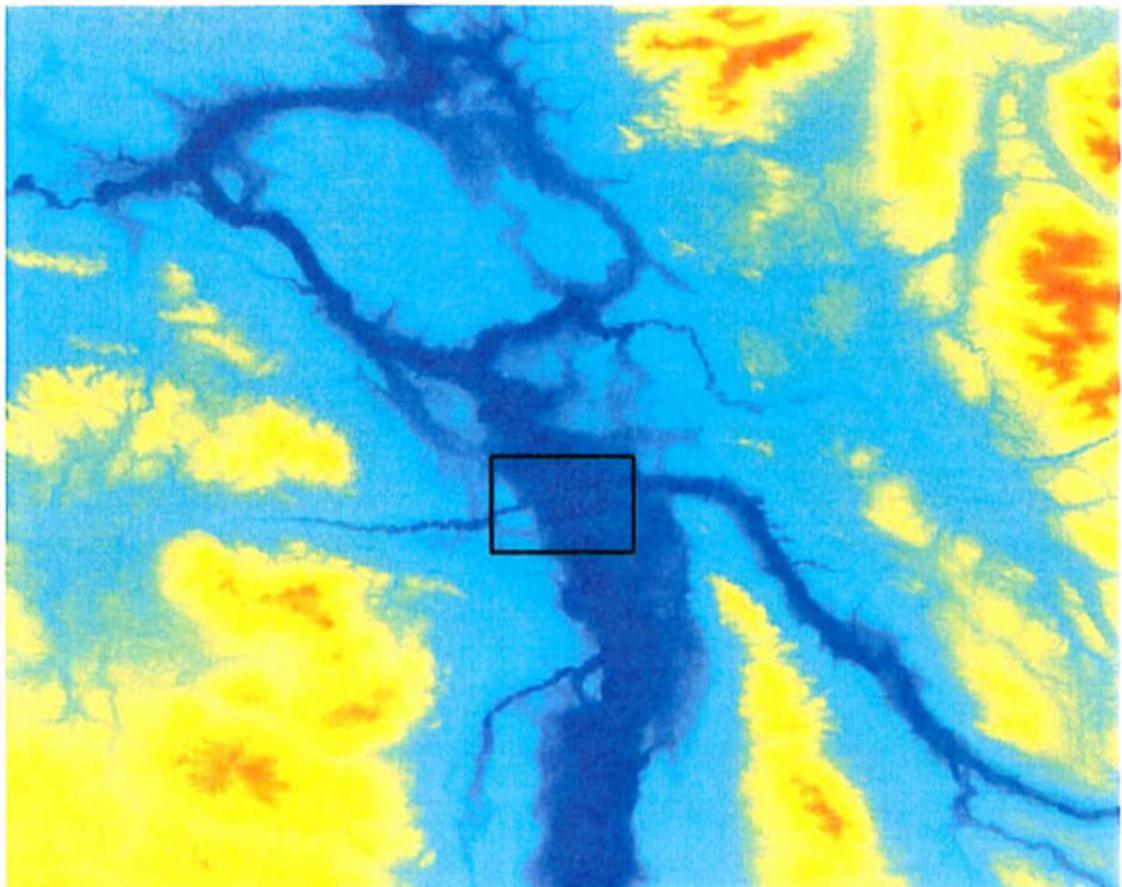


Figure 2: Color representation of the DEM used for removal of topography.

4. Results of Processing

4.1 Processing of the Raw SAR Signal Data

The raw data was processed using the EarthView™ Advanced Precision Processor (APP). The scenes were processed to a zero Doppler, slant range projection, and as phase preserved single-look complex (SLC) imagery. SLC scenes represent the focused images and are the required input for InSAR processing.

4.2 Image Coregistration and Interferogram Generation

Before an interferogram can be formed the input SAR images (master and slave) must be very accurately co-registered. Since the images are collected from slightly different locations, one, referred to as the slave, must be transformed to be coincident with the other, and referred to as the master. Given that under the condition of sufficient coherence the images differ only slightly, it is relatively straightforward to co-register them automatically.

If the baseline distance is nonzero then the interferometric phase will vary with the surface topography. If surface deformation occurred between the first SAR acquisition and the second SAR acquisition, it will also contribute to the interferometric phase, as a SAR signal-path difference. Thus in order to extract surface deformation measurements the topographic phase component must be removed. The DEM must first be co-registered with the master SAR image. This is done using the georeferencing of the DEM and the SAR geometry corresponding to the master. However, the absolute georeferencing is often not accurate enough to provide exact coregistration and usually it must be refined manually. The phase due to the topography is calculated directly from the DEM using the SAR geometry corresponding to the master image and is simply subtracted from the interferogram phase. Once the topographic phase has been removed the result is termed a differential interferogram. The phase information in a differential interferogram solely represents temporal differences (surface movement, atmospheric effects). Figure 3 (a) (b) and (c) shows the backscatter image, coherence image and enhanced interferogram of pair 02-03. The intensities in the coherence maps range from 0 (black, low coherence = high phase noise) to 1 (high coherence, low phase noise) and are indicative of the quality of the interferogram. The phase colour scheme is next to the interferogram. Negative phase identified by fringes going from blue to red to green, which is represented as negative values on the deformation. Positive phase is identified by fringes going from red to blue to green and is represented as positive values on the deformation map.

4.3 Phase Unwrapping and Deformation Map Generation

The phase as measured by the SAR and represented in the interferogram is referred to as wrapped, having values only in the range between 0 and 2π . In order to relate the phase to topography or surface deformation the phase must be unwrapped. In generating a DEM with areas of rugged terrain phase unwrapping can be a difficult task. However for determining surface change the nature of deformations that can be detected are typically not as complex and it is straightforward to unwrap the phase.

The surface deformation is related directly to the unwrapped phase, through the SAR geometry and transmitted signal wavelength, and is straightforward to calculate.

4.4. Georeferencing and calibration of deformation values

The images were projected into UTM, WGS-84 map coordinate and accurately georeferenced by identifying common features from the SAR backscatter image and an orthorectified SAR image. The deformation map was calibrated using the assumption that the deformation outside of the area where motion occurs is negligible.

Figure 5 shows the slant range change map of ERS2 descending data for the time periods October 24, 1998 to July 31, 1999.



Figure 3(a) Backscatter

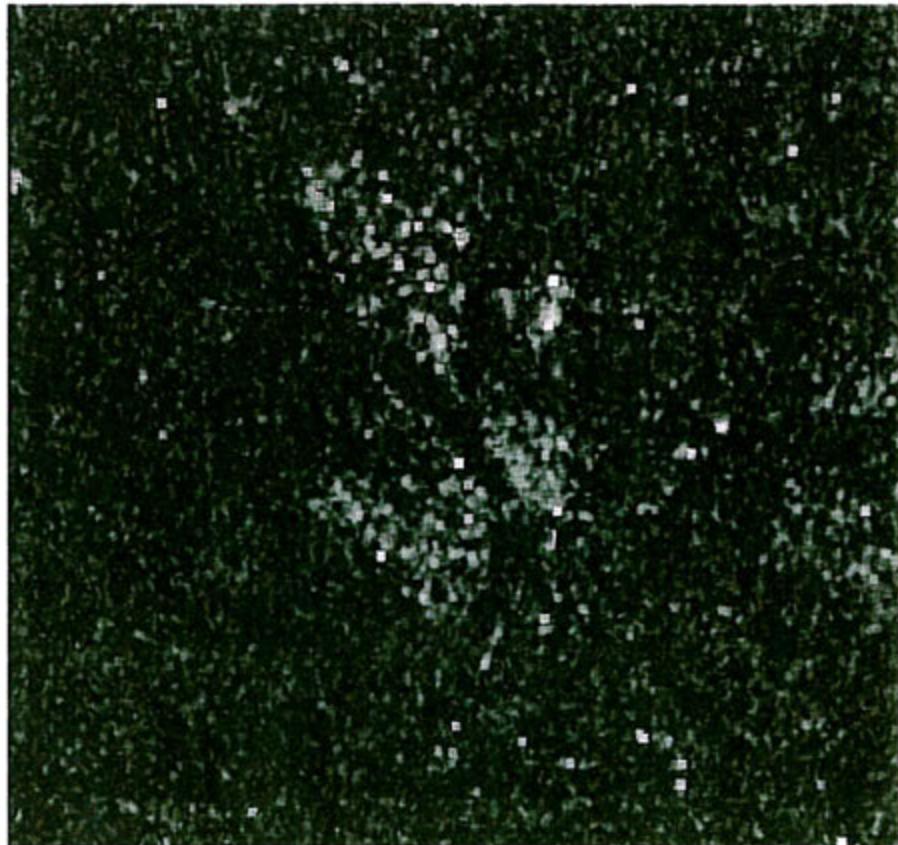


Figure 3 (b) Coherence

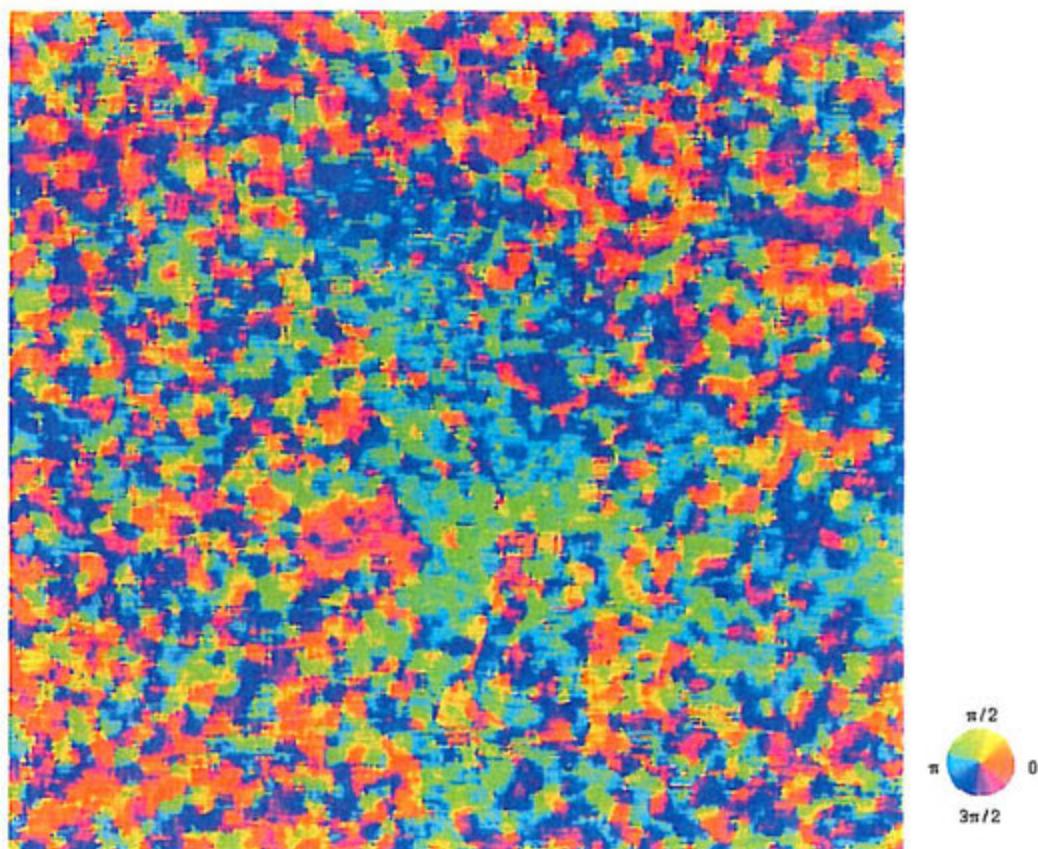


Figure 3 (c) Differential interferogram

Figure 3: Coherence and differential interferogram of the ERS pair October 24, 1998 to July 31, 1999 (Descending).

4.5. Flow-line Deformation Map

A flow line deformation map is a slant range change map projected to the line of local terrain slope. The local terrain slope was obtained by calculating gradient vector from Canadian Topographic Map. Then, slant range change derived from InSAR data was projected to the line of the gradient vectors. Figure 4 displays the geometry used for the calculation of the flow line deformation. Figure 8, shows the flow line deformation map. Since both positive and negative slant range change are projected to down slope, both features are shown with same negative sign in the flow line deformation. Because of the geometry, flow line deformation could become large when the angle between slant range vector and flow line vector is close to 90 degree. Therefore, the area was masked out.

Figure 5 shows the slant range change map of ERS descending data for the time period October 24, 1998 to July 31, 1999 with the vectors showing the direction of the motion. The vectors on the slant range change value greater than 5mm are only displayed. Also, the vectors on the layover area, low coherence area (less than 0.2), and the area where the angle between flow-line vector and slant range vector is close to 90 degrees, has been masked out. Figures 6 and 7 shows the slant range change deformation vectors displayed on the orthophoto image and the LANDSAT image.

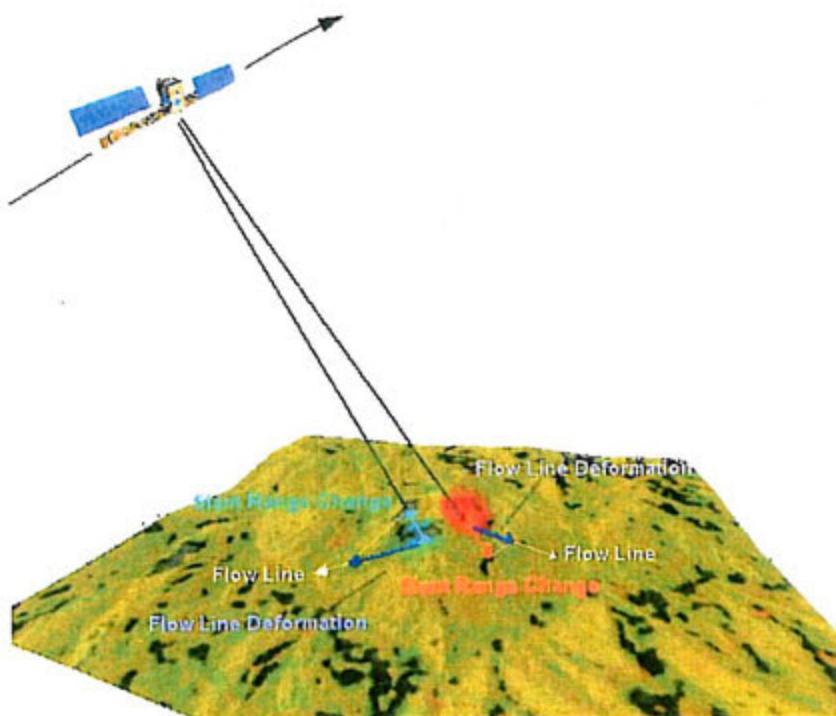


Figure 4: Geometry used for calculation of flow line

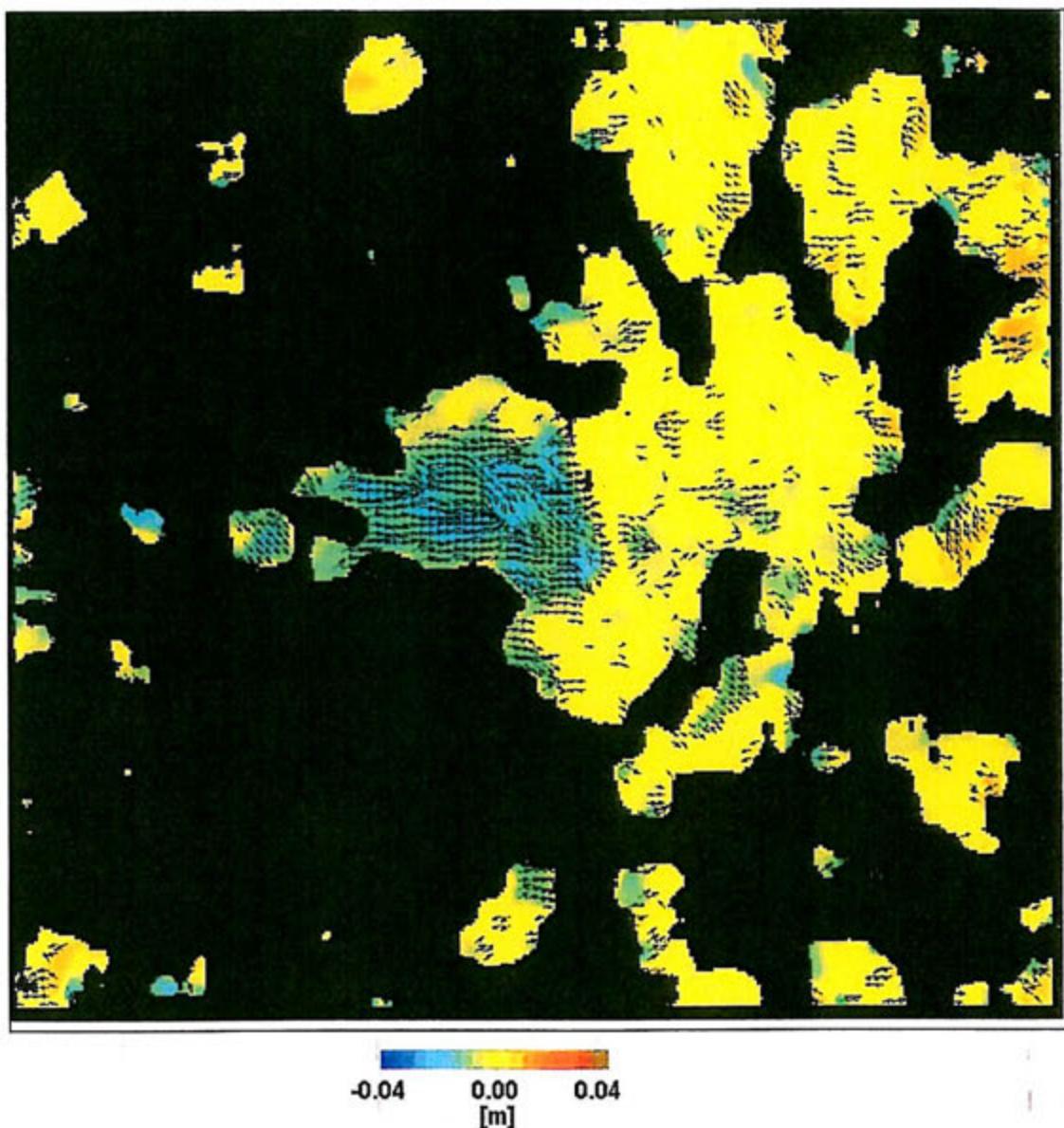


Figure 5: Slant range change map for ERS pair October 24, 1998 to July 31, 1999.

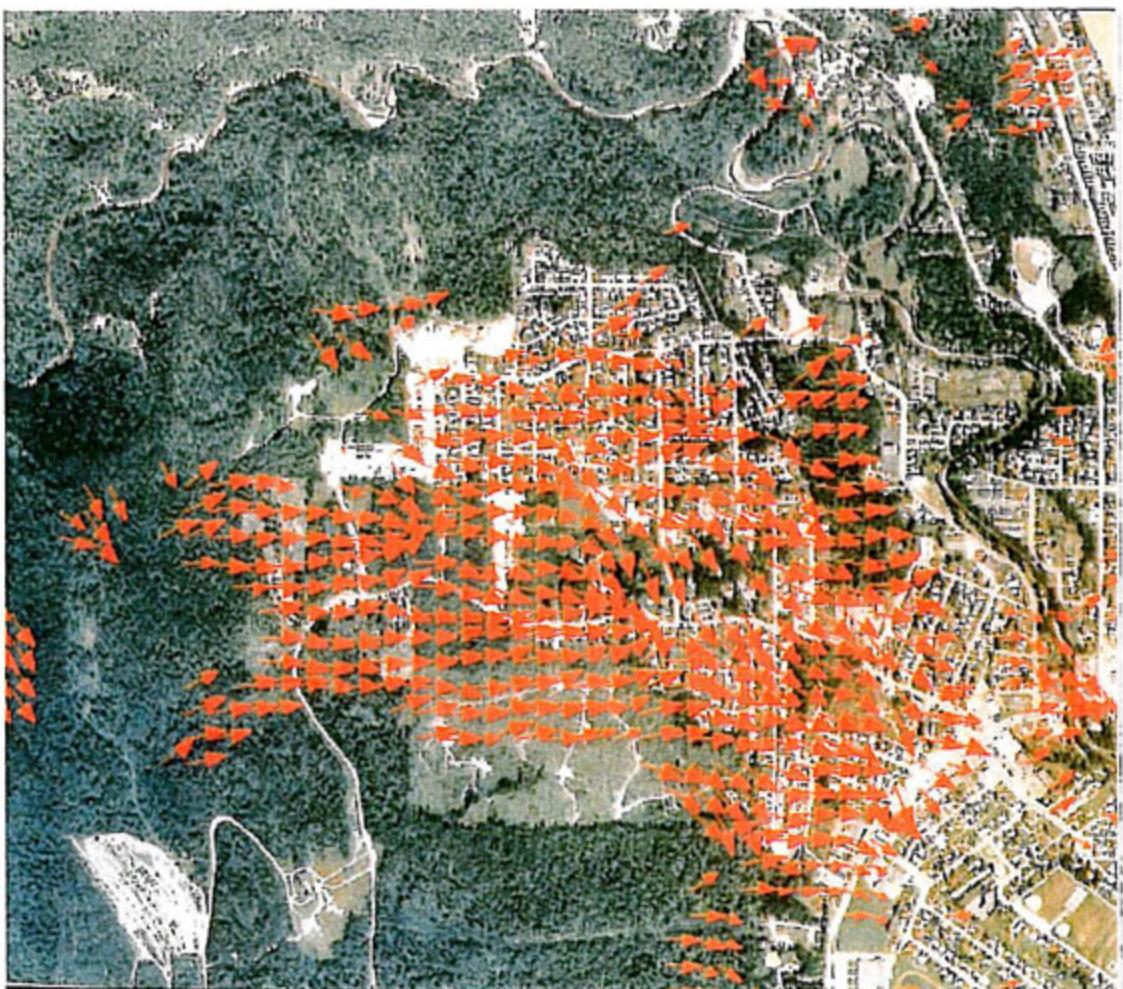


Figure 6: West Quesnel orthoimage with flow line vector layer.

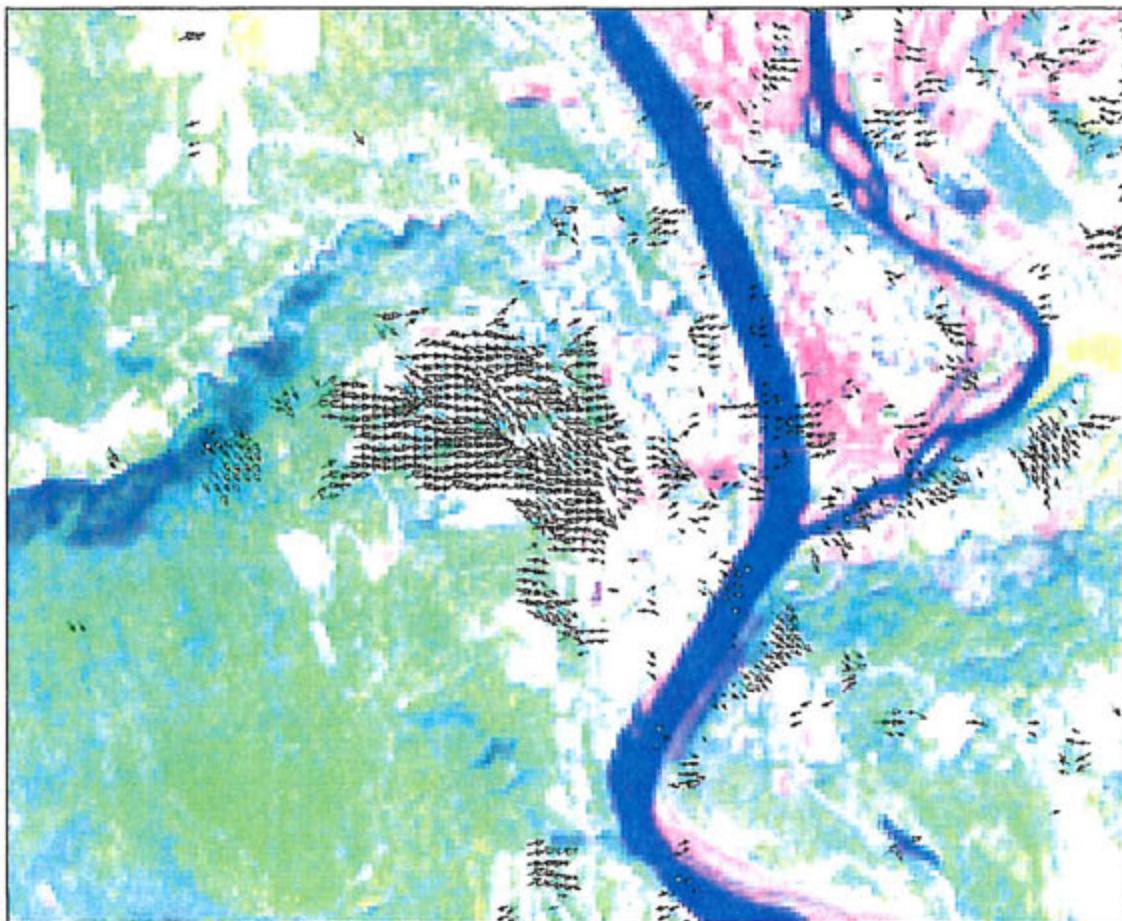


Figure 7: Landsat image with flow line vector layer.

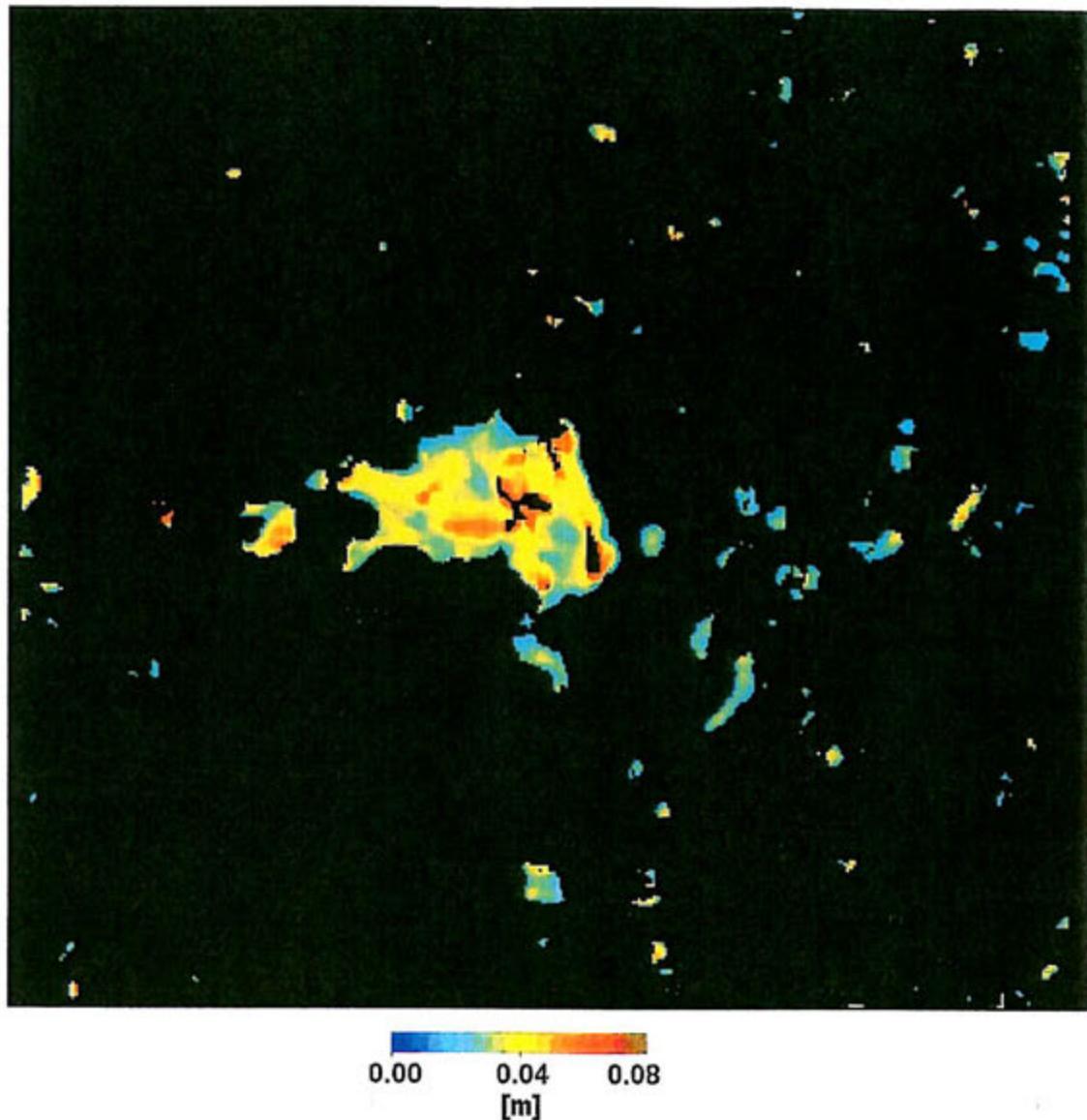


Figure 8: Flow line deformation map for ERS pair October 24, 1998 to July 31, 1999.

5. Discussion

The interferogram of ERS descending data for the time period October 24, 1998 to July 31, 1999 show the major deformation within the area 122d32'28"W 52d58'54"N and 122d30'43"W 52d58'10"N.

6. Deliverables

The deliverables consist of this report, projection report and flow line vector in shape file format. Backscatter, coherence, slant range change map and the flow line deformation map are also provided in GeoTiff file format. All files are in UTM projection system, Datum WGS84. Deformation values in metres.

West Quesnel_v1.0.doc

Descending data:

102498_073199_coherence.tif
102498_073199_backscatter.tif
102498_073199_slrq_change.tif
102498_073199_slrq_color.tif
102498_073199_flowline.tif
102498_073199_flowline_color.tif
102498_073199_vector.dbf
102498_073199_vector.shp
102498_073199_vector.shx



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January 18, 2004

The City of Quesnel
c/o: AMEC Earth & Environmental Limited
610 Richard Road
Prince George, BC
V2K 4L3

Attention: Mr. Nick Polysou, P. Eng.

Re: West Quesnel, BC Slope Study Project – Electrical Resistivity
Tomography Survey

Dear Mr. Polysou;

Please find enclosed a letter report, prepared by Surface Search Inc., on behalf of The City of Quesnel and AMEC Earth & Environmental Ltd, for the Electrical Resistivity Tomography (ERT) survey completed in August of this year within West Quesnel, BC.

Included herein are details of the ERT methodology used by Surface Search Inc, to probe subsurface conditions below West Quesnel and a discussion of the results obtained.

1. INTRODUCTION

In July 2004, Surface Search Inc. (SSI) received authorization from AMEC Earth & Environmental Ltd (upon approval from The City of Quesnel) to undertake and complete a Electrical Resistivity Tomography (ERT) geophysical survey within West Quesnel, BC.

The primary objective of the geophysical ERT survey was to obtain cross-sectional subsurface information, to be used by AMEC Earth & Environmental Ltd. (AEE) in their on-going study / assessment of slope ground movement within the West Quesnel residential area.



The scope of the ERT survey included:

- 1) Acquisition of approximately 7.5 line kilometers of ERT data along north-south and east-west running transects within the West Quesnel residential area;
- 2) Processing and interpretation of modeled ERT inversion results for all the acquired data;
- 3) Production of hard copy data results including ERT model inversion cross-sections for all of the survey transects, and cross-sectional diagrams depicting interpreted subsurface geologic features based on the ERT model inversions correlated with available borehole information;
- 4) And finally, production of a letter report detailing the ERT survey methodology used and a discussion of the ERT survey results obtained.

2. SURVEY METHODOLOGY

SURVEY DESIGN & LOGISTICS

Survey design, field work logistics and the scope of the ERT survey program were established in consultation with Mr. Nick Polysou, P. Eng., of AEE, prior to commencement of the geophysical program. To design a geophysical survey program for the West Quesnel Slope Study Project, ground surface conditions and potential subsurface conditions within the survey area were discussed. At this time, subsurface conditions were anticipated (from available AEE borehole information) to consist primarily of unconsolidated silt and clay deposits overlying slightly more consolidated tertiary silt and clay sediments and/or consolidated bedrock. The primary target of interest, to potentially be delineated by ERT, was the interface boundary between unconsolidated sediments overlying the partially consolidated Tertiary deposits. Based on the available borehole data, this interface was anticipated to lie within the upper 80 m from ground surface throughout the survey area. Of secondary importance was the delineation of the upper surface of consolidated bedrock, underlying the Tertiary deposits.

To satisfy these objectives, an ERT survey program was proposed which included the acquisition of ERT data measurements to 80 m in depth below ground levels. The ERT transects were to be laid out along existing residential roads and pathways in both north-south and east-west line orientations.



ERT DATA COLLECTION

The ERT data acquired for this project were collected using a multi-electrode Syscal R1 Plus ground resistivity system. The R1 Plus ERT instrumentation is comprised of a highly sensitive resistivity meter, an electrode switch-box, four 120 m long multi-core cables, and 48 stainless steel electrode ground stakes. To set up the ERT system for survey measurements, electrodes are pushed into the ground at evenly spaced distance intervals and connected to the multi-core cables, which in turn, are connected to the R1 Plus resistivity meter / switch-box.

The ERT survey setup used for this project involved pushing electrodes (i.e. stainless steel stakes) 0.1 to 0.2 m into the ground at 10 m station intervals. The grounded electrodes were then attached to the multi-core cables, which in turn, were connected to the R1 Plus switchbox and resistivity meter.

Electrode contact tests were completed at all electrode positions, prior to the acquisition of ERT measurement data, to ensure sufficient electrical current transmission between the electrodes and the ground. Where contact test measurements exceeded 5000 Ohm m, adjustments were made to attempt to improve the electrical contact between the electrode(s) and the ground. Typically, these adjustments involved pounding suspect electrodes further into the ground or checking to ensure proper jumper wire connections between the electrode and the array cable. At relatively infrequent intervals throughout the survey area, individual electrodes could not be pushed into the ground because of the presence of concrete driveways, associated with residential lots, and/or asphalt road crossings.

All of the ERT data recorded for this project were acquired using Wenner current/potential electrode activation sequences with automatic power input ranging up to 200W.

The aforementioned ERT survey parameters provided 5.0 m vertical resolution subsurface tomography imagery to a maximum survey depth of 80 m below the ground surface along the survey transects.

Survey position control for the ERT transect layouts and all electrode positions were referenced to Global Satellite Positioning (GPS) measurements acquired from a hand-held Garmin 76 GPS unit. Topographical control for the ERT transects was accomplished using NASA DEM data downloaded off the internet.



ERT DATA PROCESSING AND ANALYSIS

The ERT data acquired for this project were processed using RES2DINV ERT inversion software. Prior to running the inversion program and creating ground resistivity profile models, measurement data values with greater than 1% RMS error were removed from the raw data sets. To further improve data quality, an ILFA smoothing routine was used to remove isolated noise spikes from the raw data sets.

Filters, dampeners, surface topography, and model constraint parameters were then adjusted using iterative pre-inversion routines to account for:

- Measurement geometry and noise levels;
- Range and distribution of recorded resistivity values;
- Range and distribution of model sensitivity.

The pre-inversion iterative process gave specific consideration to:

- Topographic corrections (using NASA DEM data);
- Vertical compression of the inverse model grid;
- Spatial smoothing of the inverse model resistivity values
- Limiters on the range of resistivity values produced during inversion to decrease the influence of spurious values and/or noise;
- Values of the robust constraints on the resistivity values produced during forward modeling
- Dampener levels

All of the ERT data were inverted using selected parameters until model iterations stabilized to produce interpretable ground resistivity cross-section models of less than 5% RMS error (in comparison with the pseudo-section ERT values).

Data values from the modeled ERT results were overlain onto cross-section profile drawings for each ERT transect. Analysis of the combined ERT model data and available borehole data was completed to interpret / infer subsurface geologic conditions beneath the surveyed transect areas.



3. RESULTS

The results of the West Quesnel ERT survey include:

1. A plan map showing all of the ERT transect line locations acquired within the West Quesnel area (see Drawing No. 04-860-00);
2. Cross-section plot diagrams showing contoured electrical resistivity values, in Ohm m, produced from the inverted ERT data transects (see Drawing Nos 04-860-01 through 04-860-10, inclusive);
3. Cross-section plot diagrams showing contoured electrical resistivity values, in Ohm m, produced from the inverted ERT data transects, plotted alongside available borehole information and including our interpretation of the possible sedimentary interface between unconsolidated Quaternary deposits overlying slightly consolidated Tertiary deposits (see Drawing Nos 04-860-01A through 04-860-10A, inclusive);
4. Plan maps showing residual data values taken from the inverted ERT models at pre-defined depths below ground surface levels (i.e. at 10, 20, 40 and 60 m respectively (see Drawing Nos. 04-860-0R1 – 04-860-0R4).

4. DISCUSSION OF THE ERT SURVEY RESULTS

The inverted ERT model cross-sections obtained for this project are believed to contain reasonably reliable information about the electrical properties of the subsurface conditions below the surveyed areas, to a maximum depth of 80 m below ground surface levels. Although these results may be imperfect (see section below discussing possible sources for error in the data results), it is our belief that they are of significant value in providing direction for subsequent subsurface investigation and assessment initiatives within the West Quesnel area.

Our initial attempt at understanding or interpreting the ERT results, given the available borehole log data, is provided below. It should be noted however, that given the scope of the ERT survey, in comparison with the limited borehole log data available, we do not believe that there is sufficient ground control information to provide a fully detailed interpretation of the ERT results at this time. Should additional geotechnical information become available within the ERT survey area, it is recommended that these data be reviewed.



PRELIMINARY DISCUSSION ON THE WEST QUESNEL ERT DATA RESULTS

In correlation with borehole logs BH-3A, BH-4A and BH-6A, we interpret the 30 Ohm m interface boundary, within each of the ERT model sections, to depict the possible interface between unconsolidated Quaternary deposits overlying slightly more consolidated Tertiary deposits. In the slope indicator data obtained for SI-3, SI-4 and SI-6A, our inferred sedimentary interface appears to be in close approximation, in depth, with zones of maximum ground movement. However, in the slope indicator data obtained for SI-5, our interpreted sedimentary interface is shown to be significantly higher in elevation in comparison to the zone of maximum ground movement recorded by the slope indicator.

In addition to our interpretation of the upper Tertiary boundary interface, relatively high electrical resistivity value anomalies (e.g. modeled resistivity values in excess of 500 Ohm m) are noted within ERT profiles B-B', C-C', H-H' and I-I'. We believe these anomalies indicate the presence of granular sediment deposits (e.g. sand and/or gravel deposits) at depth. They may also be associated with the presence of water bearing unconsolidated sediments at depth.

To illustrate the lateral extent of the relatively high resistivity value anomalies, residual data values, taken from within the contoured ERT cross-sections, have been extracted at specified depths (e.g. at 10, 20, 40 and 60 m) at 50 m intervals along the length of each ERT data profile. Each depth layer of residual resistivity values was then contoured to provide a pseudo-plan view of the modeled ERT data results.

NOTES ON HOW TO INTERPRET THE WEST QUESNEL DATA

- 1 The ERT method measures the electrical resistivity of ground conditions below the surface. Resistivity, measured in Ohm m (or $\Omega \cdot m$), is the mathematical inverse of conductivity. It is a bulk physical property of materials that describes how difficult it is to pass an electrical current through the material. Resistivity measurements can be made with either an alternating current (AC) or a direct current (DC). As resistivity measurements are frequency dependant, care must be taken when comparing resistivity values collected using different techniques.

Clay materials, metallic oxides, and sulphide minerals are the only common sedimentary materials that can carry significant electrical current through the material itself. As such, the resistivity of most near surface sedimentary materials is primarily controlled by the quantity and chemistry of the pore fluids within the material. Any particular material can have a broad range of resistivity responses that is dependant on the level of saturation, the concentration of ions, the presence of organic fluids (such as non aqueous phase liquids, NAPLs), faulting, jointing, weathering, etc.



Table 1. provided below characterizes the normal range of resistivity values for common geologic materials.

Table 1. Normal range of resistivity values for common geologic materials.

Material	Normal Range of Resistivity ($\Omega \cdot m$)	
Galena	0.000,03	to 300
Hematite (Iron Oxide)	0.0035	to 10,000,000
Clays	1	to 100
Silts	20	to 400
Sands	80	to 1050
Gravels	100	to 1400
Permafrost	1000	to 10,000
Consolidated Shales	20	to 2000
Sandstones	8	to 4000
Limestones	50	to 400
Glacier Ice	50,000	to 120,000,000

- 2 By comparing the ERT results with the available borehole log data, resistivity values shown to be in excess of 30 Ohm m appear to coincide with unconsolidated Quaternary sand, silt and clay deposits.
- 3 By comparing the ERT results with the available borehole log data, resistivity values shown to be less than 30 Ohm m appear to coincide with more consolidate Tertiary sand, silt and clay deposits.
- 4 The 30 Ohm m boundary highlighted within the ERT cross-sections may be associated with the depth zone a maximum ground movement as identified in slope indicators SI-3, SI-4 and SI-6A. Where the ERT data intersects slope indicator SI-5, the 30 Ohm m boundary is significantly higher in elevation than the zone of maximum ground movement.



-
- 5 Users of this information should be aware that the horizontal and vertical resolution of the modeled resistivity value distributions shown on the cross-section plots is +/- 5.0 m.
 - 6 The vertical resolution of subsurface features diminishes with depth for all geophysical methodologies. Therefore users of this information should be cautioned to the possibility that relatively thin geologic sediment or rock sequence layers may not have been imaged, or detected at depth by the geophysical techniques used for this survey investigation.

SOURCES OF ERROR IN THE ERT DATA

Although every attempt has been made to obtain and provide accurate geophysical results for this investigation, sources for error must be taken into account by users of this information. Possible sources for ERT data measurement and subsequent inversion model errors for this project include:

- i) Irregular electrode spacings associated with locations where significant curvatures in the ERT transects occur (ERT data measurements assume a constant distance interval spacing between electrodes).
- ii) Inadequate smoothing/filtering of the data to infill absent electrode data (e.g. where concrete or asphalt was encountered and an electrode could not be pushed into the ground).
- iii) 3-D side effects associated with volumetric measurements taken in 3-dimensionally complex sediment/rock environments.
- iv) Problems associated with equivalence of geophysical measurements where two or more differing sets of geologic conditions produce similar geophysical responses.
- v) Shadow effects in the ERT data which tend to exaggerate the base depth of relatively high resistivity value anomalies (e.g. in situations where modeled ERT values > 1000 Ohm m) occur.



5. CLOSING REMARKS

The geophysical interpretations outlined in this report are the result of our best effort analysis Electrical Resistivity Tomography collected within the West Quesnel survey area. Should additional geotechnical information become available for this project, these results may be subject to review and/or require further alteration.

The geologic information depicted in this report is intended to serve as an assessment of the subsurface conditions likely to be encountered within the geophysical survey investigation area(s).

We trust that these geophysical results contribute to the overall geotechnical assessment of the West Quesnel slope study. Please contact the undersigned at (403) 531-9721 should you require any additional information or clarifications regarding this report.

Respectfully submitted,

A handwritten signature in blue ink, appearing to read "P. Tarrant".

Paul Tarrant
President - Surface Search Inc.

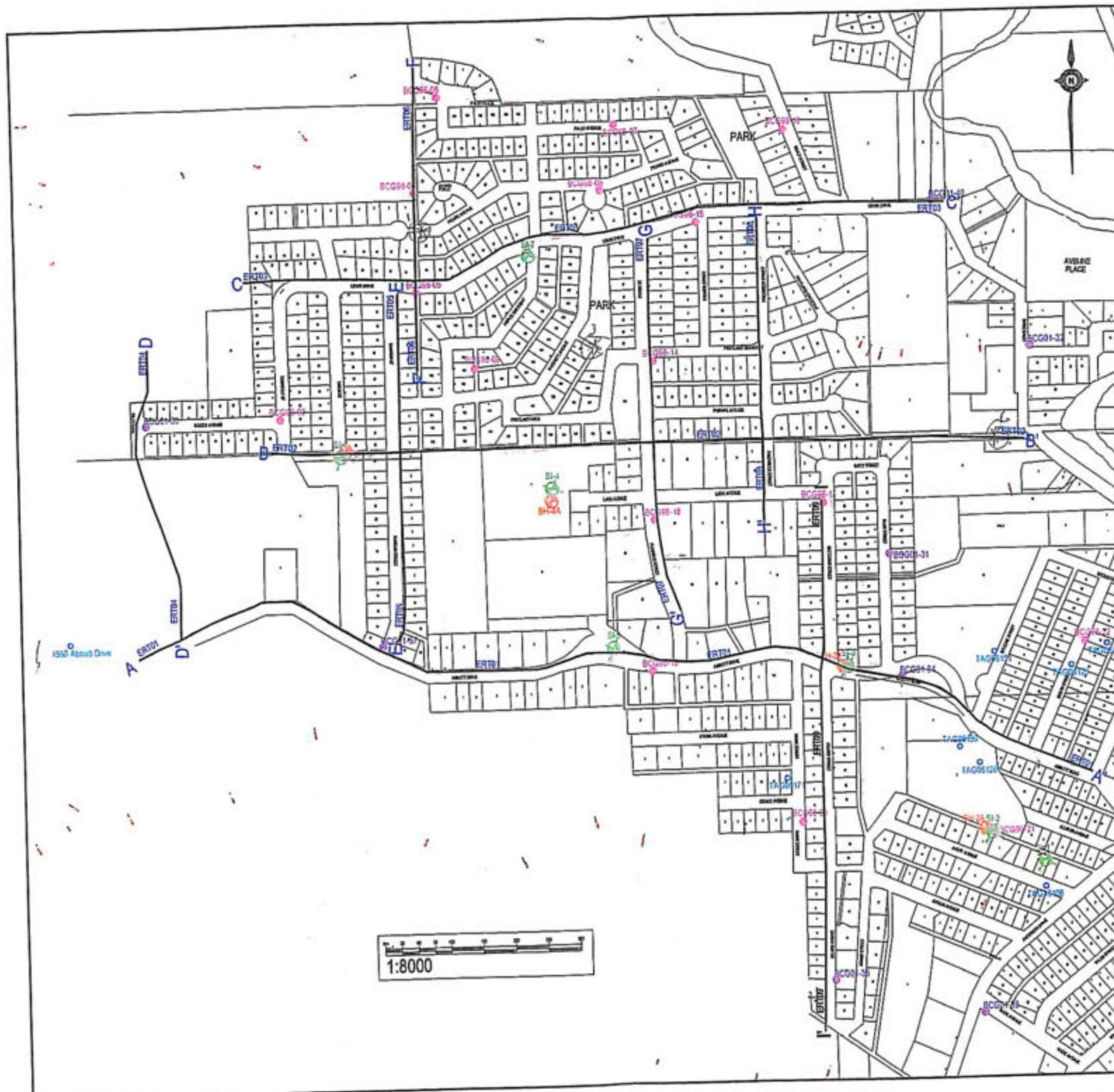


ENCLOSED RESULTS

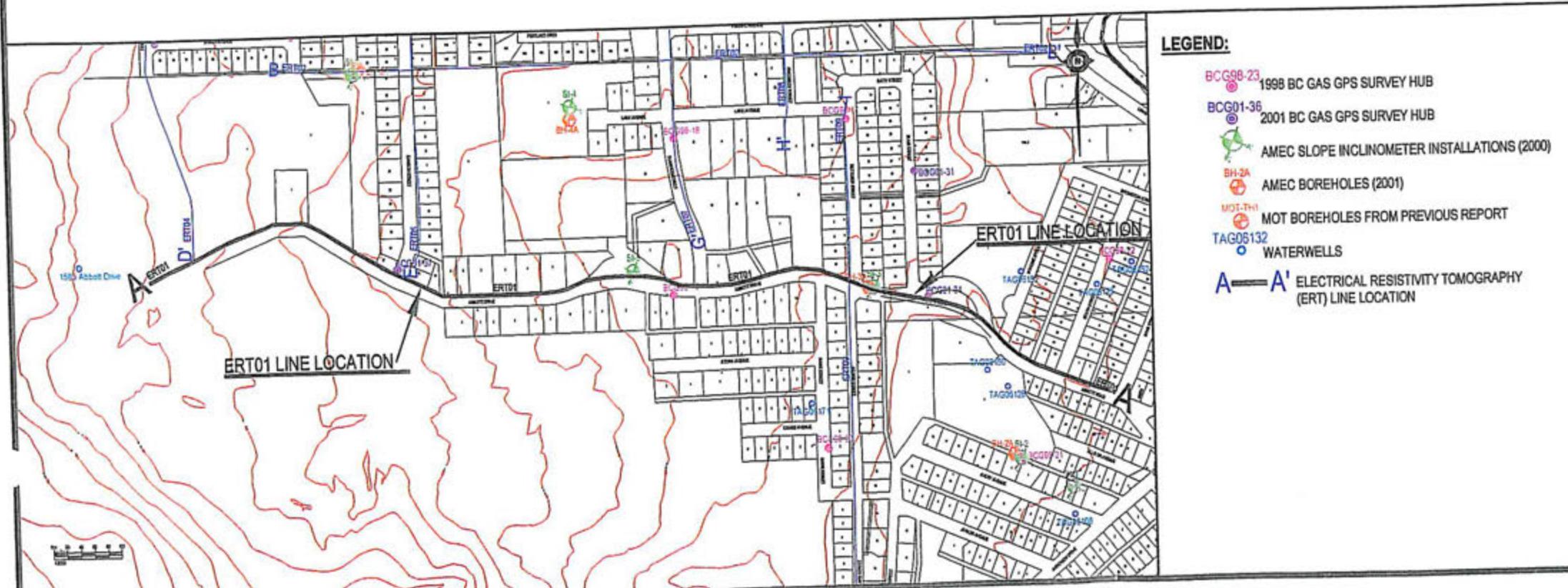
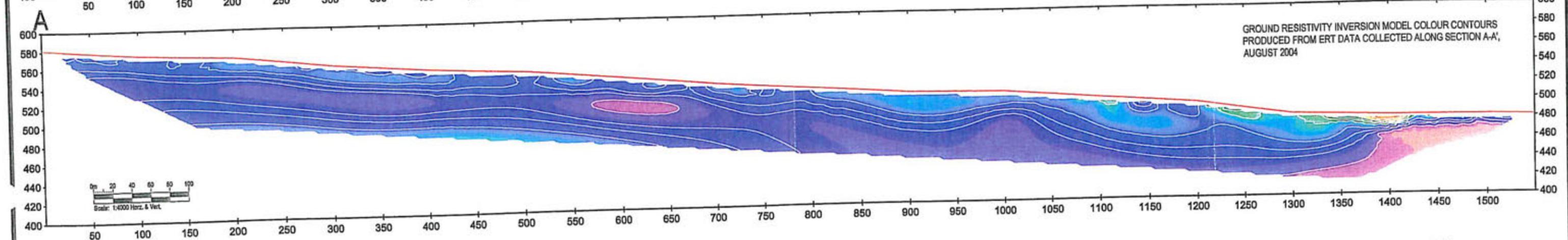
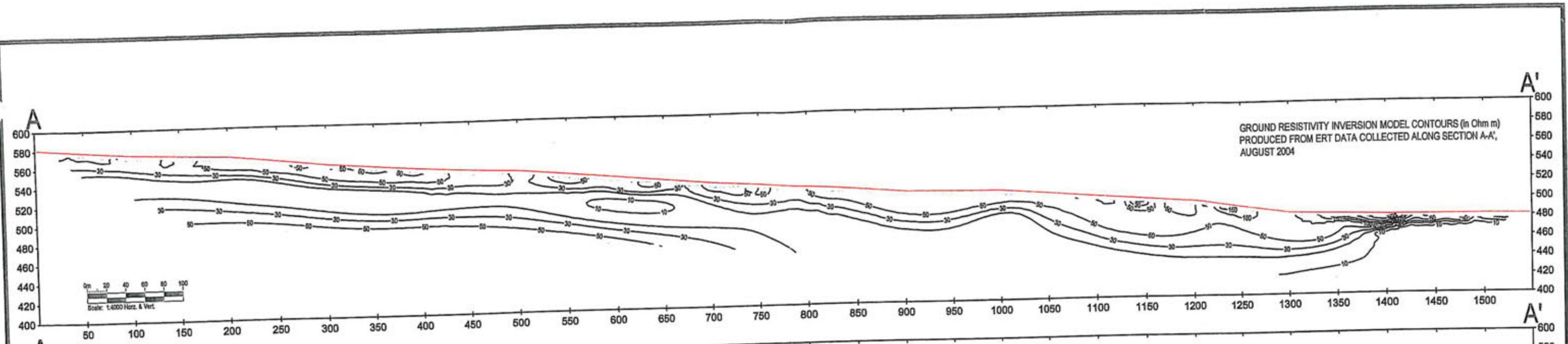
1. ERT SURVEY LINE LOCATION MAP, Drawing No. 04-860-00
2. ERT INVERSION MODEL CROSS SECTION A-A', Drawing No. 04-860-01
3. INTERPRETATION CROSS SECTION A-A', Drawing No. 04-860-01A
4. ERT INVERSION MODEL CROSS SECTION B-B', Drawing No. 04-860-02
5. INTERPRETATION CROSS SECTION B-B', Drawing No. 04-860-02A
6. ERT INVERSION MODEL CROSS SECTION C-C', Drawing No. 04-860-03
7. INTERPRETATION CROSS SECTION C-C', Drawing No. 04-860-03A
8. ERT INVERSION MODEL CROSS SECTION D-D', Drawing No. 04-860-04
9. INTERPRETATION CROSS SECTION D-D', Drawing No. 04-860-04A
10. ERT INVERSION MODEL CROSS SECTION E-E', Drawing No. 04-860-05
11. INTERPRETATION CROSS SECTION E-E', Drawing No. 04-860-05A
12. ERT INVERSION MODEL CROSS SECTION F-F', Drawing No. 04-860-06
13. INTERPRETATION CROSS SECTION F-F', Drawing No. 04-860-06A
14. ERT INVERSION MODEL CROSS SECTION G-G', Drawing No. 04-860-07
15. INTERPRETATION CROSS SECTION G-G', Drawing No. 04-860-07A
16. ERT INVERSION MODEL CROSS SECTION H-H', Drawing No. 04-860-08
17. INTERPRETATION CROSS SECTION H-H', Drawing No. 04-860-08A
18. ERT INVERSION MODEL CROSS SECTION I-I', Drawing No. 04-860-09
19. INTERPRETATION CROSS SECTION I-I', Drawing No. 04-860-09A
20. 10 m DEPTH LAYER ERT RESIDUALS MAP, Drawing No. 04-860-R1
21. 20 m DEPTH LAYER ERT RESIDUALS MAP, Drawing No. 04-860-R2
22. 40 m DEPTH LAYER ERT RESIDUALS MAP, Drawing No. 04-860-R3
23. 60 m DEPTH LAYER ERT RESIDUALS MAP, Drawing No. 04-860-R4

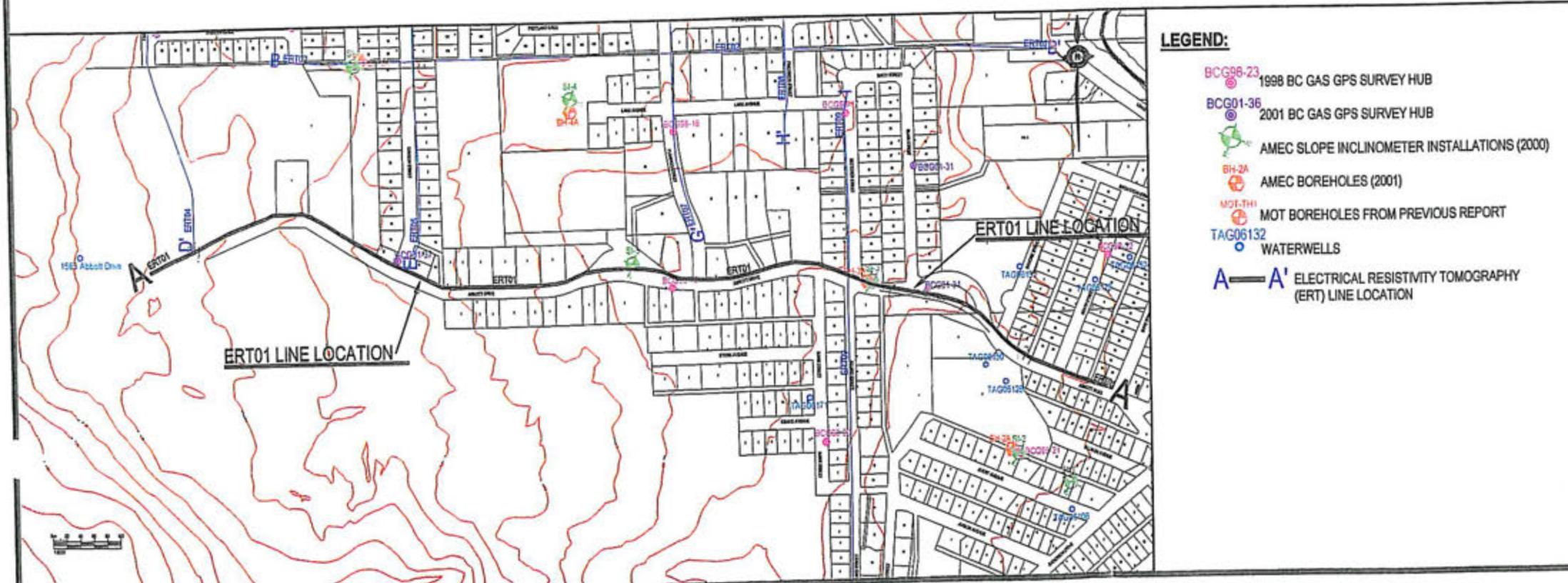
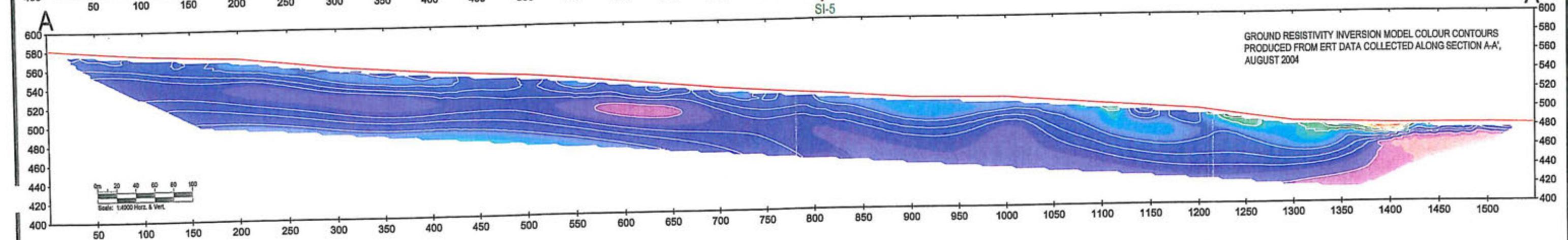
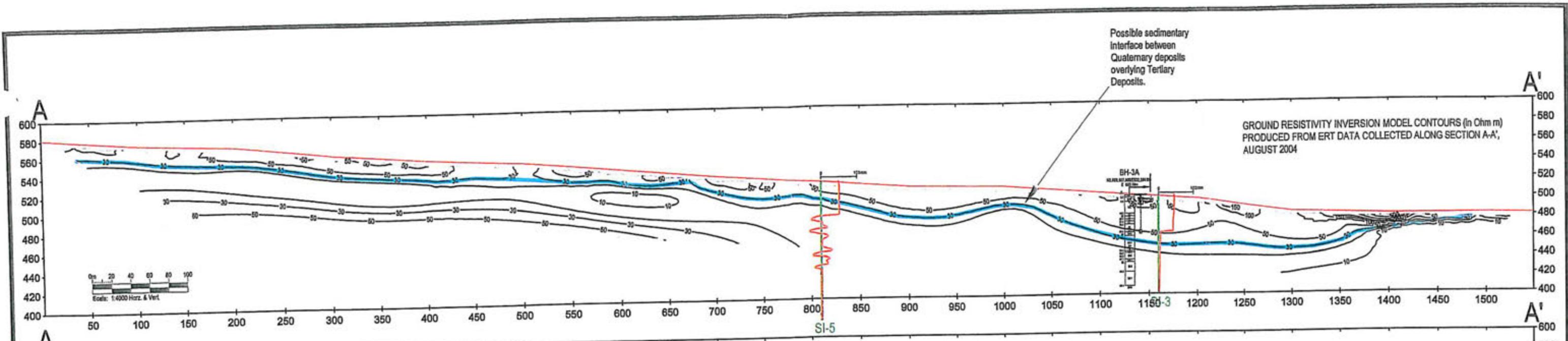
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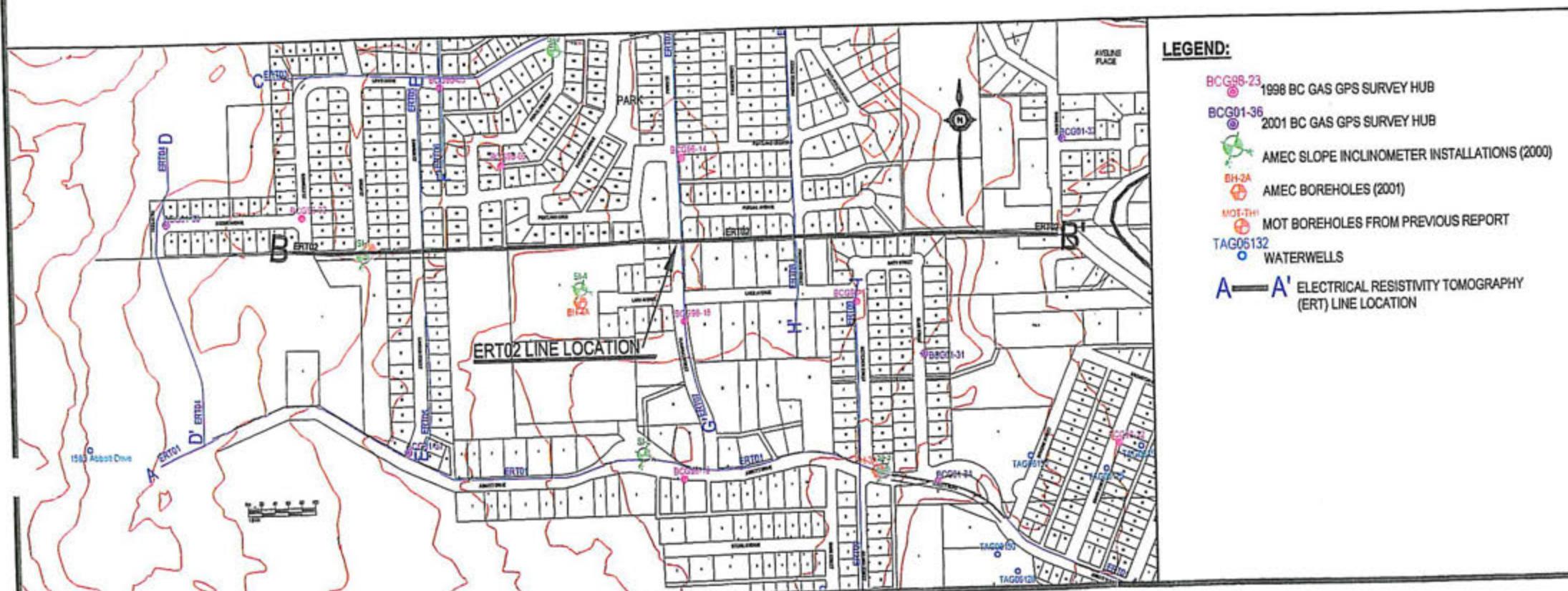
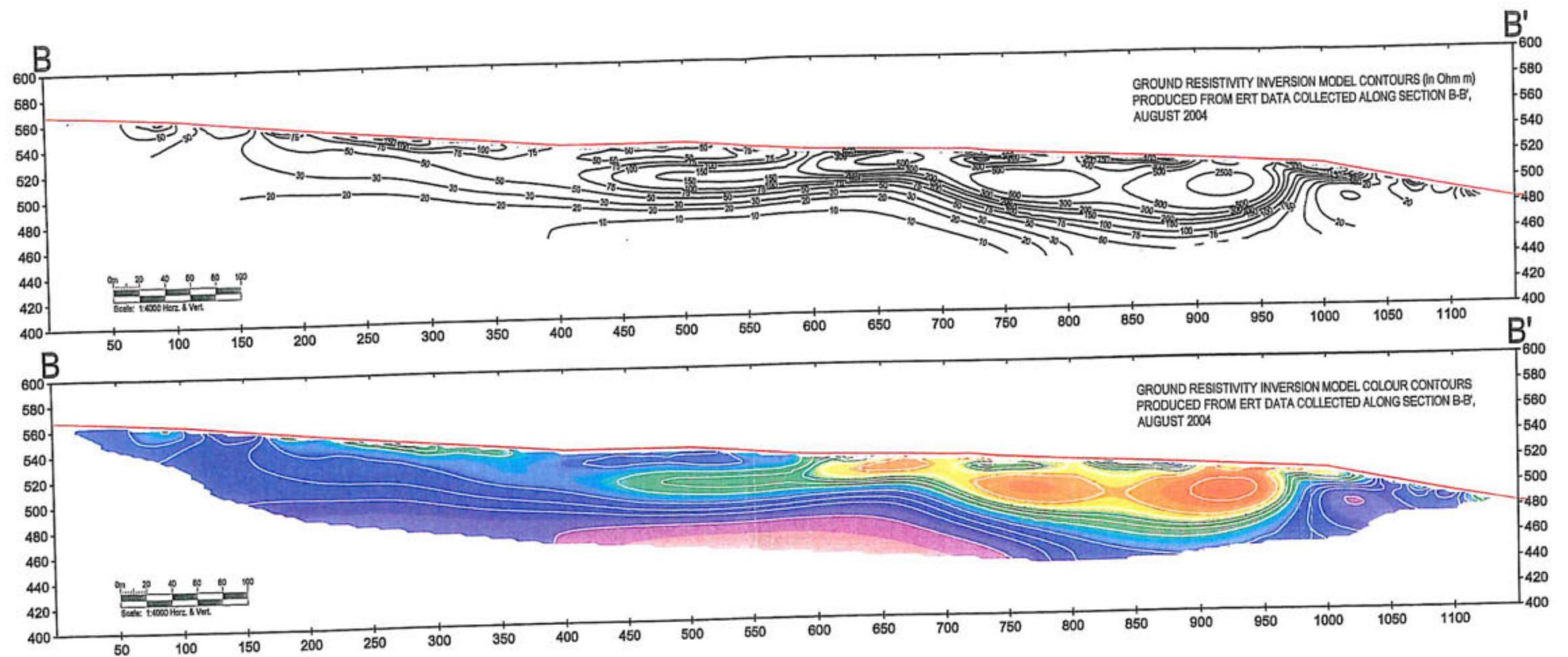
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- BCG01-36 2001 BC GAS GPS SURVEY HUB
- AMEC SLOPE INCLINOMETER INSTALLATIONS (2000)
- BH-2A AMEC BOREHOLES (2001)
- MOT-TH1 MOT BOREHOLES FROM PREVIOUS REPORT
- TAG06132 WATERWELLS
- A—A' ELECTRICAL RESISTIVITY TOMOGRAPHY (ERT) LINE LOCATION

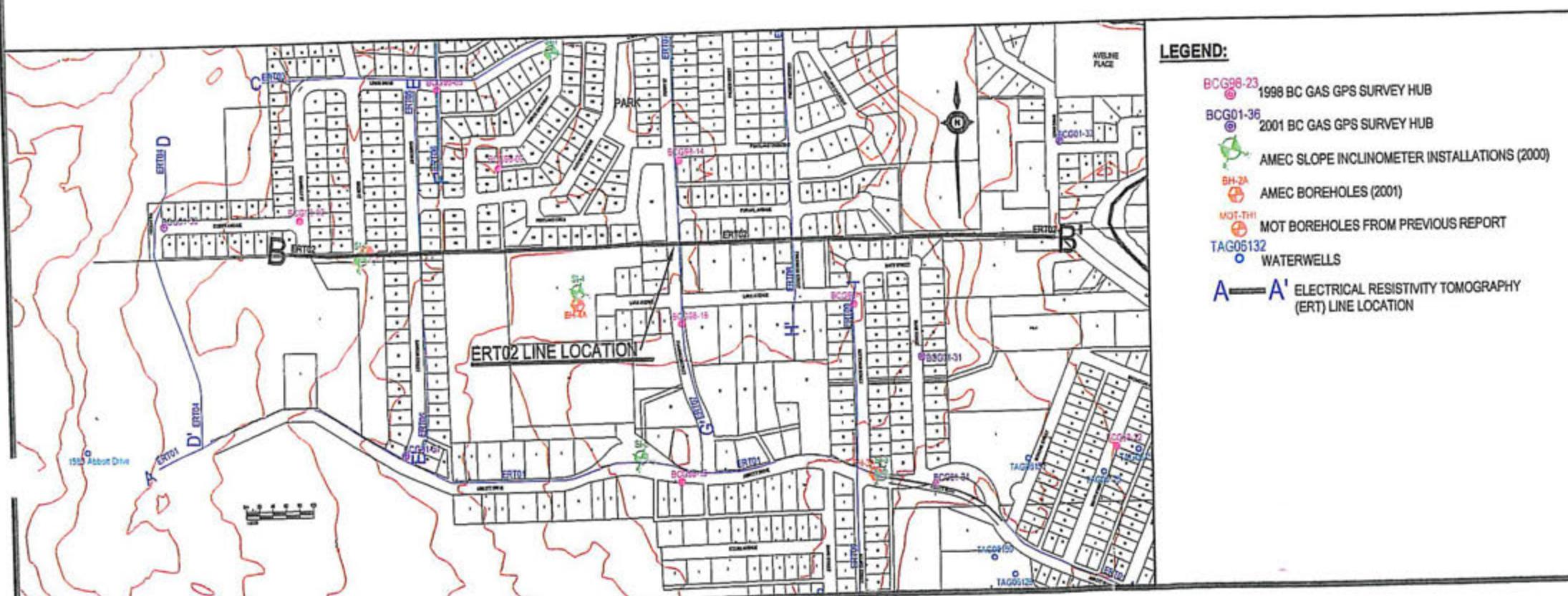
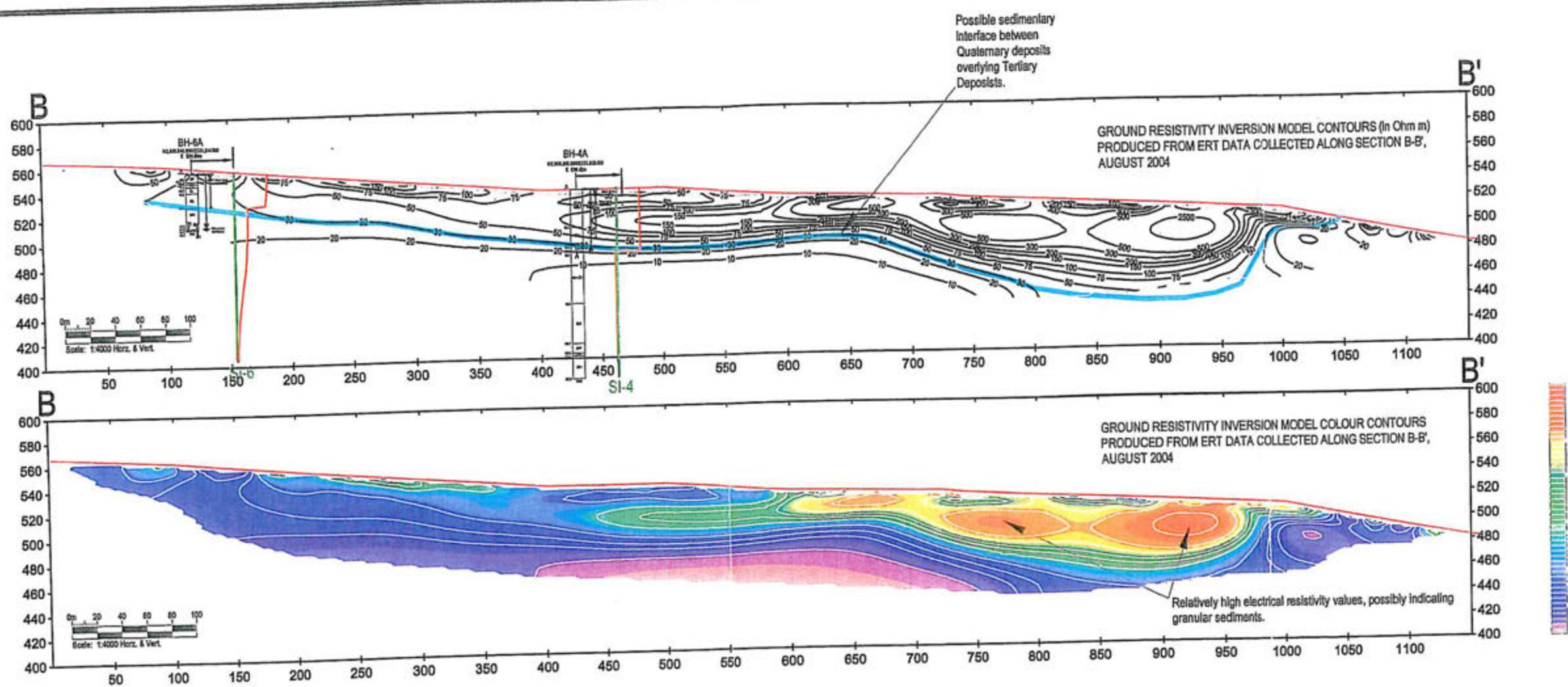


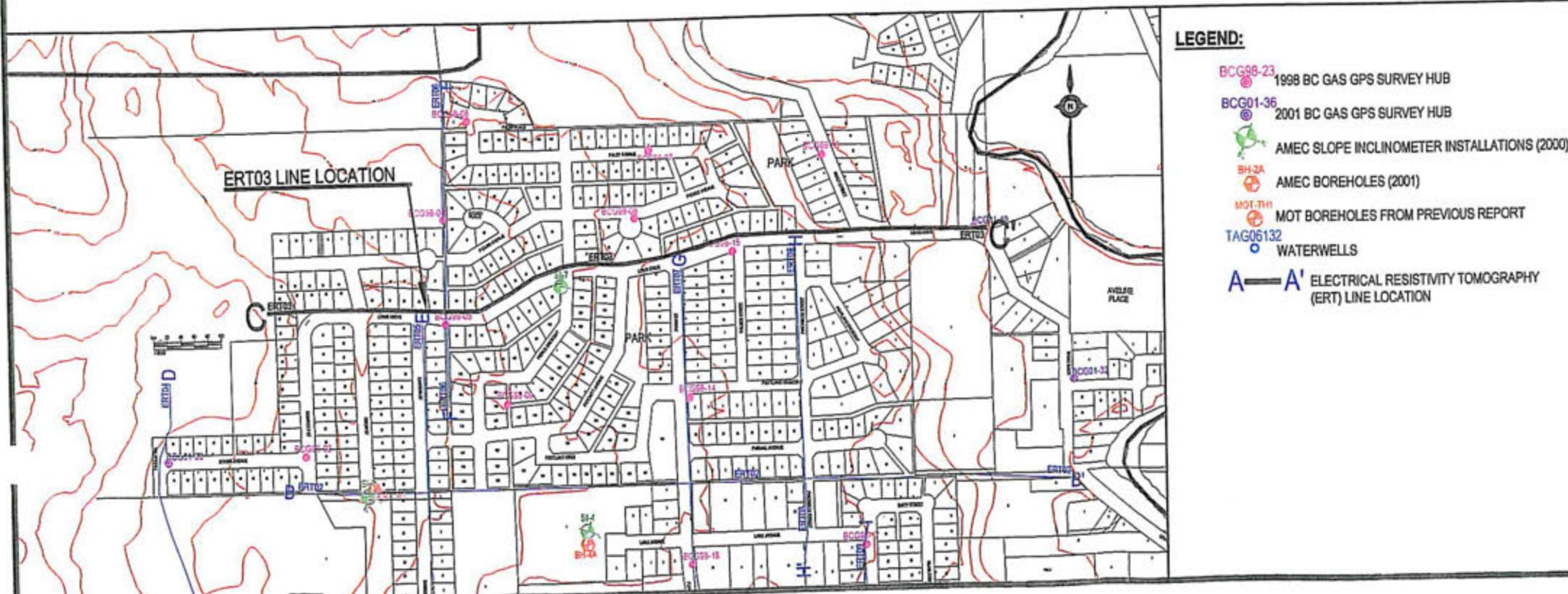
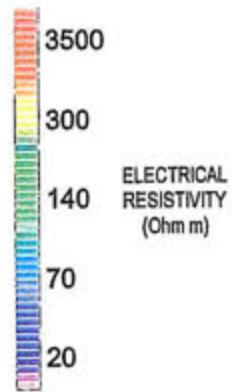
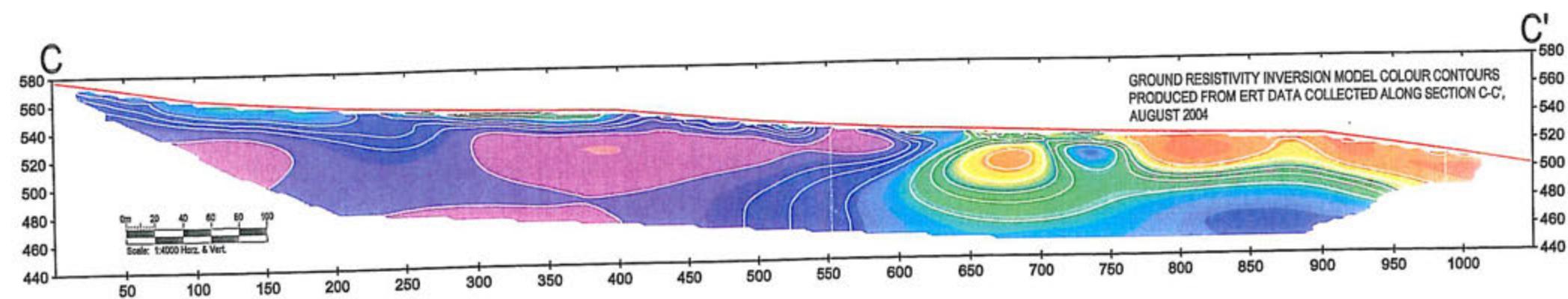
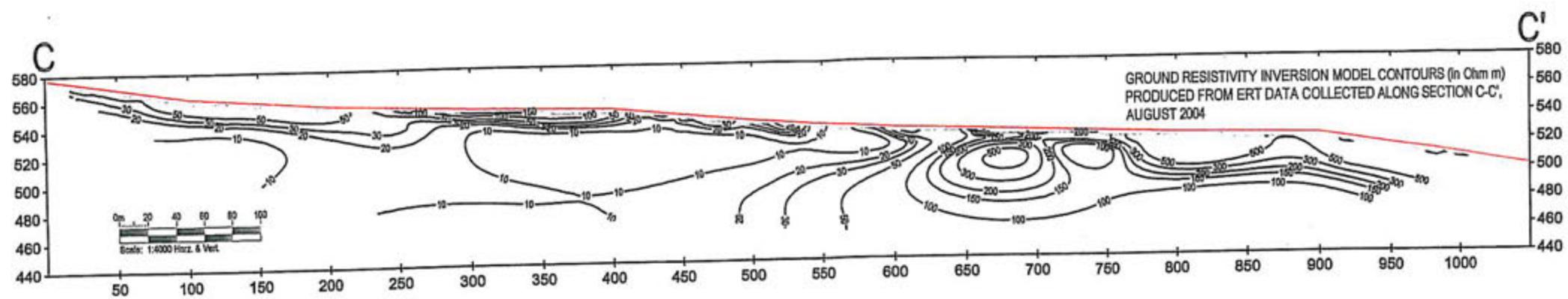
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PROJECT: WEST QUESNEL ELECTRICAL RESISTIVITY TOMOGRAPHY SURVEY	
CLIENT: CITY OF QUESNEL	CONSULTANT/ENGINEER: AMEC Earth & Environmental Ltd.
DRAWING PRODUCED BY: SURFACE SEARCH INC. ENGINEERING & ENVIRONMENTAL GEOPHYSICS 12038 Lake Emerald Cr. SW, Calgary, AB, T2J 2Z5 Ph: (403) 531-6721 Fax: (403) 294-1240 Email: headoffice@surfacesearch.com	
DRAWING SCALE: AS SHOWN	DATE: October 2004
DRAWN BY: PT	JOB NO.: 04-863
DRAWING NO.: 04-860-00	REV. NO.: 01

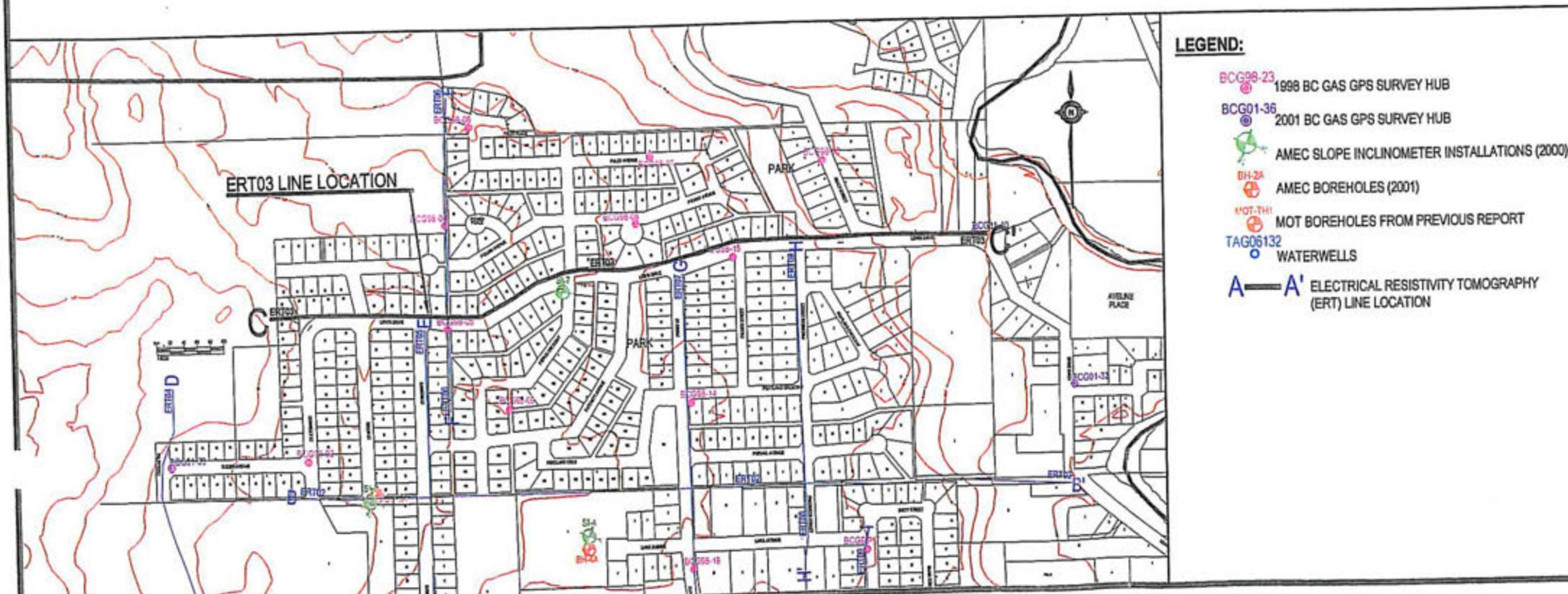
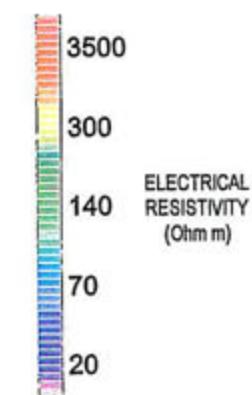
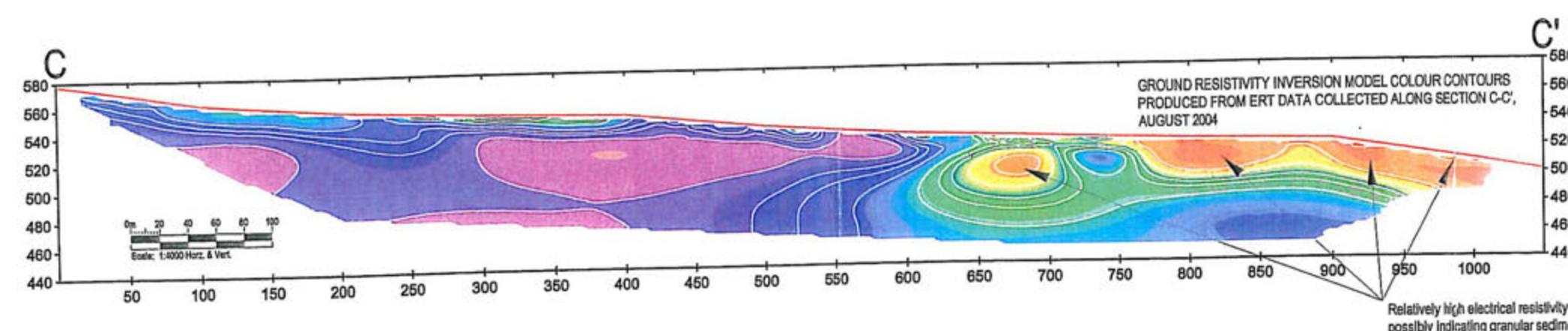
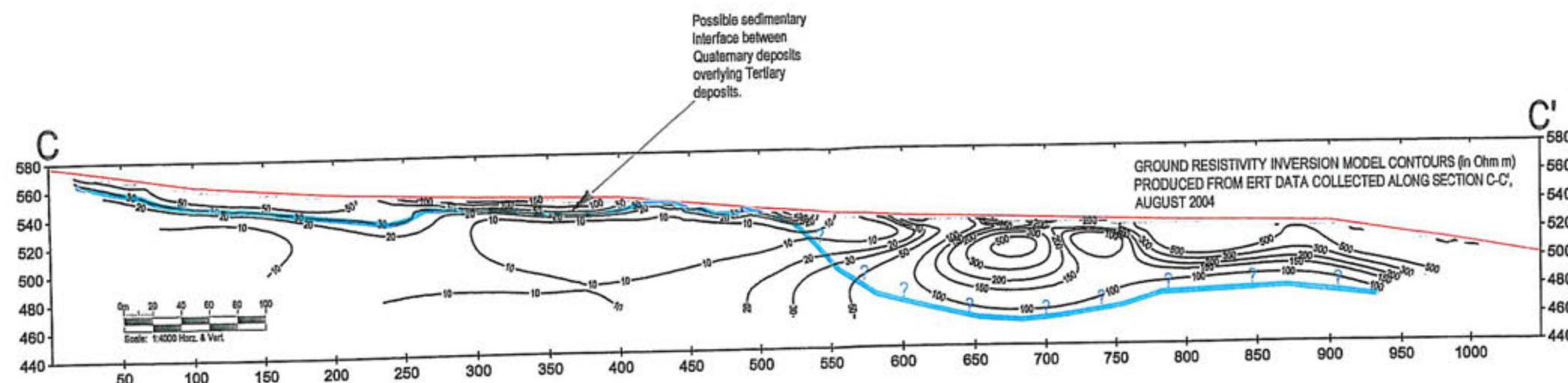


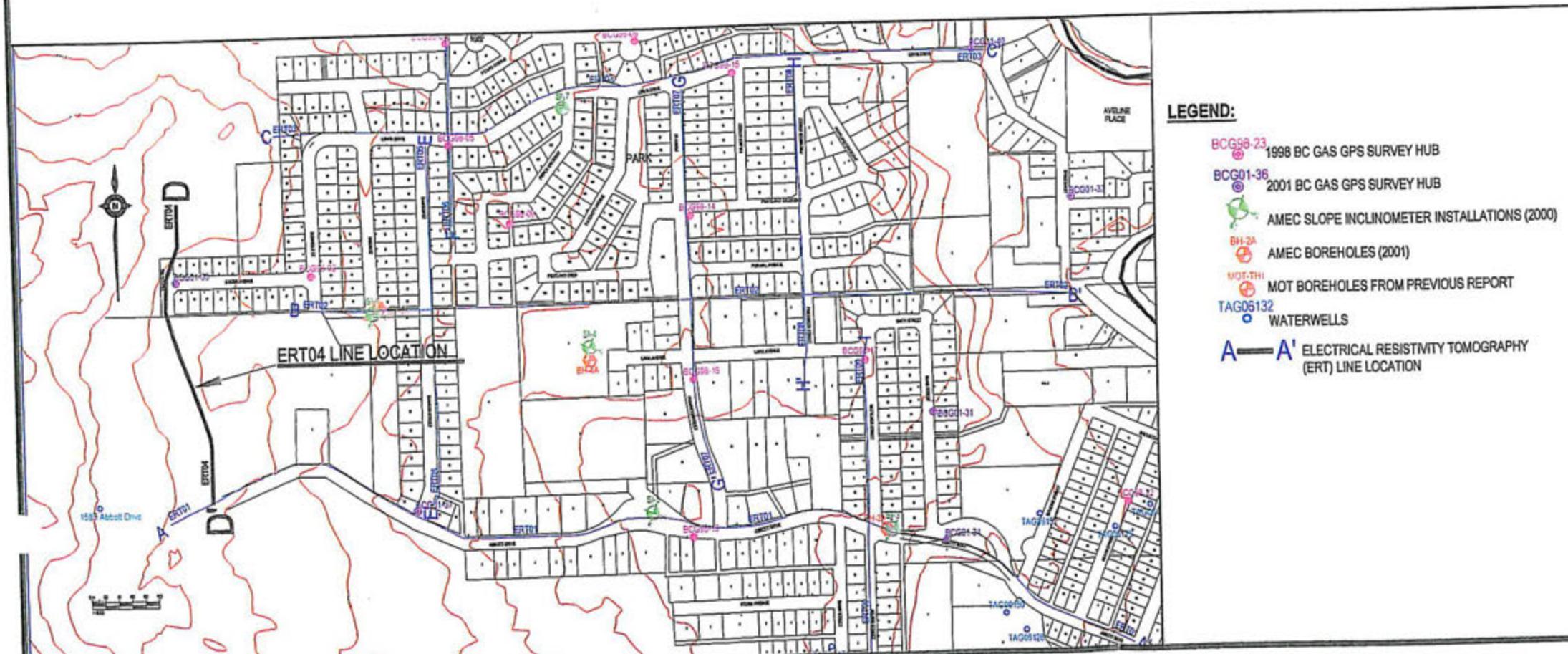
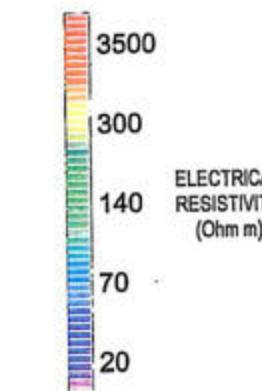
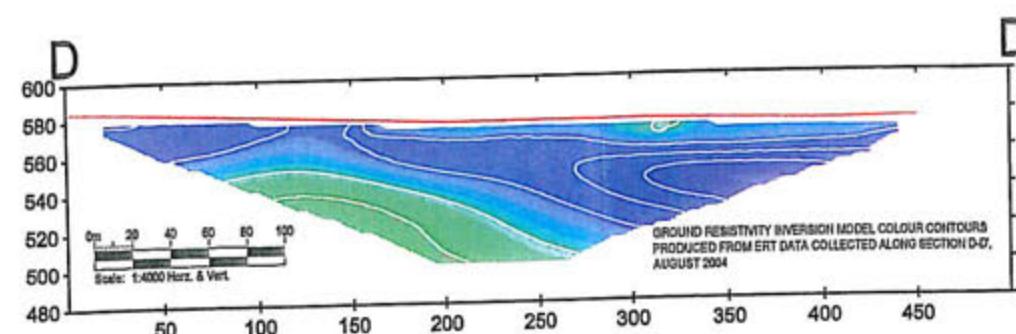
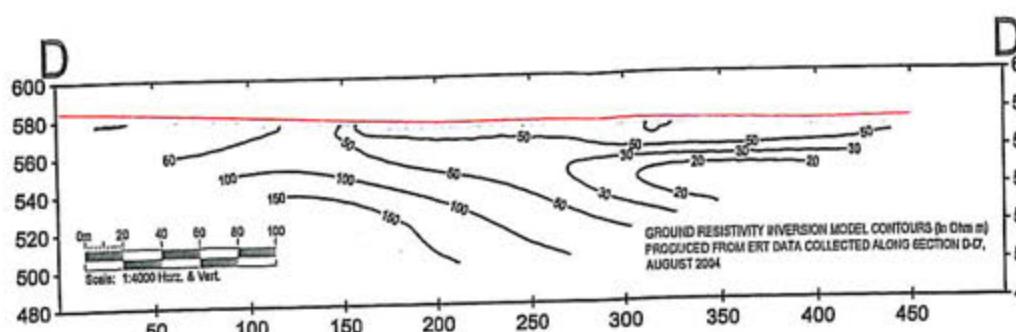


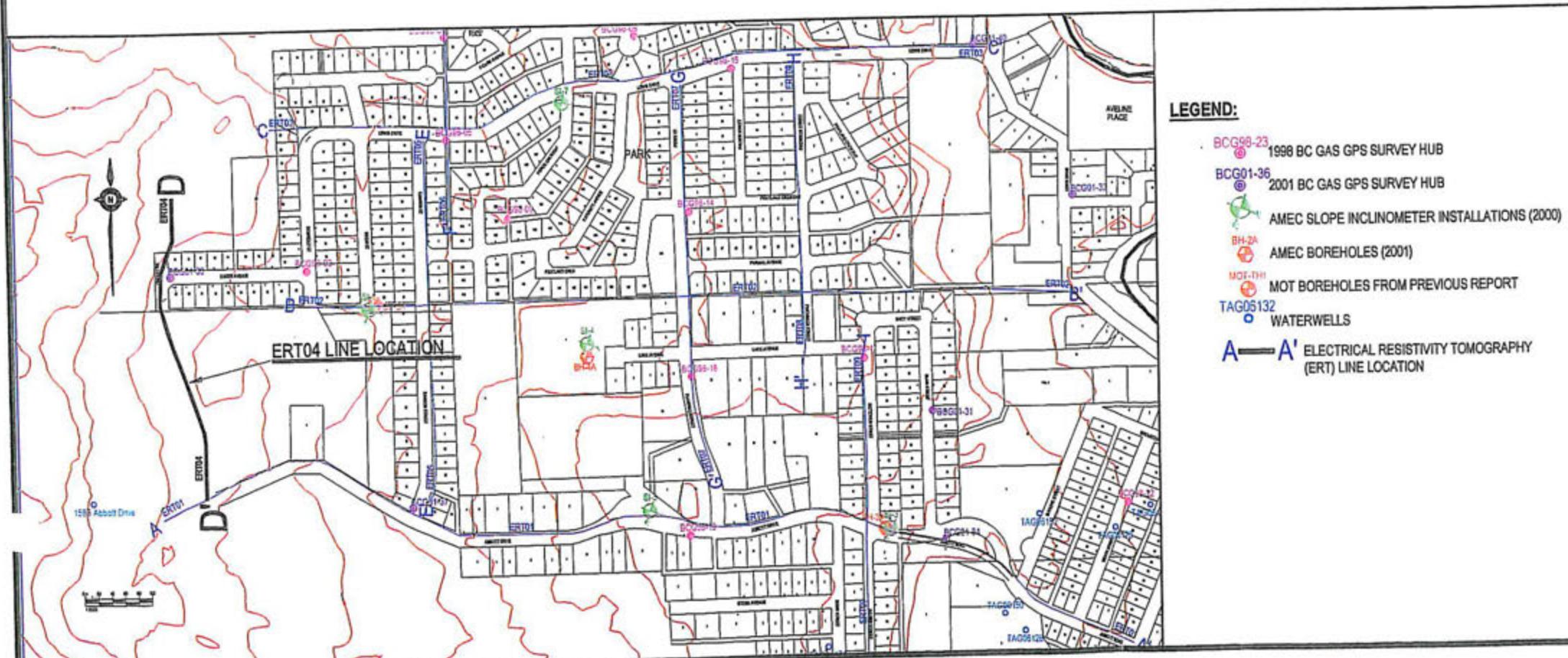
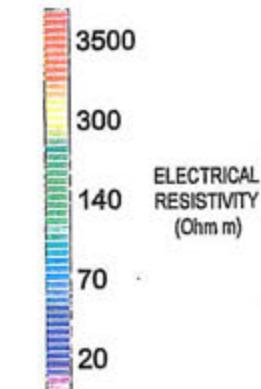
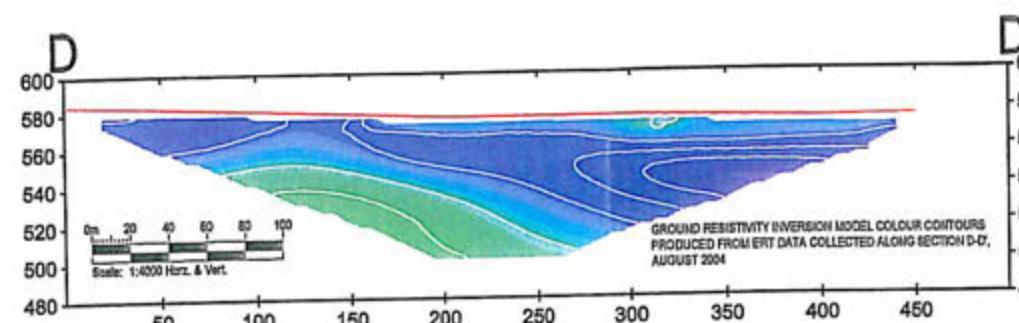
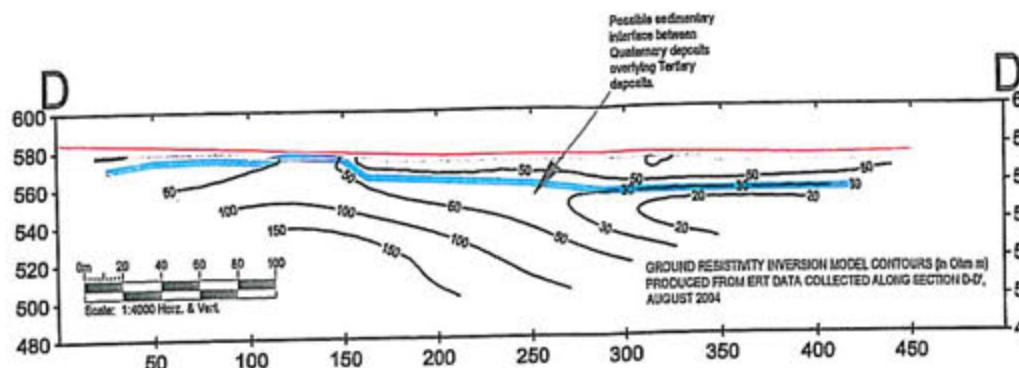


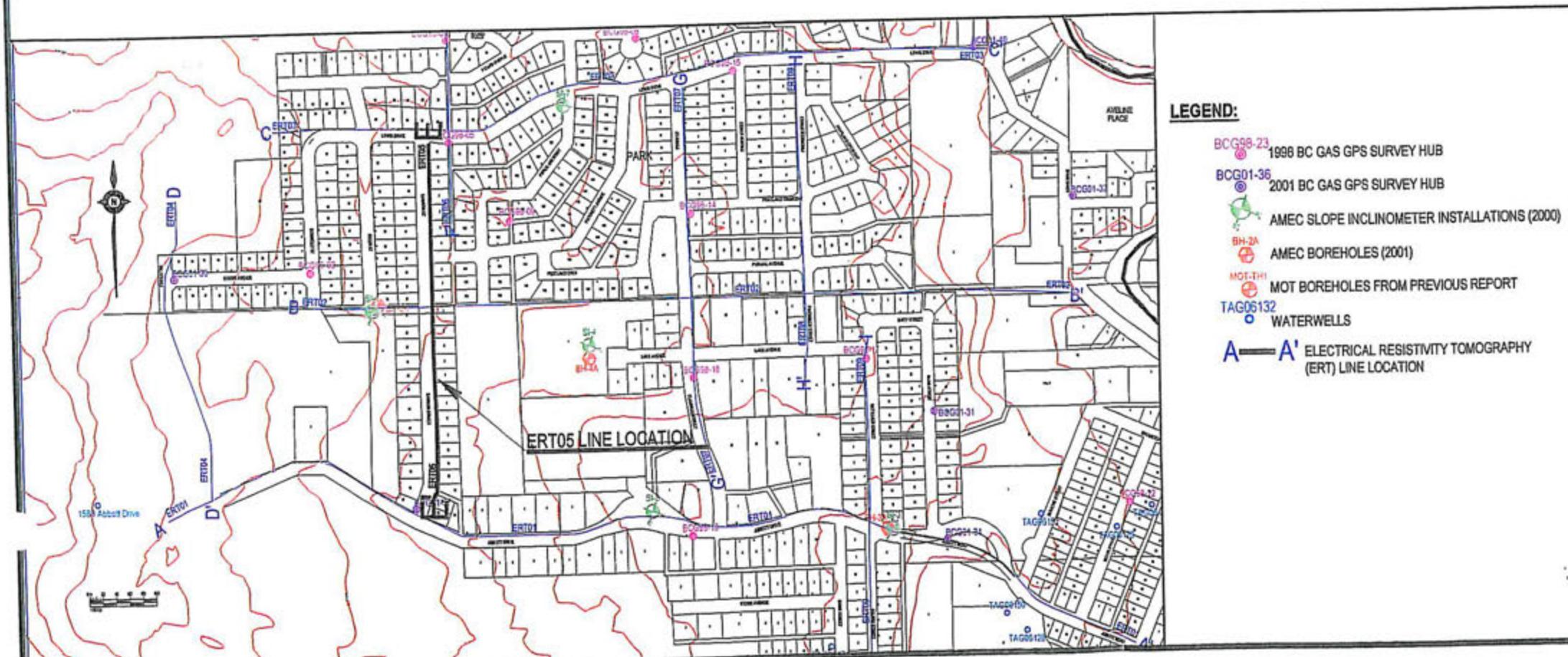
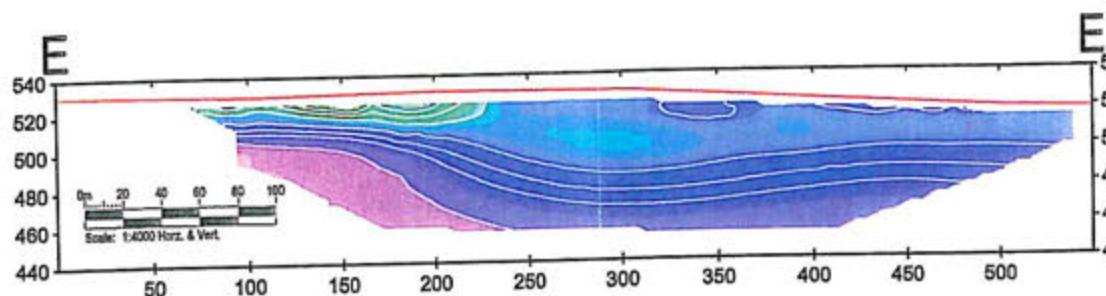
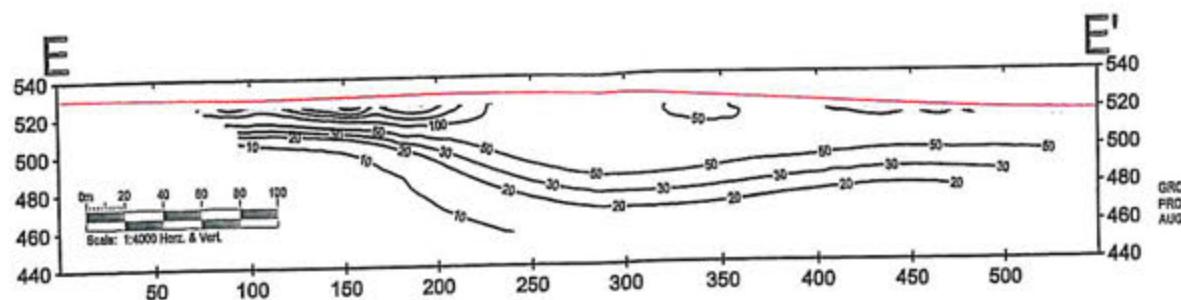


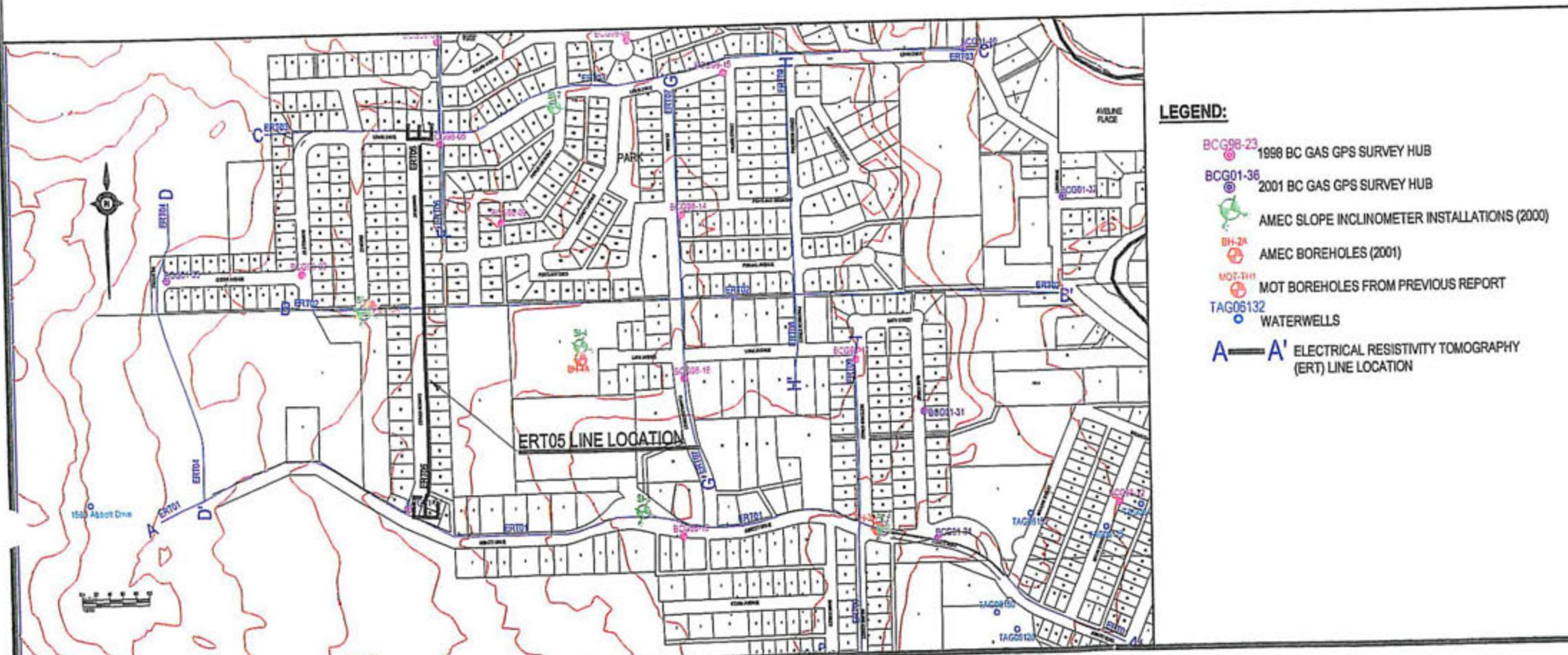
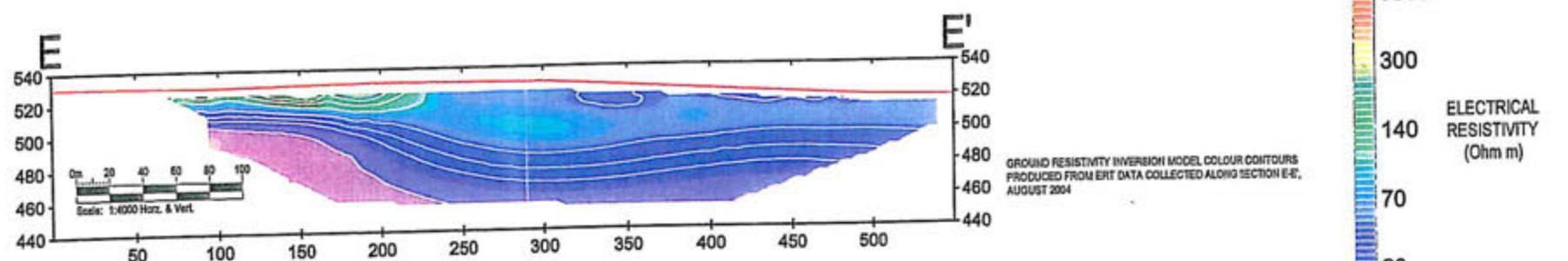
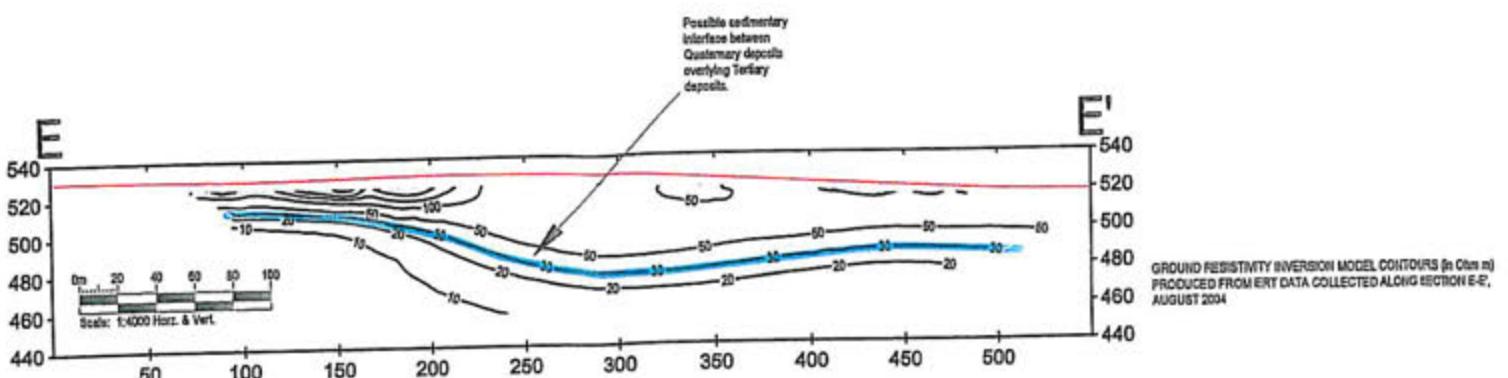


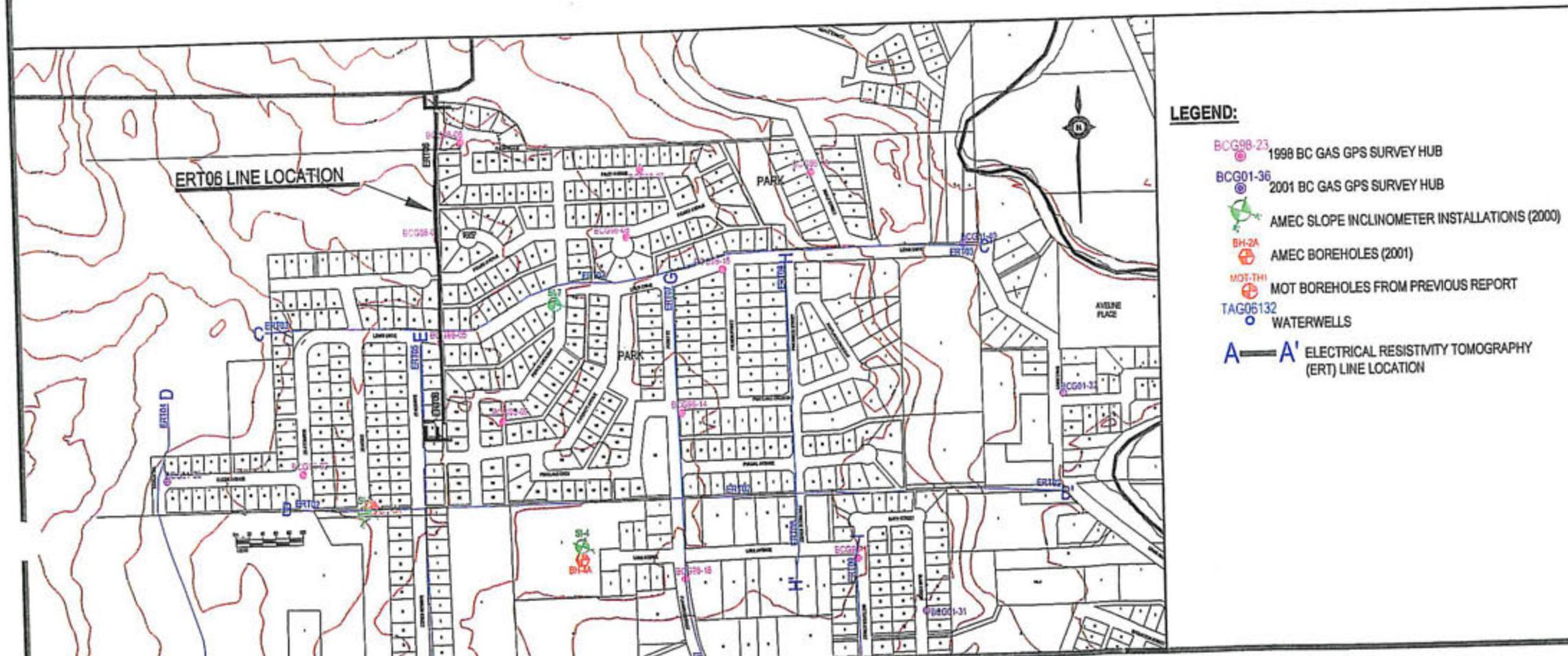
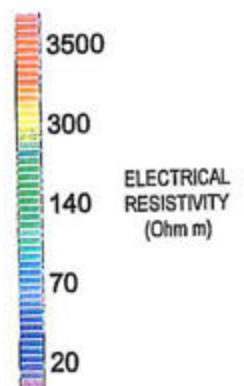
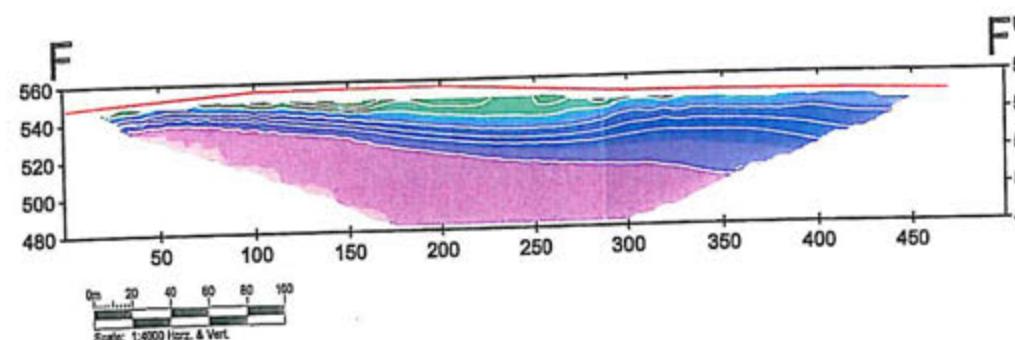
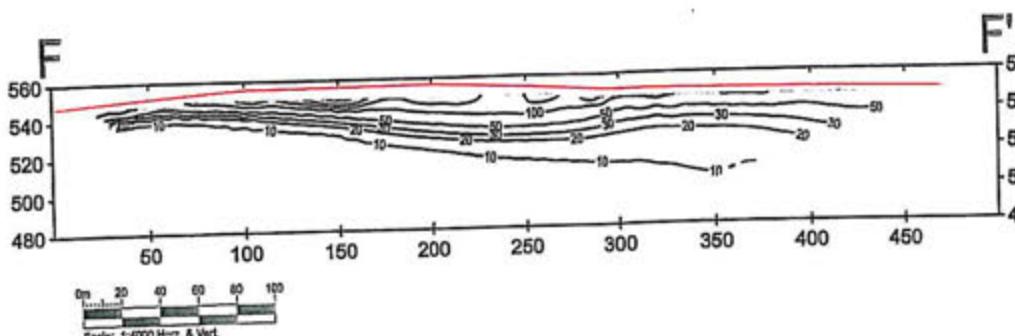


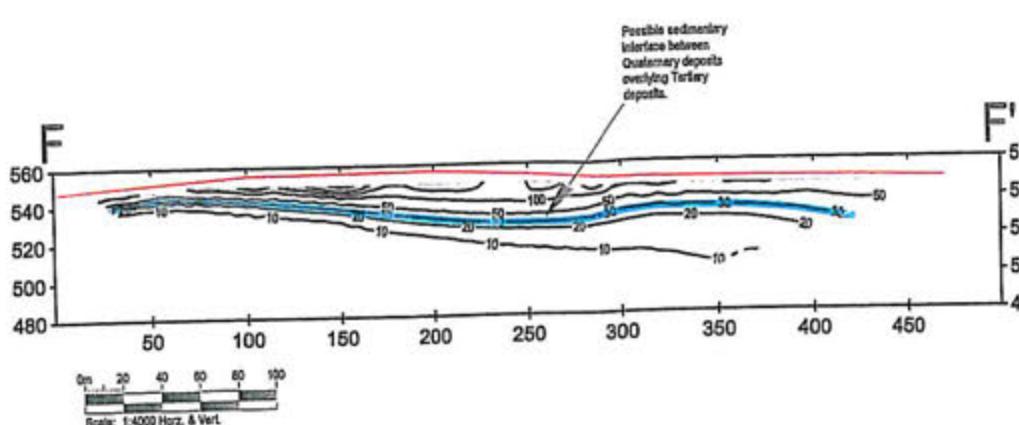




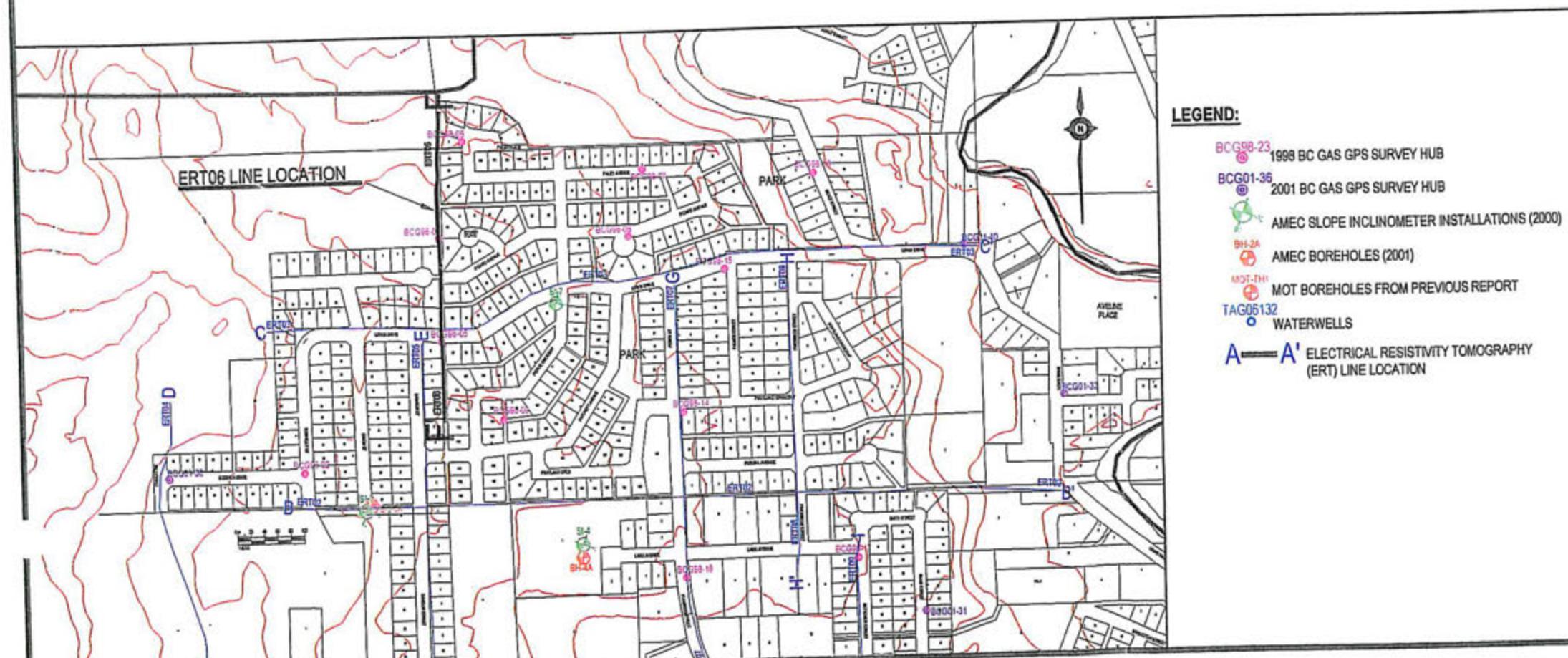
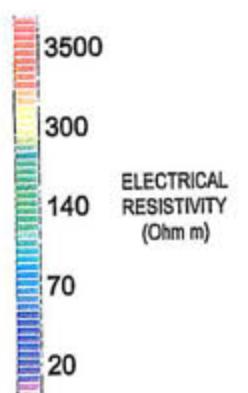
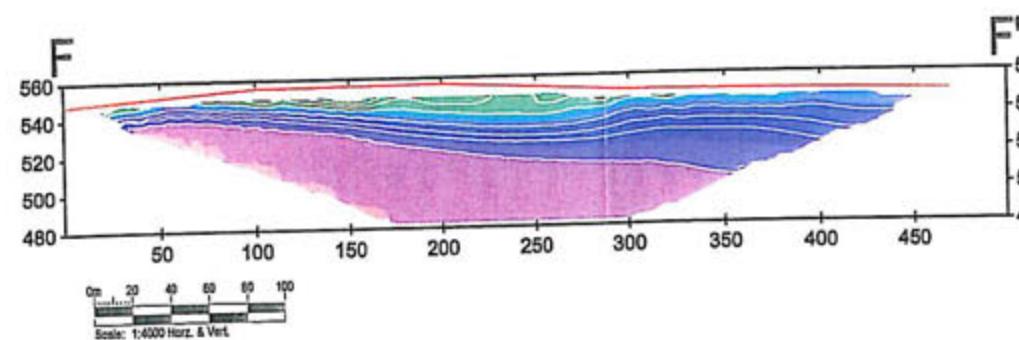


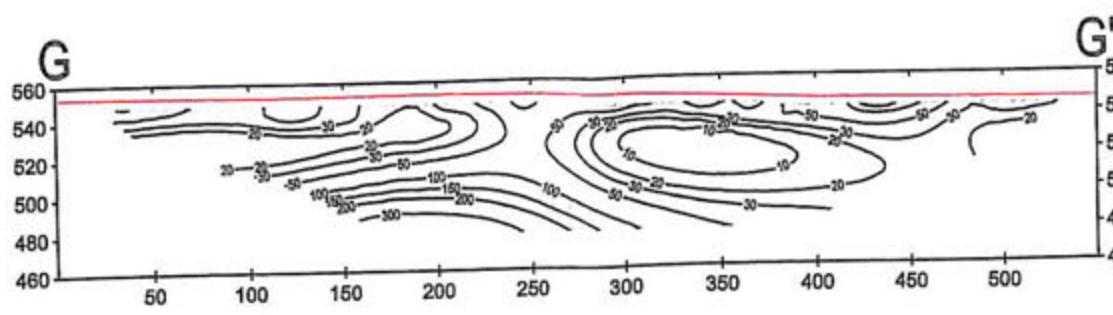




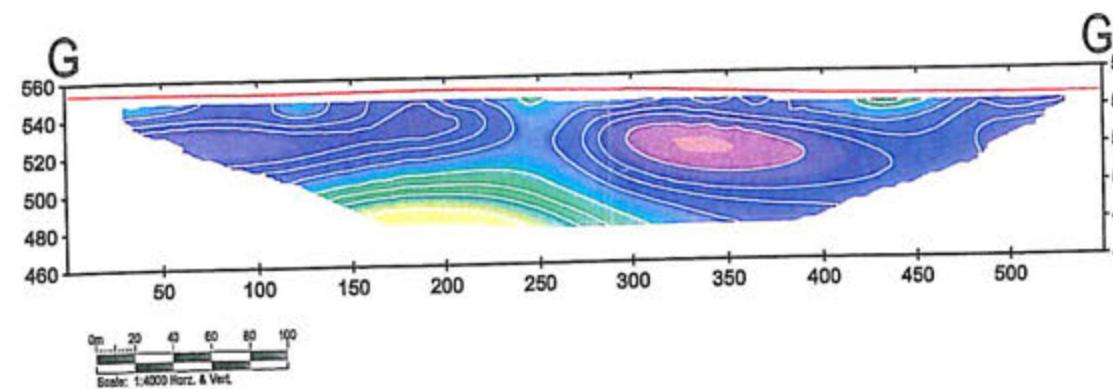


GROUND RESISTIVITY INVERSION MODEL CONTOURS (in Ohm m)
PRODUCED FROM ERT DATA COLLECTED ALONG SECTION F-F'.
AUGUST 2004

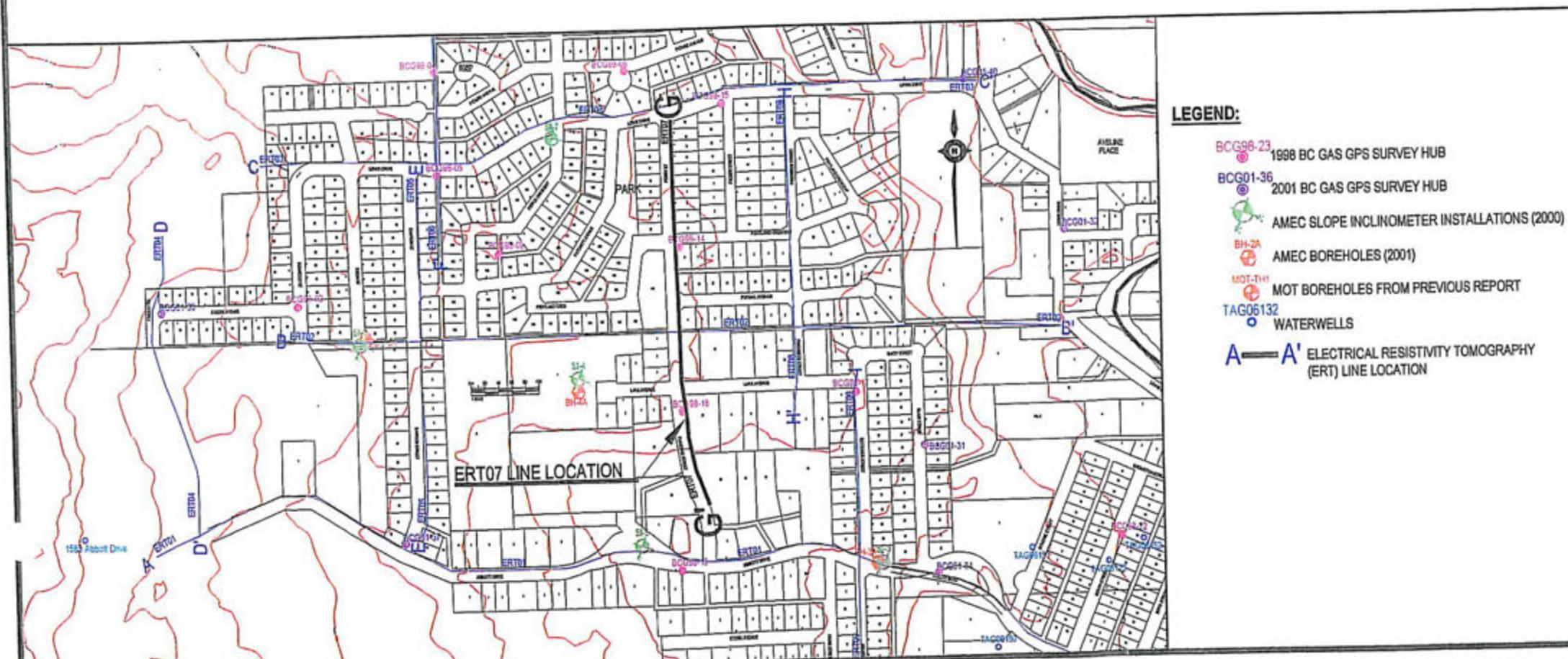
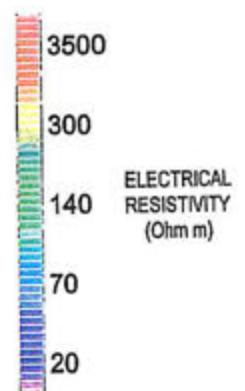


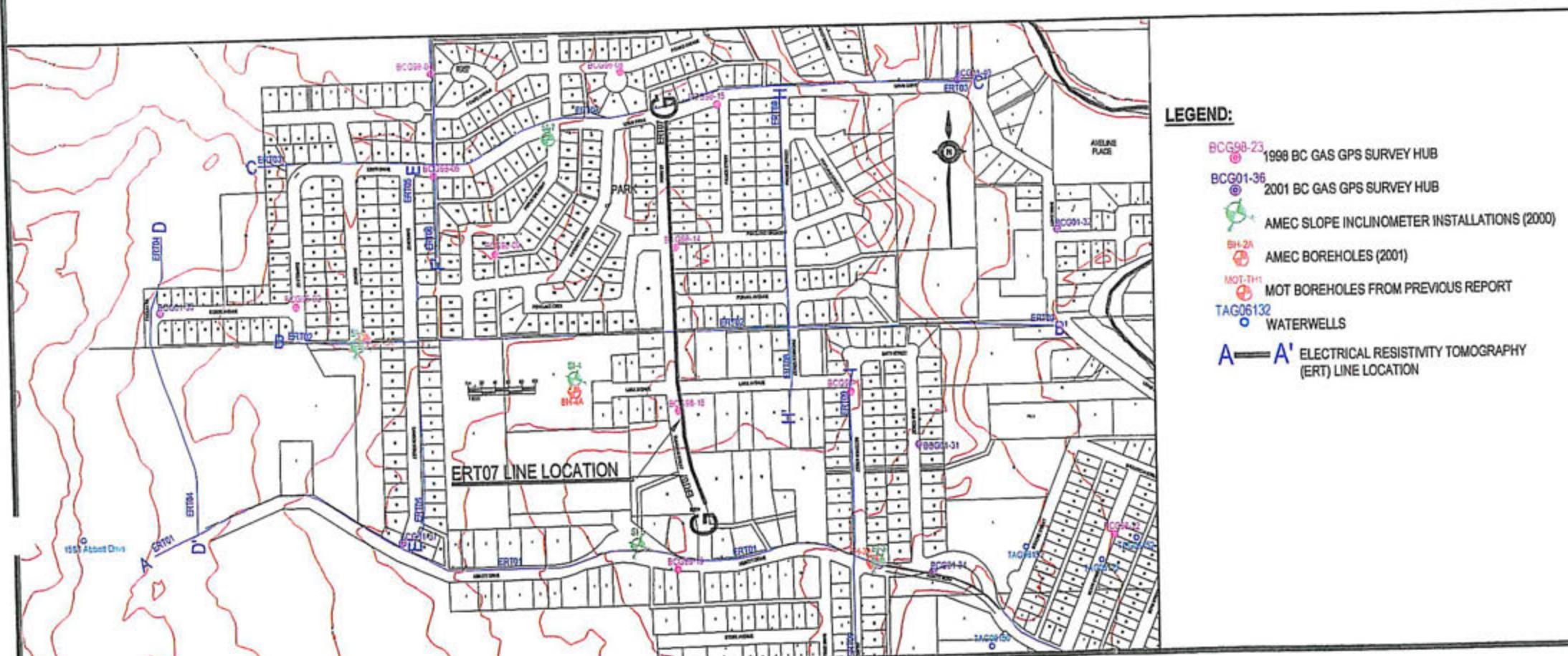
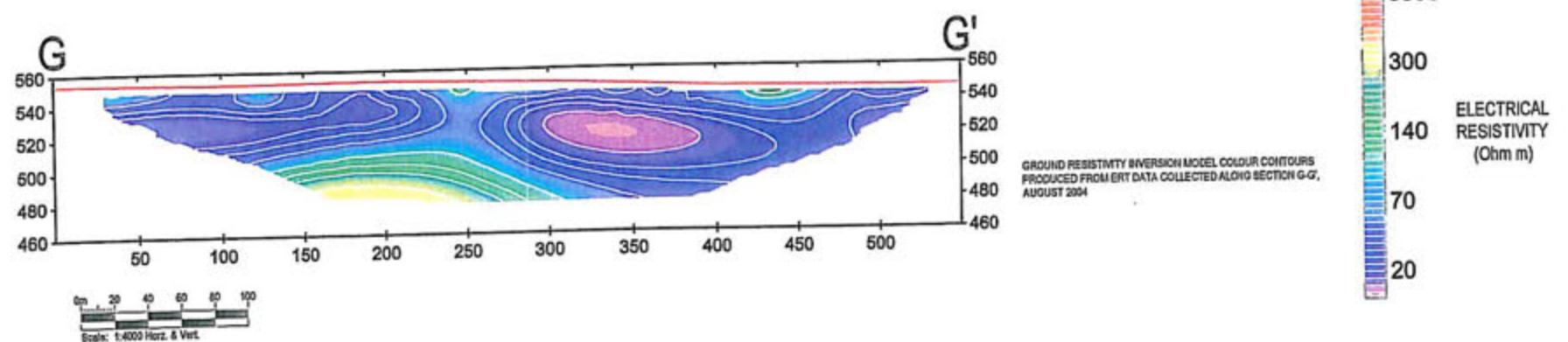
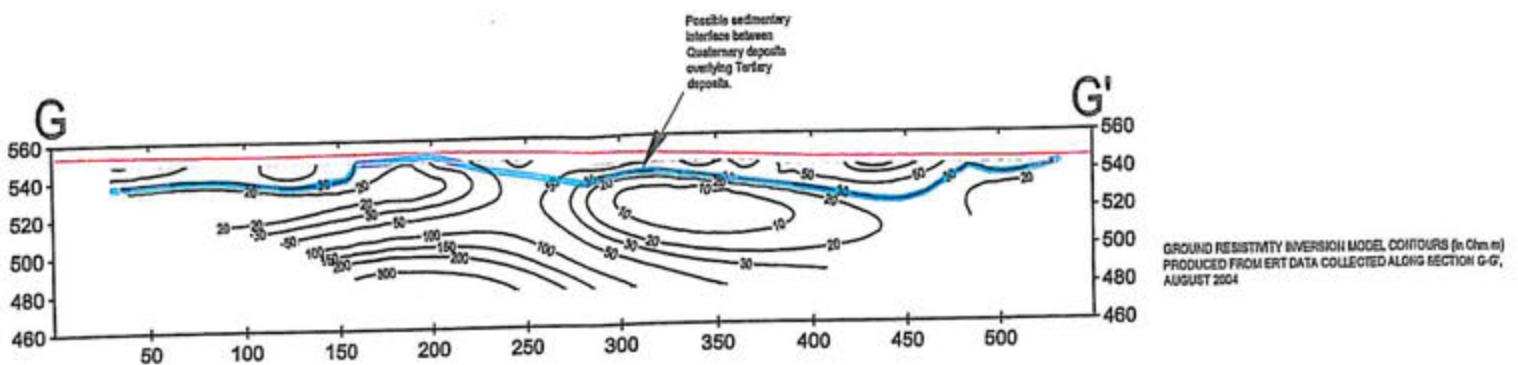


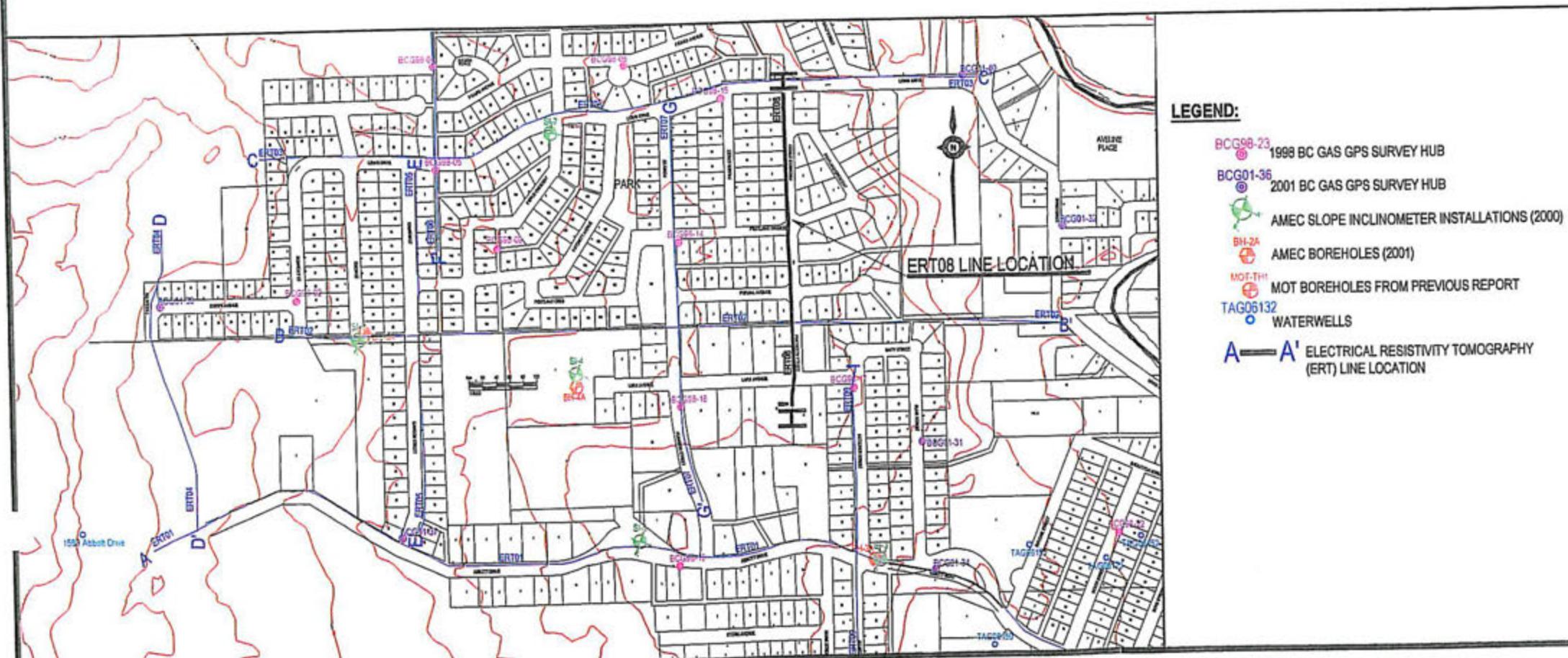
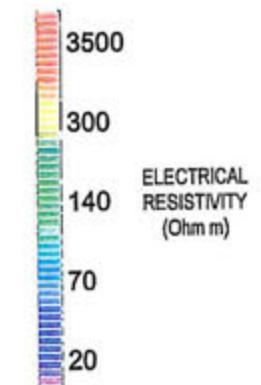
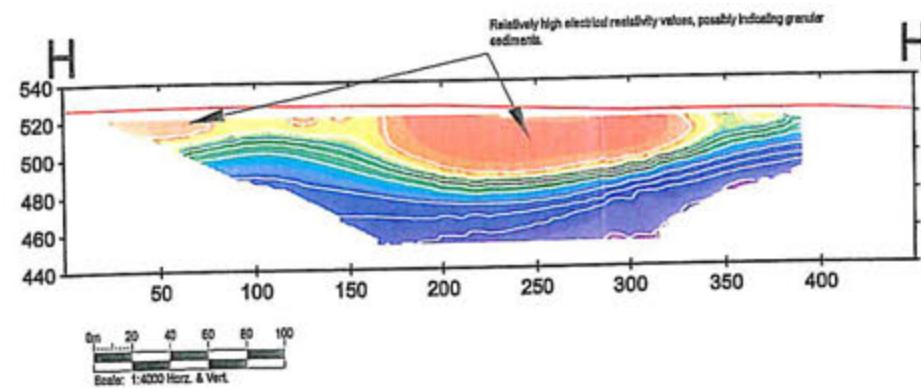
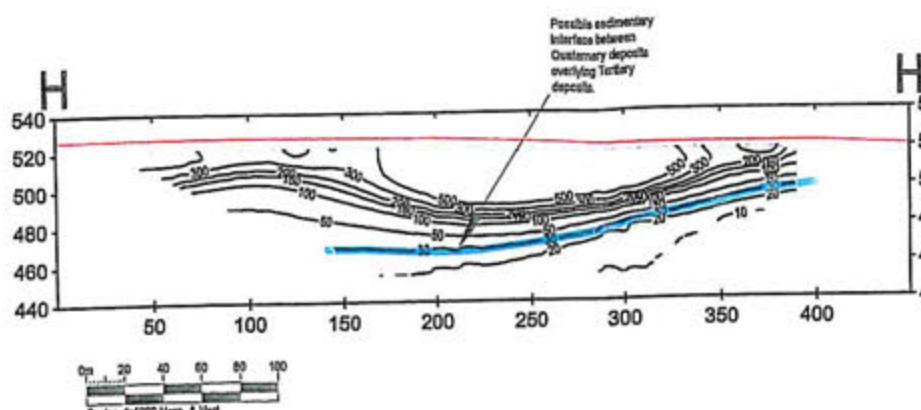
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PRODUCED FROM ERT DATA COLLECTED ALONG SECTION G-G',
AUGUST 2004

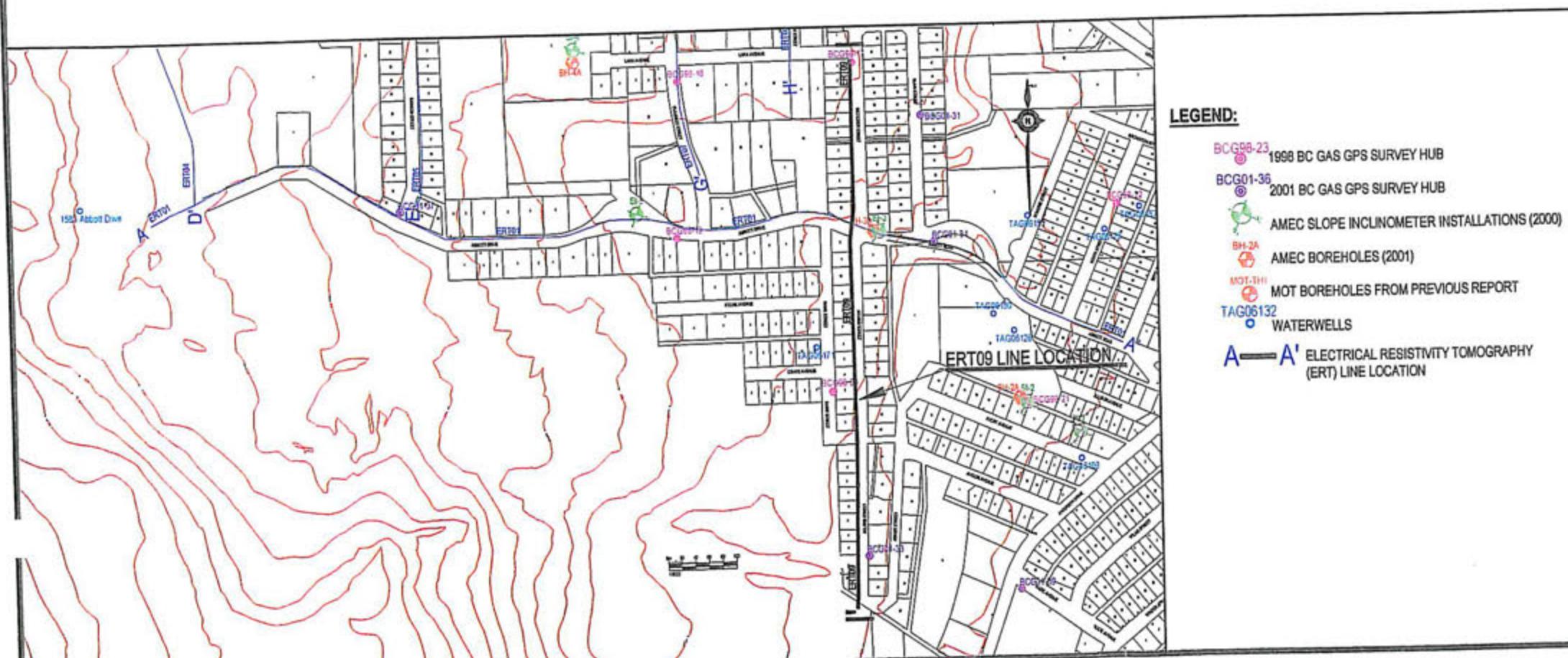
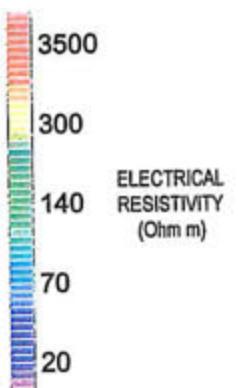
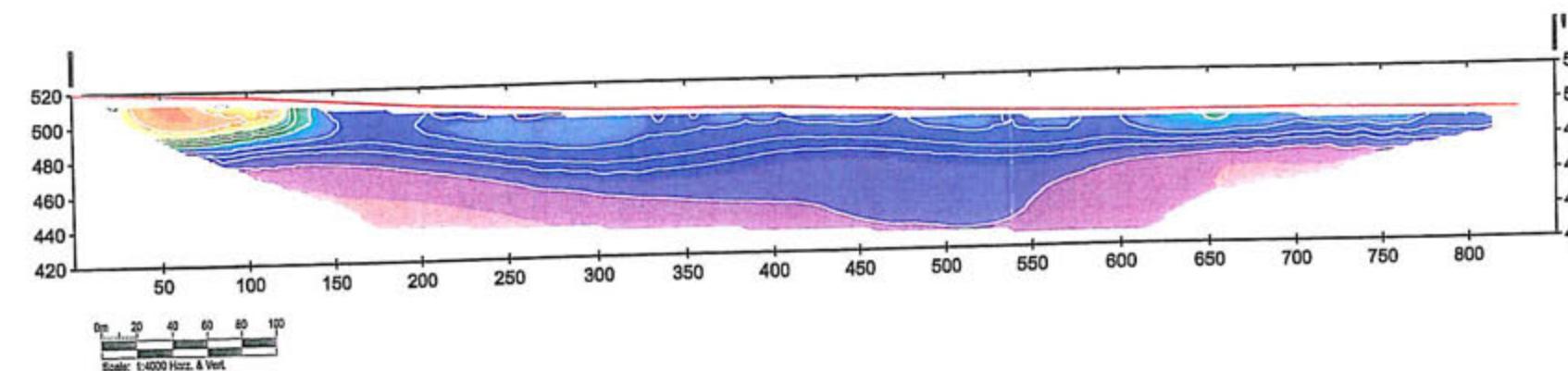
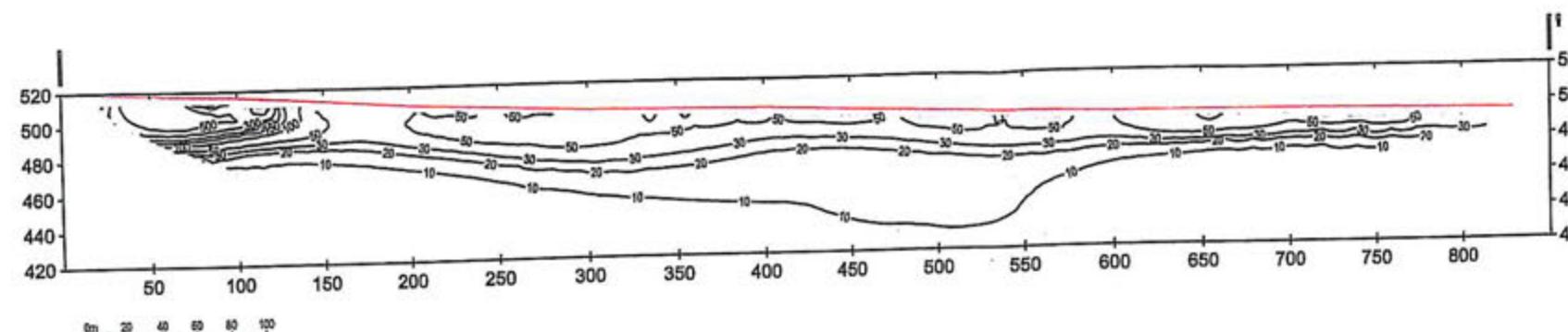


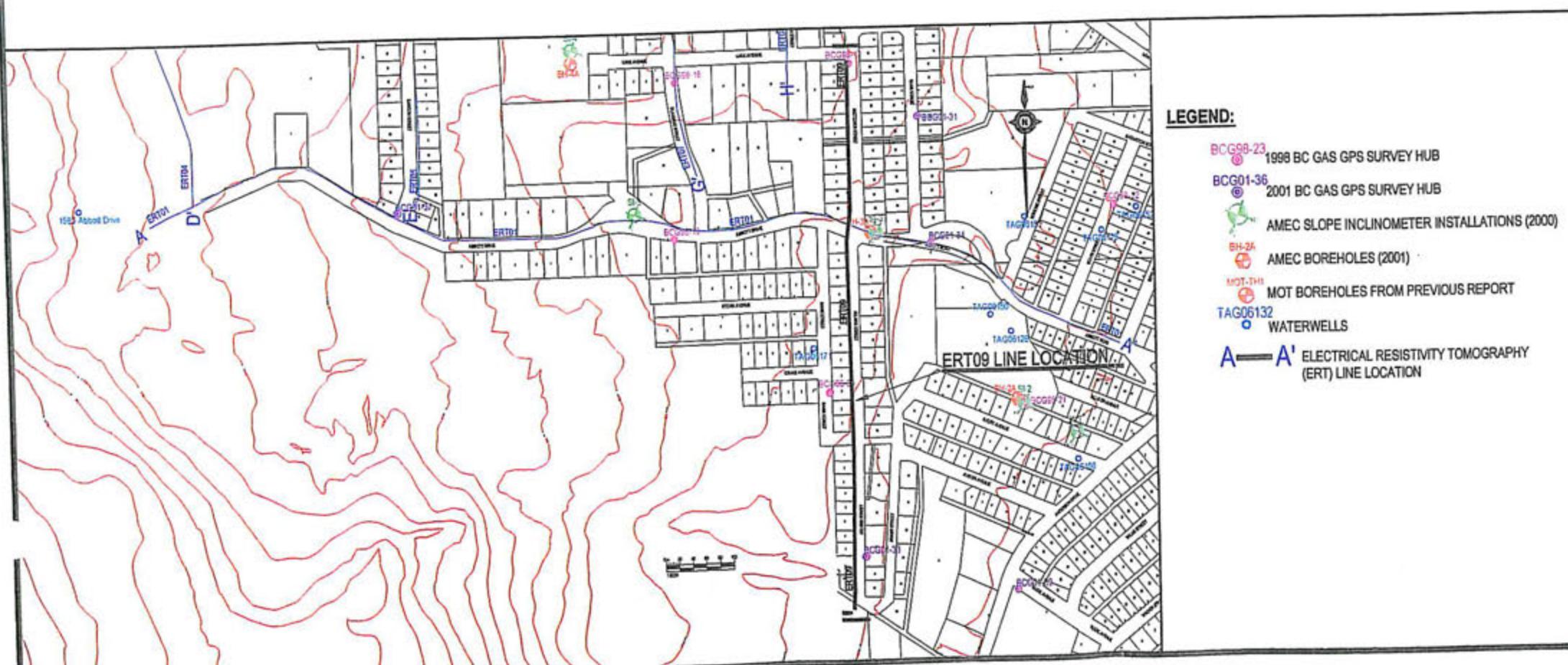
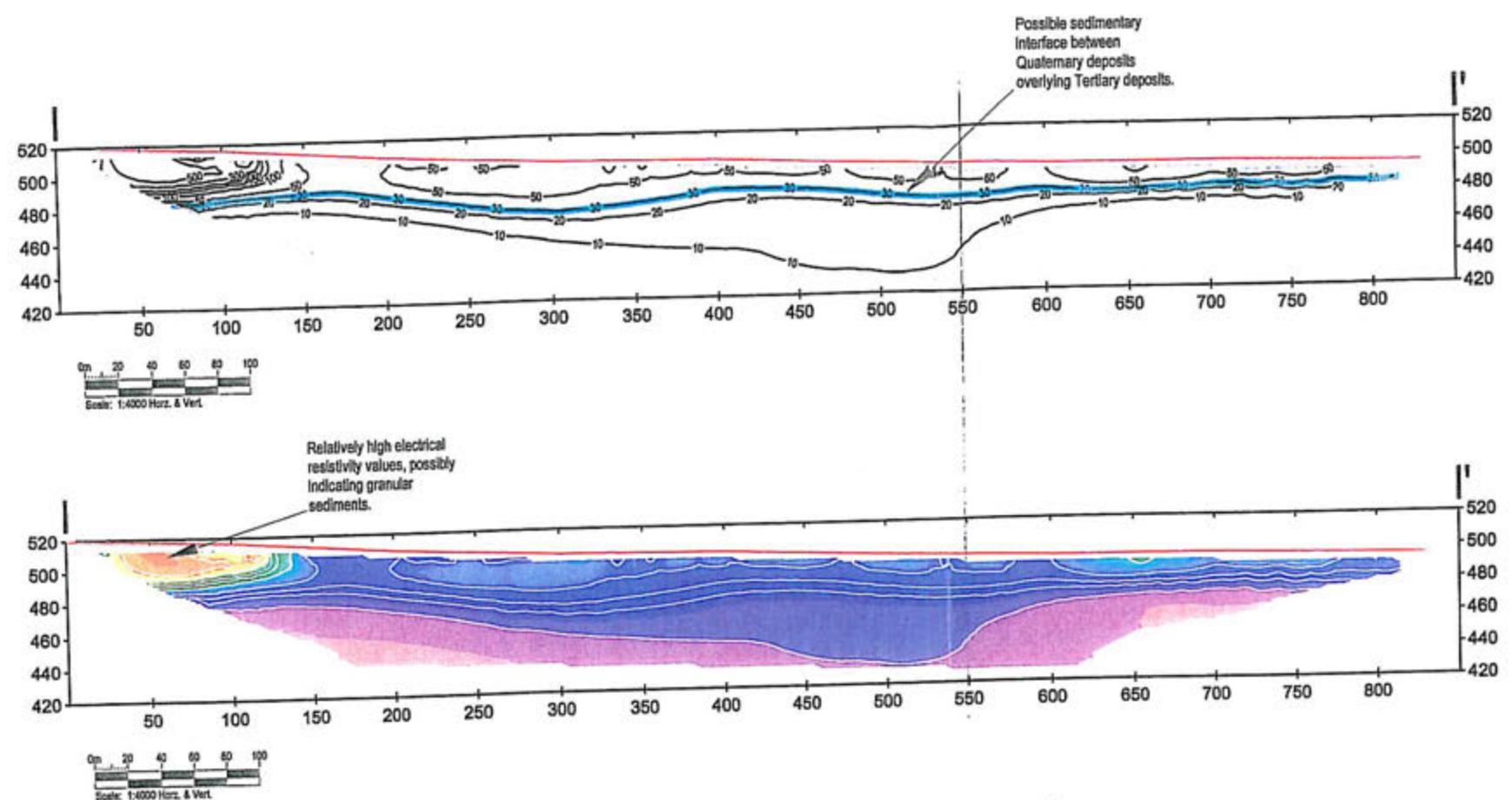
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PRODUCED FROM ERT DATA COLLECTED ALONG SECTION G-G',
AUGUST 2004

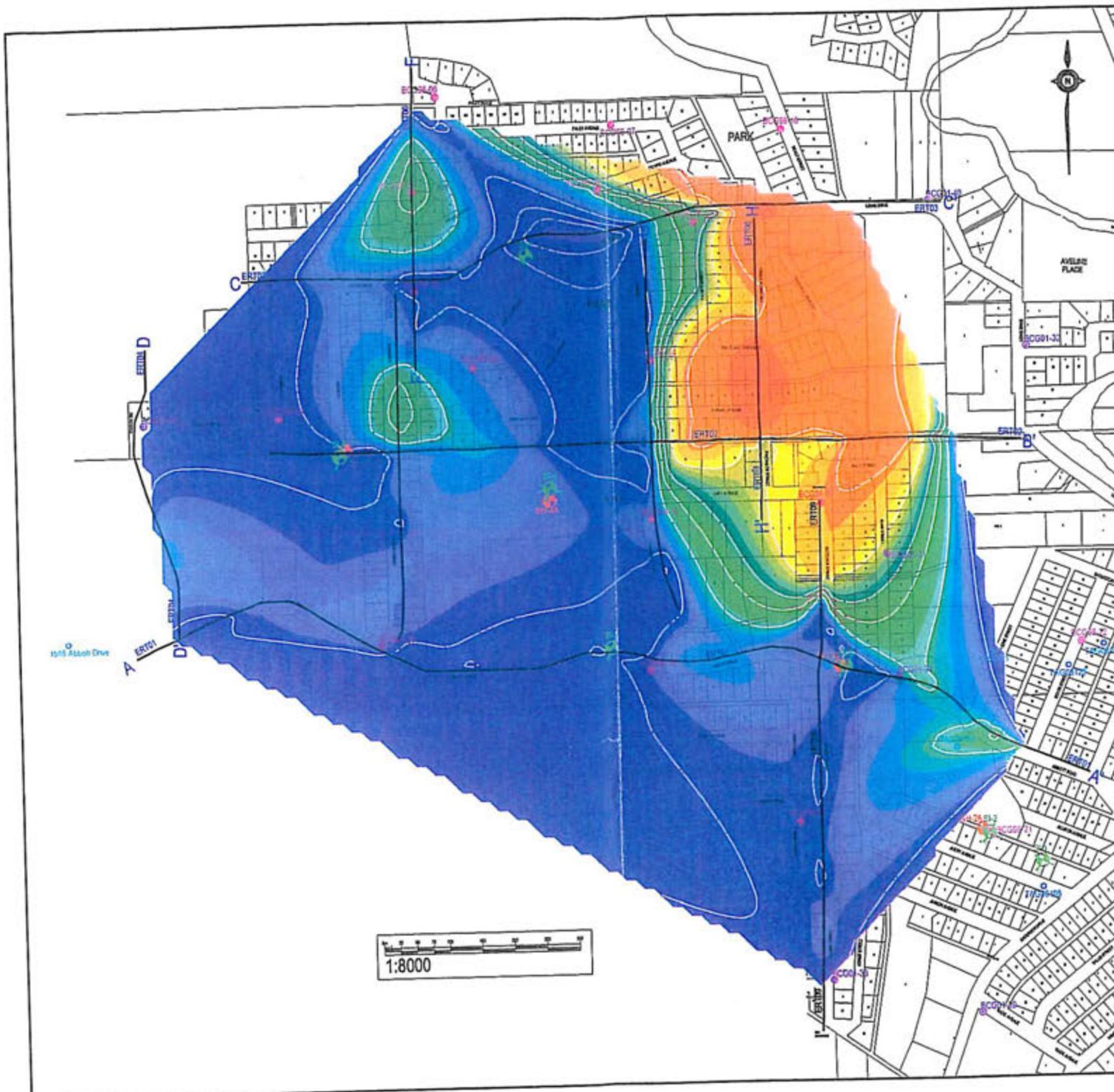
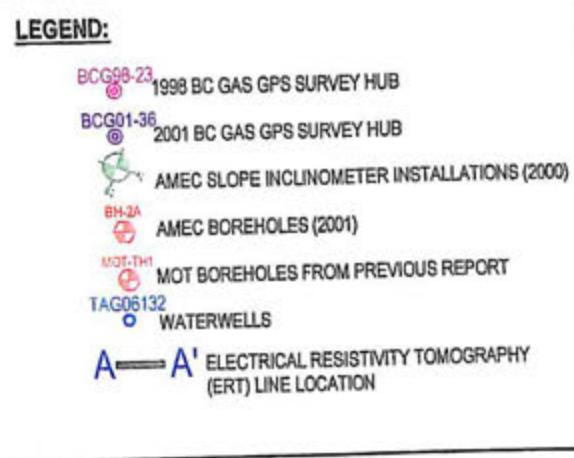




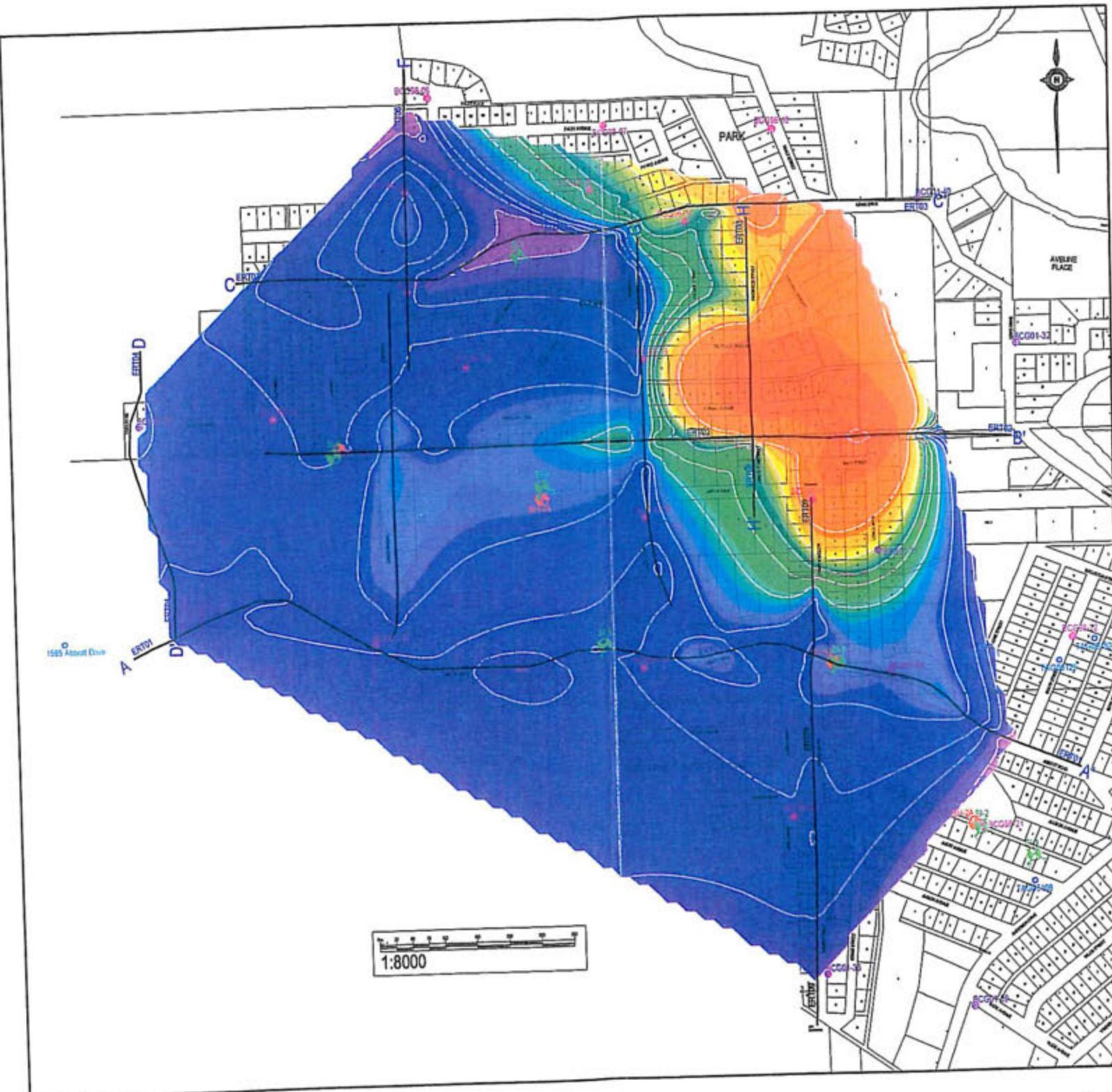
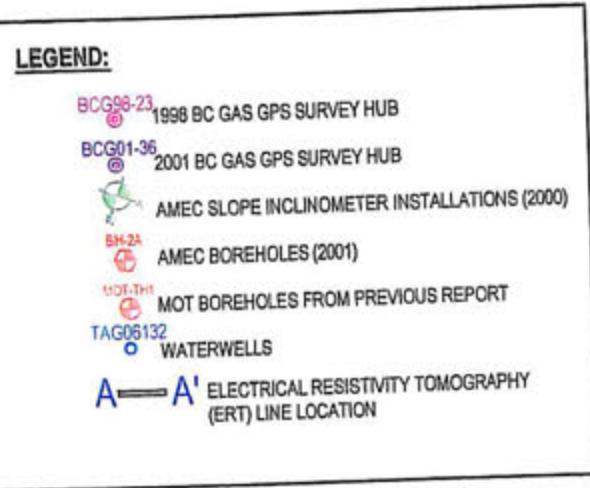








TITLE: 10 m Depth Layer ERT Residuals Map	
PROJECT: WEST QUESNEL ELECTRICAL RESISTIVITY TOMOGRAPHY SURVEY	
CLIENT: CITY OF QUESNEL	CONSULTING ENGINEER: AMEC Earth & Environmental Ltd.
DRAWING PRODUCED BY: SURFACE SEARCH INC. ENGINEERING & ENVIRONMENTAL GEOGRAPHICS 12038 Lake Emerald Cr. #11, Calgary, AB, T5J 2J5 Ph: (403) 531-9721 Fax: (403) 294-1240 (email: headoffice@surfacesearch.com)	DRAWING SCALE: AS SHOWN DATE: January 2005 E.M.L NAME: 10 m ERT Residuals Map Plot.DWG
DRAWN BY: PT	DRAWN BY: 04-863
REVIEWED BY: PT	REVIEWED BY: 04-860-R1
REV. NO.: 01	REV. NO.: 01

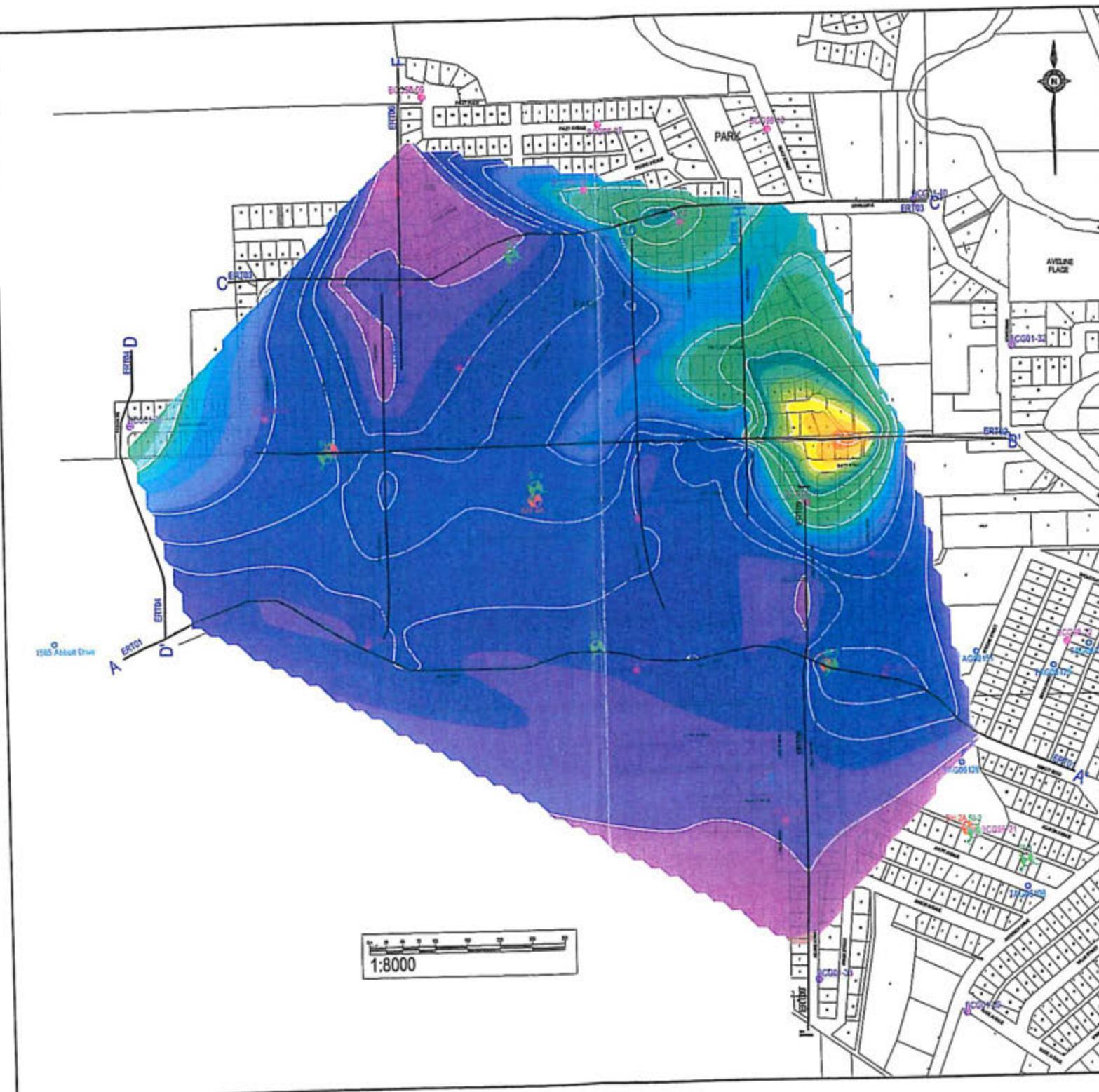


Map showing contoured pseudo-plan plot of the ERT data from residual electrical resistivity values taken at a 20 m subsurface depth level, as derived from the inverted ERT cross-section models.

TITLE: 20 m Depth Layer ERT Residuals Map			
PROJECT: WEST QUESNEL ELECTRICAL RESISTIVITY TOMOGRAPHY SURVEY			
CLIENT: CITY OF QUESNEL	CONSULTING ENGINEER: AMEC Earth & Environmental Ltd.		
DRAWING PRODUCED BY: SURFACE SEARCH INC. ENGINEERING & ENVIRONMENTAL GEOPHYSICS 12038 Lake Emerald Cr. SW, Calgary, AB, T2J 2J5 Ph: (403) 531-9721 Fax: (403) 294-1240 Email: headoffice@surfacesearch.com	DRAWING SCALE: AS SHOWN	DATE: January 2005	FILE NAME: 20 m ERT Residuals Map Plot.DWG
DRAWING NO.: PT	JOB NO.: 04-863	DRAWING NO.: 04-860-0R2	REV. NO.: 01

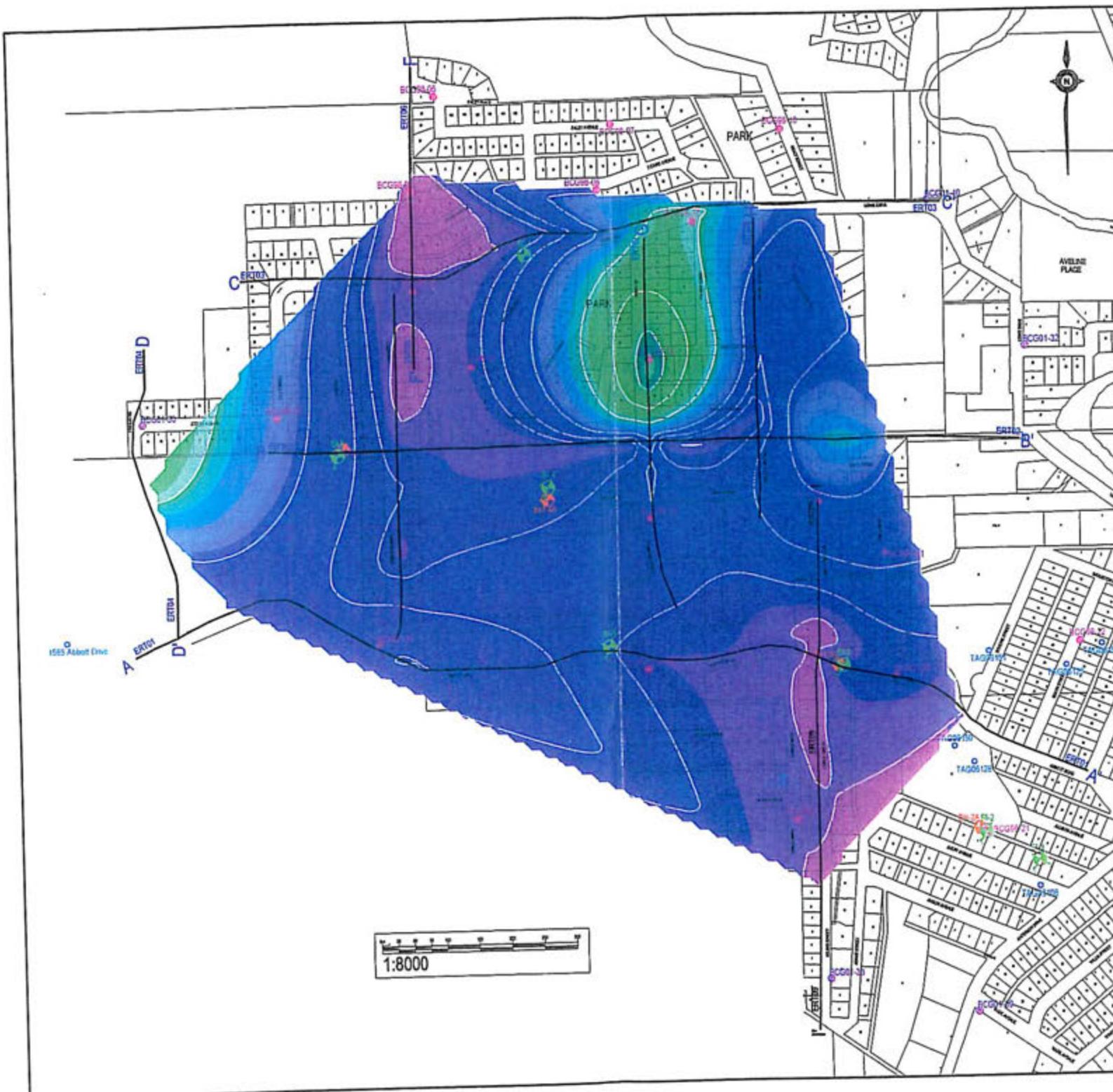
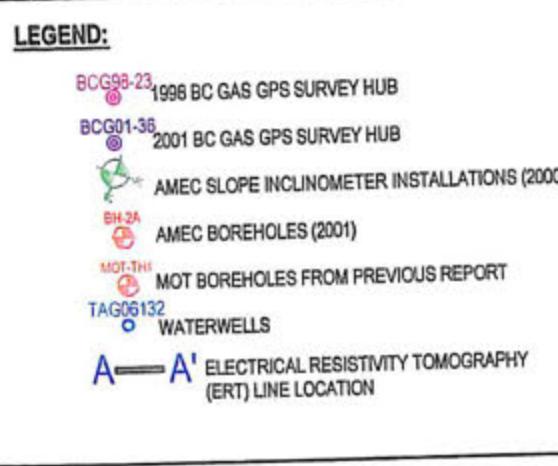
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- BCG01-36 2001 BC GAS GPS SURVEY HUB
- AMEC SLOPE INCLINOMETER INSTALLATIONS (2000)
- BH-2A AMEC BOREHOLES (2001)
- MOT-THI MOT BOREHOLES FROM PREVIOUS REPORT
- TAG06132 WATERWELLS
- A—A' ELECTRICAL RESISTIVITY TOMOGRAPHY (ERT) LINE LOCATION



Map showing contoured pseudo-plan plot of the ERT data from residual electrical resistivity values taken at a 40 m subsurface depth level, as derived from the inverted ERT cross-section models.

TITLE: 40 m Depth Layer ERT Residuals Map	
PROJECT: WEST QUESNEL ELECTRICAL RESISTIVITY TOMOGRAPHY SURVEY	
CLIENT: CITY OF QUESNEL	CONSULTING ENGINEERS: AMEC Earth & Environmental Ltd.
DRAWING PRODUCED BY: SURFACE SEARCH INC. ENGINEERING & ENVIRONMENTAL GEOPHYSICS	DRAWING SCALE: AS SHOWN
12035 Lake Emerald Cr. SW, Calgary, AB, T5J 2J5 Ph: (403) 531-9721 Fax: (403) 294-1240 Email: headoffice@surfacesearch.com	DATE: January 2005
DRAWN BY: PT	DRAWING NO.: 04-863
REV. NO.: 01	FILE NAME: 40 m ERT Residuals Map Plot.DWG



60 m Depth Layer ERT Residuals Map

PROJECT: WEST QUESNEL ELECTRICAL RESISTIVITY TOMOGRAPHY SURVEY

CLIENT: CITY OF QUESNEL	CONSULTING ENGINEERS: AMEC Earth & Environmental Ltd.
DRAWING PRODUCED BY: SURFACE SEARCH INC. INTEGRATED ENVIRONMENTAL GEOPHYSICS 12035 Lake Emerald Cr. SW, Calgary, AB, T2J 2J5 Ph: (403) 331-9721 Fax: (403) 294-1240 Email: headoffice@surfacesearch.com	DRAWING SCALE: AS SHOWN
DATE: January 2005	FILE NAME: 60 m ERT Residuals Map Plot.DWG
DRAWN BY: PT	JOB NO.: 04-863
	DRAWING NO.: 04-860-0R4
	REV. NO.: 01

Table C1: GPS Movement Hub Data to November 2006

Movement Hub	Date Installed	Horizontal Displacement Since Installation	
		Magnitude (mm)	Vector* (°)
98-02	September 1998	350	100
98-04	September 1998	296	55
98-05	September 1998	307	89
98-06	September 1998	369	43
98-07	September 1998	313	63
98-08	September 1998	297	66
98-09	September 1998	389	91
98-14	September 1998	373	94
98-15	September 1998	275	77
98-16	September 1998	199	73
98-17	September 1998	421	97
98-18	September 1998	379	99
98-19	September 1998	340	14
98-20	September 1998	317	106
98-21	September 1998	191	115
98-22	September 1998	41	100
98-23	September 1998	21	106
01-30	December 2001	135	106
01-31	December 2001	221	92
01-32	December 2001	16	142
01-33	December 2001	120	106
01-34	December 2001	160	108
01-35	December 2001	16	135
01-36	December 2001	13	61
01-37	December 2001	185	102
01-38*	December 2001	13	122
01-39	December 2001	73	134
01-40	December 2001	85	84
06-01	November 2006	-	-
06-02	November 2006	-	-
06-03	November 2006	-	-
06-04	November 2006	-	-
06-05	November 2006	-	-
06-06	November 2006	-	-
06-07	November 2006	-	-
06-08	November 2006	-	-
06-09	November 2006	-	-
06-10	November 2006	-	-
06-11	November 2006	-	-
06-12	November 2006	-	-
06-13	November 2006	-	-
06-14	November 2006	-	-
06-15	November 2006	-	-
06-16	November 2006	-	-
06-17	November 2006	-	-
06-18	November 2006	-	-
06-19	November 2006	-	-
06-20	November 2006	-	-
06-21	November 2006	-	-

* Quesnel No.1 Station, GPS hub is located out of the study area

Figure C1: Monthly Recorded Total Precipitation vs. Monthly Total Precipitation from 30-Year Normal Climate Data

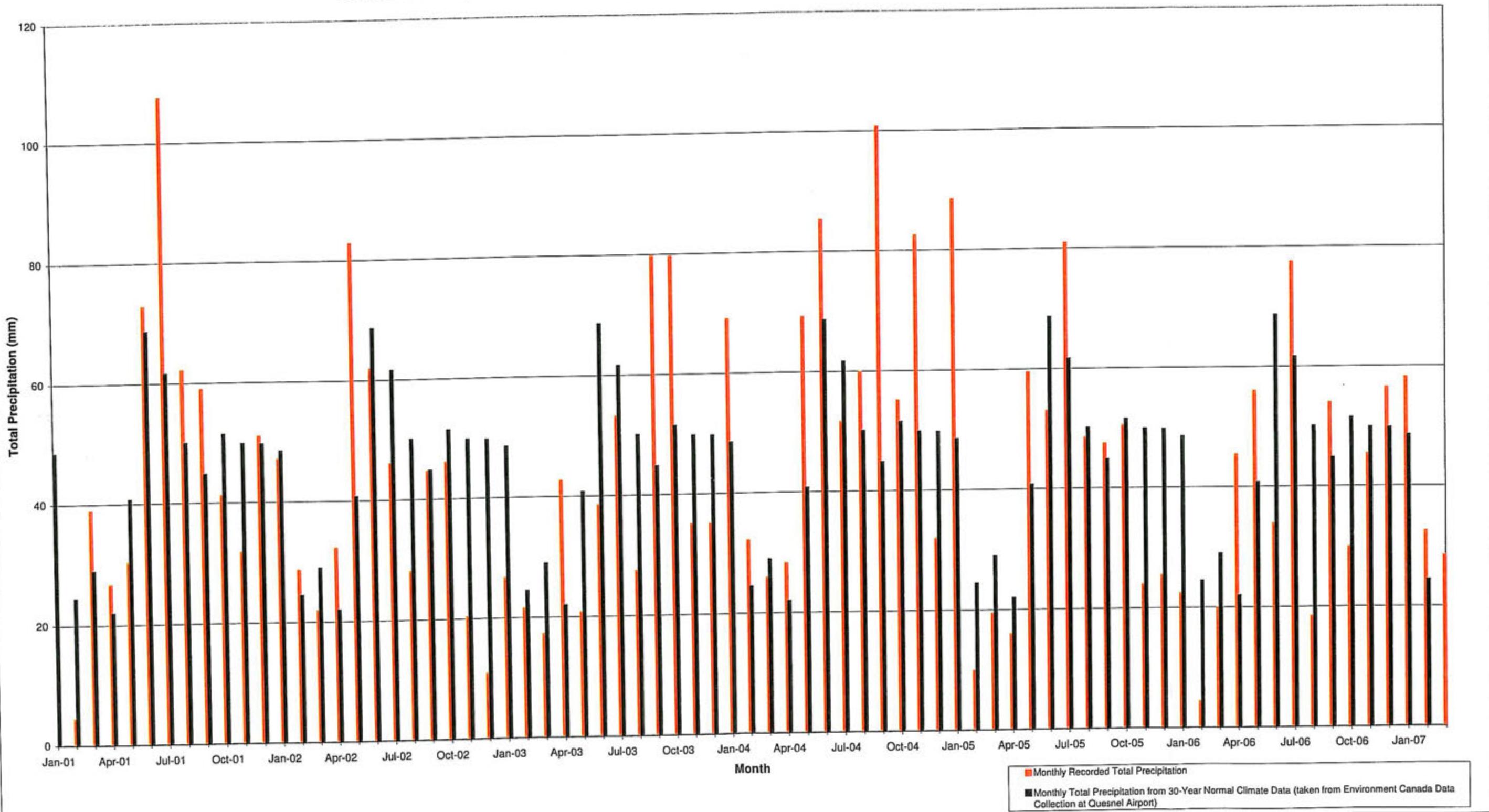


Chart 1: Cumulative Net Lateral Displacement vs. Time (September 1998 to November 2006)

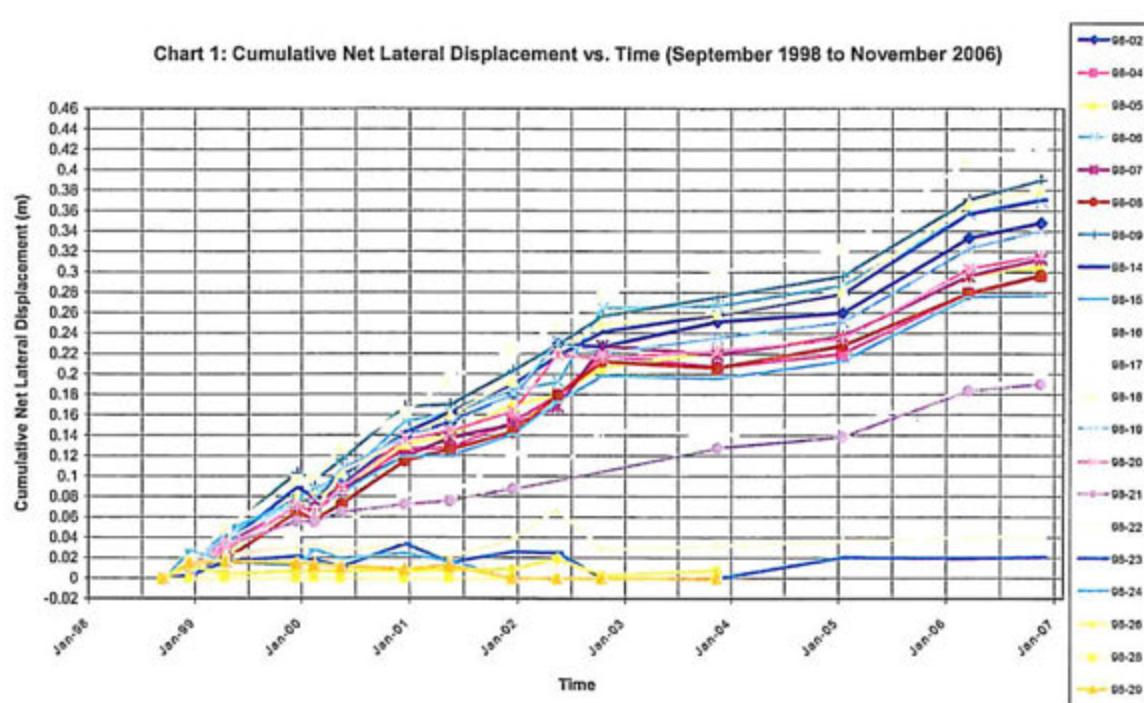
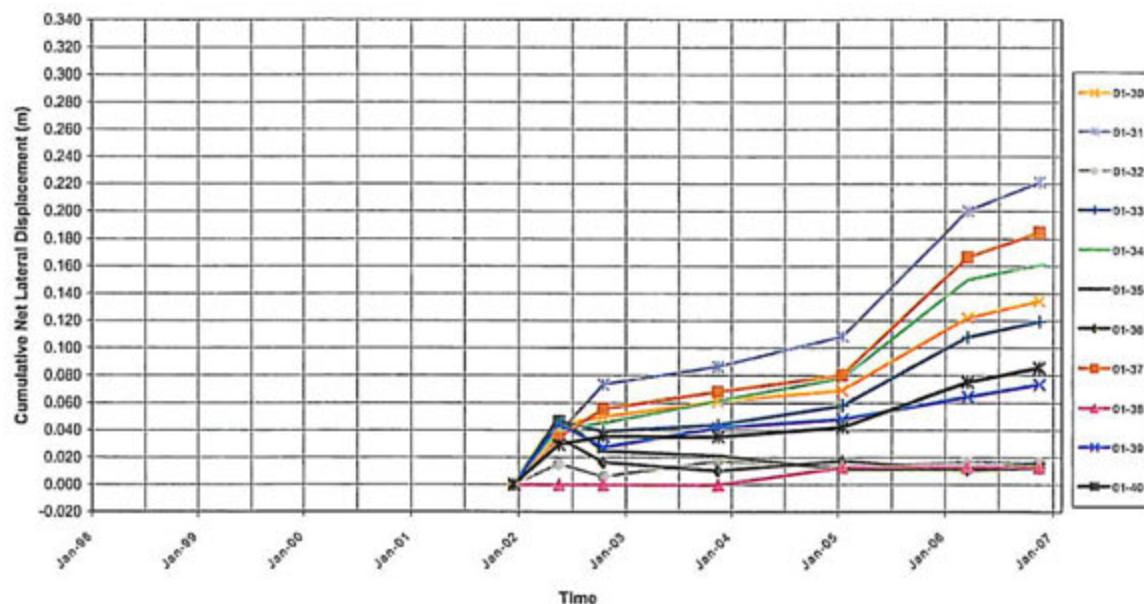


Chart 2: Cumulative Net Lateral Displacement vs. Time (December 2001 to November 2006)



CITY OF QUESNEL

WEST QUESNEL LAND STABILITY STUDY
QUESNEL, BC

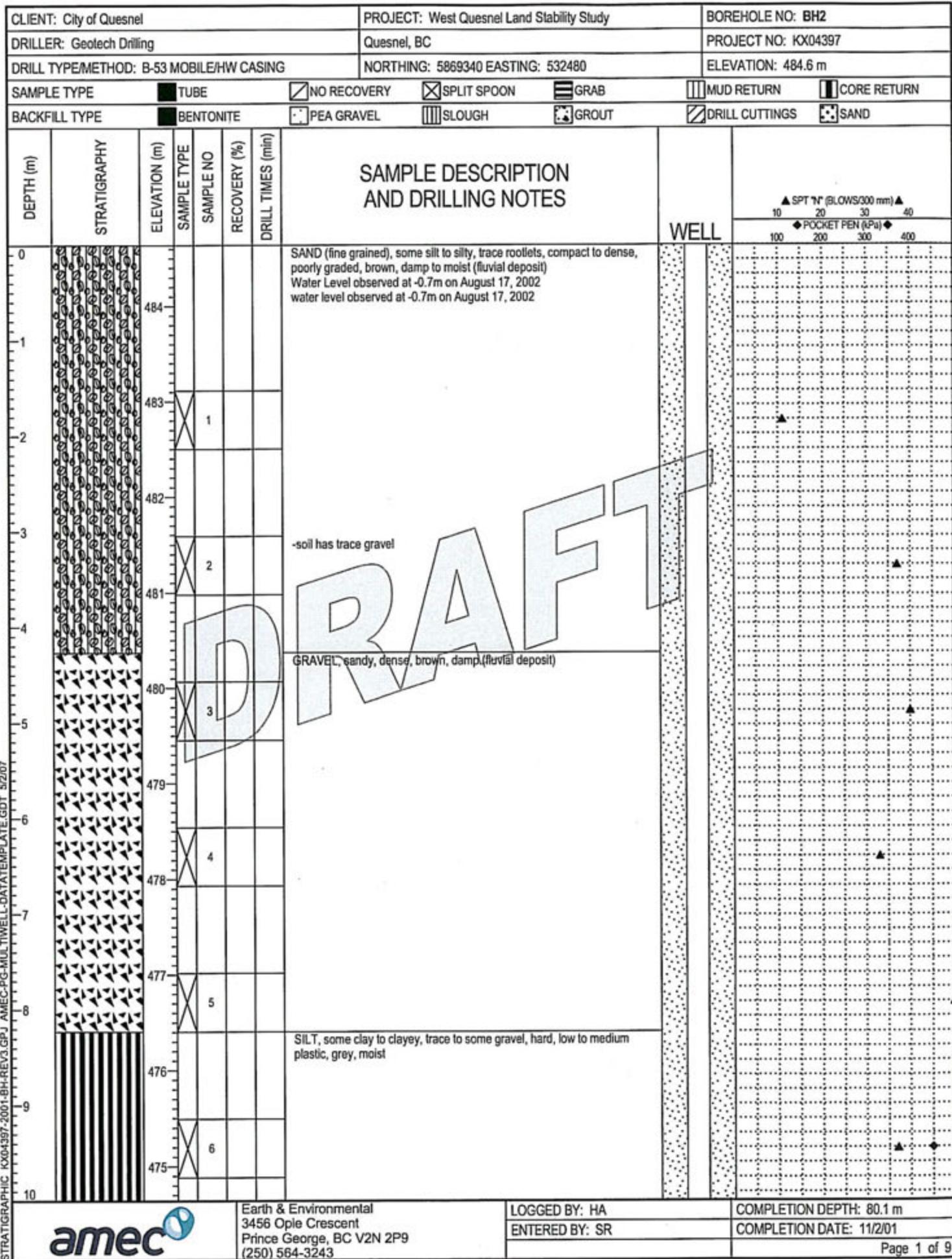
Table D1: Drill Hole Locations and Log Comments

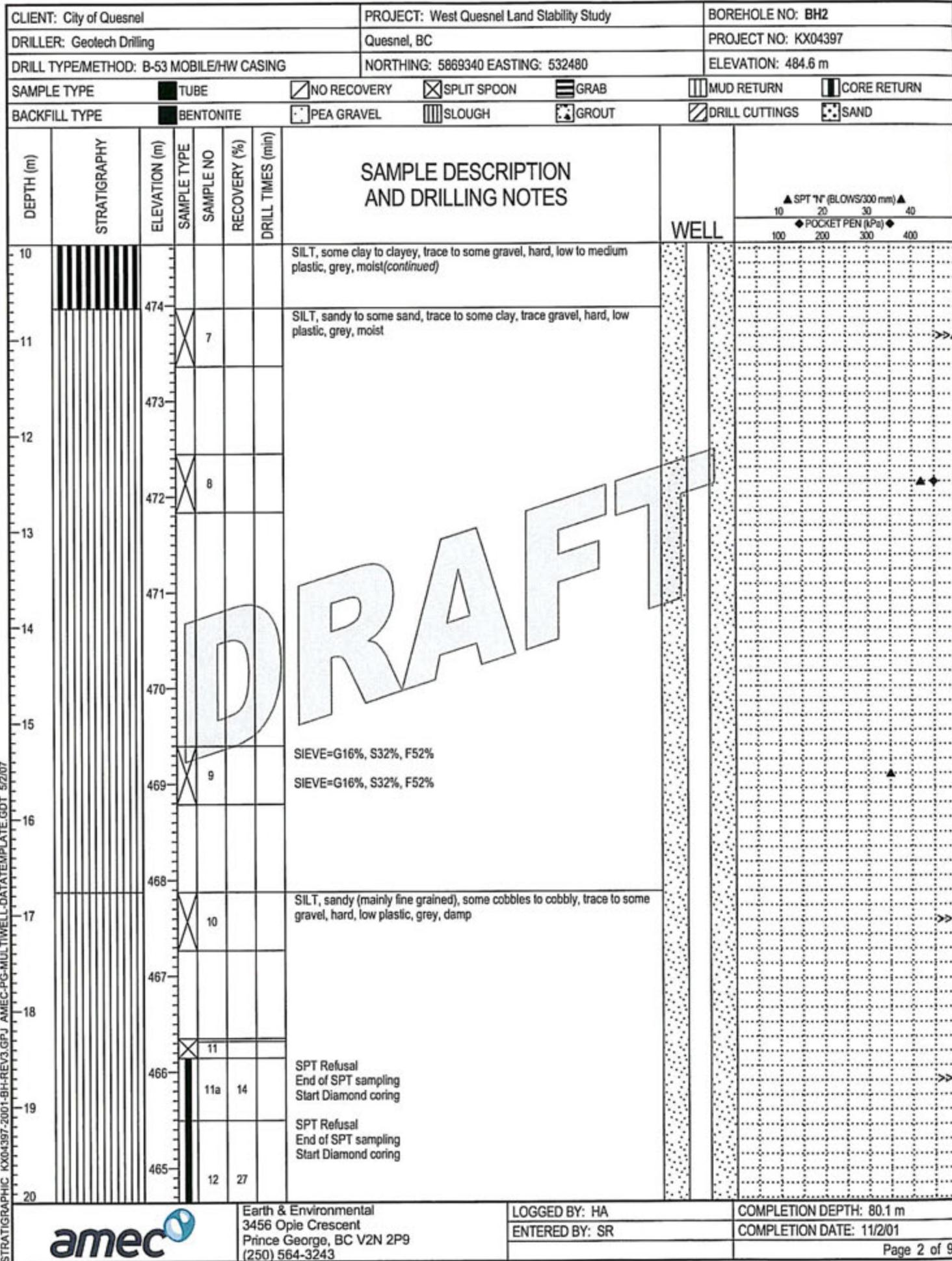
Hole ID	Location UTM (NAD 83)		Log Comments
	Northing	Easting	
SI1	5869298	532569	Drill logs not included as SIs were drilled close to BHs (corresponding BH logs include details for both holes)
SI2	5869343	532490	
SI3	5869598	531830	
SI4	5869878	531830	
SI5	5869633	531919	
SI6	5869933	531510	
SI7	5870232	531799	
SI8	5870035	532164	
SI9	5870114	531882	
SI10	5869649	531587	
SI11	5870355	532041	
SI12	5870278	532622	
SI13	5869938	532468	
SI14	5869683	531277	
SI15	5870278	531402	
BH2	5869340	532480	Drill log included
BH2A/B	5869742	532208	
BH2C	5869742	532210	
BH3	5869597	532264	
BH3A	5869771	532007	
BH3B/C	5869720	522005	
BH4	5869868	531829	
BH4A/B	5869783	532041	
BH4C	5869783	532043	
BH6	5869940	531514	
BH7	5870226	531799	
BH8	5870043	532161	
BH8A	5870043	532163	Drill log not included, BH8A combined with BH8 to make one drill log
BH9	5870120	531883	Drill log included, hole drilled twice in close proximity due to drilling difficulties
BH10	5869655	531592	Drill log included
BH11	5870360	532037	
BH12	5870272	531620	
BH13	5869938	532468	
BH14	5869693	531276	
BH15	5870278	531417	
BH16	5869544	530634	
BH16A	5869552	530634	Drill log not included, BH16A combined with BH16 to make one drill log
PW1	5869772	531993	Drill log included
PW2	5869595	532272	

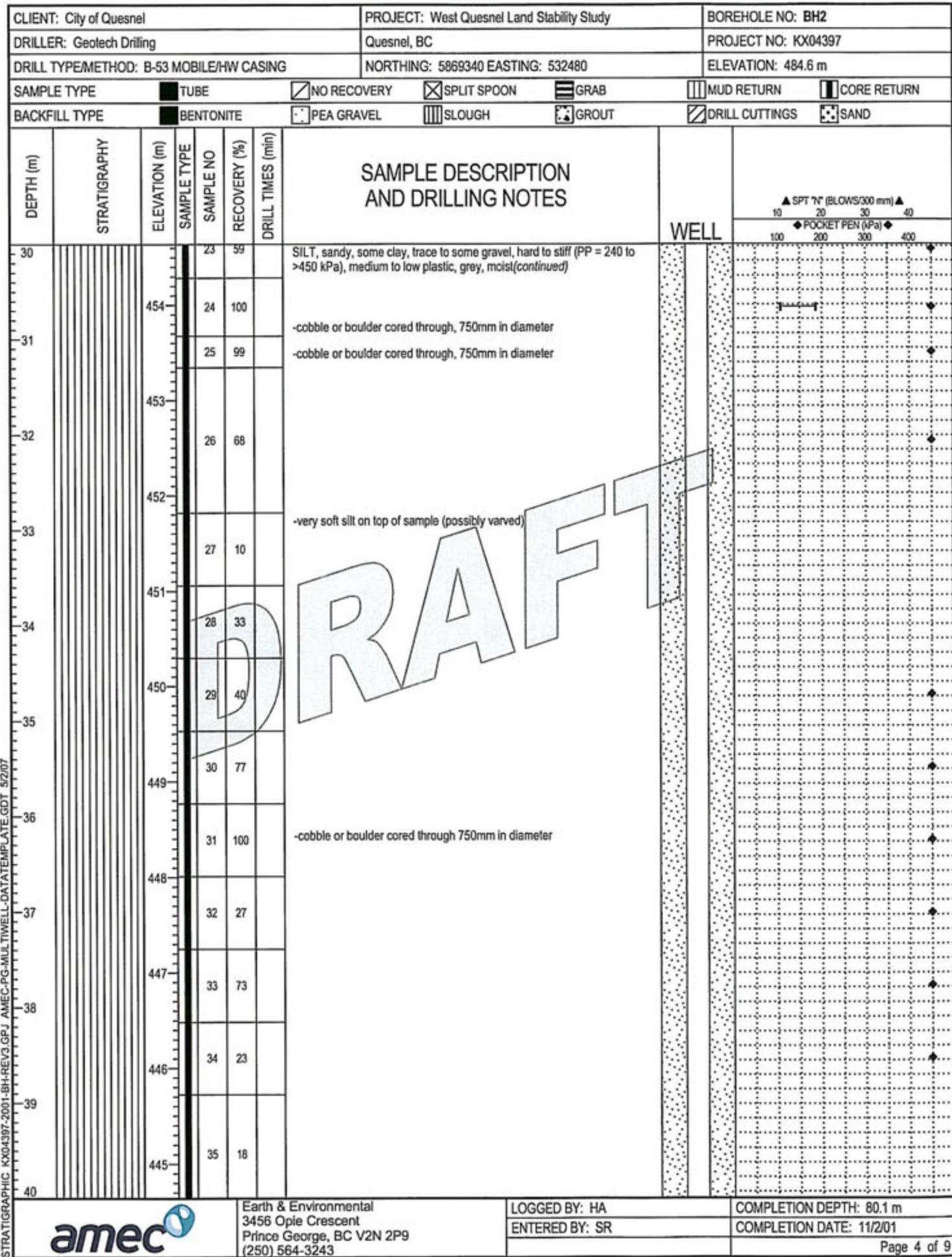
Description of Rock Logging Terms

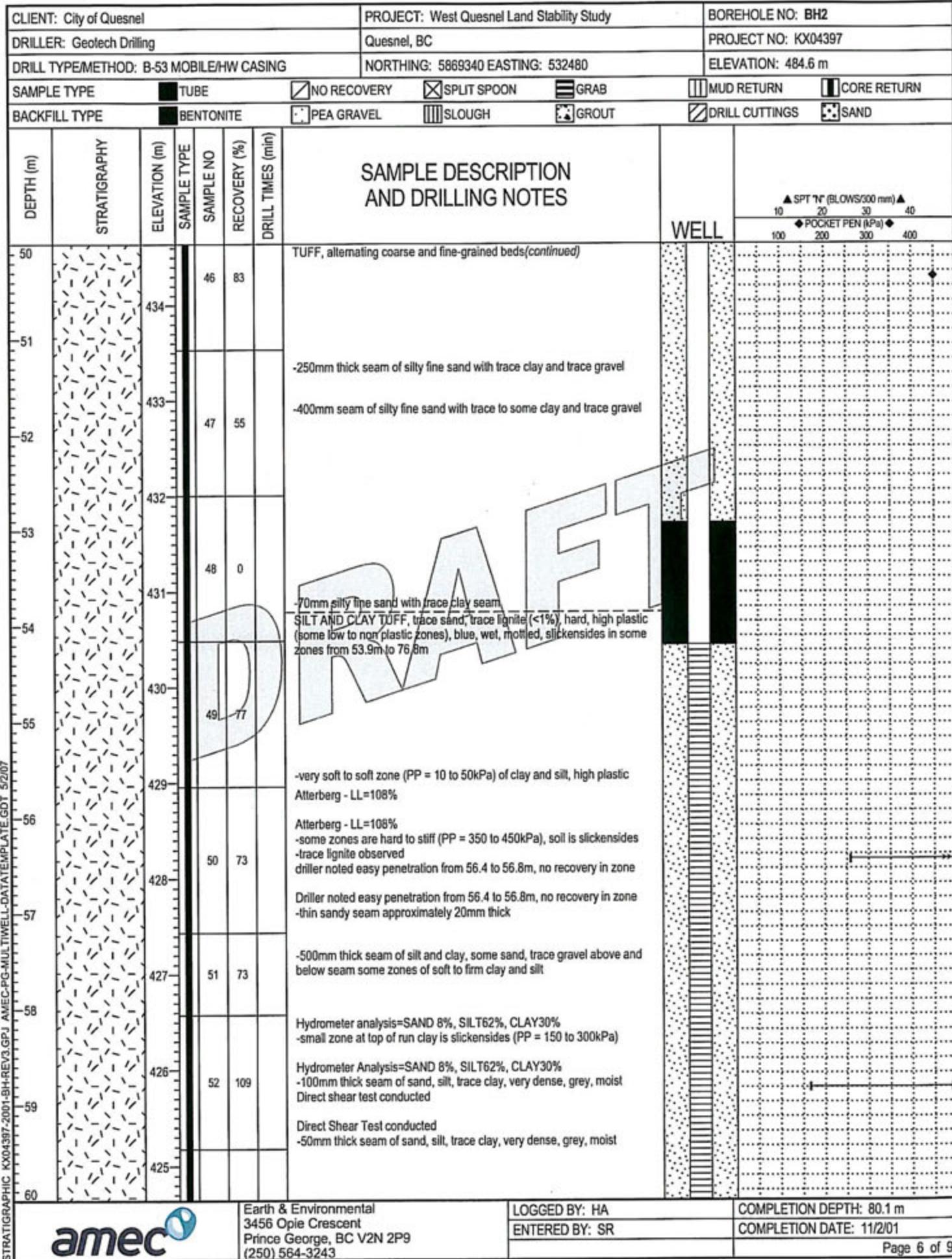
Core Recovery and Fracture Index		
Field	Description	Example
Run Length	The drilled length of the run as measured by the driller at the drill head. All depths are measured from the drill collar.	Drilled 1.0 m
Core Recovery	Total length of all recovered rock (including non-intact core), expressed as a percentage the run length. Reassemble the core to measure and bunch sections of non-intact core so they don't over-represent the recovery. Shouldn't be greater than 100% unless the barrel has picked up rock core from a previous run.	Recovered 0.90 m Core recovery = 90%
Rock Quality Designation (RQD)	Total length of SOLID CORE pieces in the run that are greater than 100 mm in length, expressed as a percentage the run length. Drilling induced fractures should be ignored, though if in doubt, assume a fracture is natural.	Pieces longer than 100 mm add up to 0.80 m RQD = 80%

Intact Rock Strength		
Estimated UCS (MPa)	Description	Field Identification
0.25-1.0	RO – Extremely Weak Rock	Indented by thumbnail, core crumbles in hand
1-5	R1 – Very Weak Rock	Crumbles under firm blows with the point of a geological hammer; can be peeled with a pocket knife
5-25	R2 – Weak Rock	Can be peeled with a pocket knife with difficulty; shallow indentations made by firm blow with point of geological hammer
25-50	R3 – Medium Strong Rock	Cannot be scraped or peeled with a pocket knife; specimen can be fractured with a single blow with point of geological hammer
50-100	R4 – Strong Rock	Specimen requires more than one blow with point of geological hammer to fracture it
100-250	R5 – Very Strong Rock	Specimen requires many blows with point of geological hammer to fracture it
>250	R6 – Extremely Strong Rock	Specimen can only be chipped with point of geological hammer and rings when struck

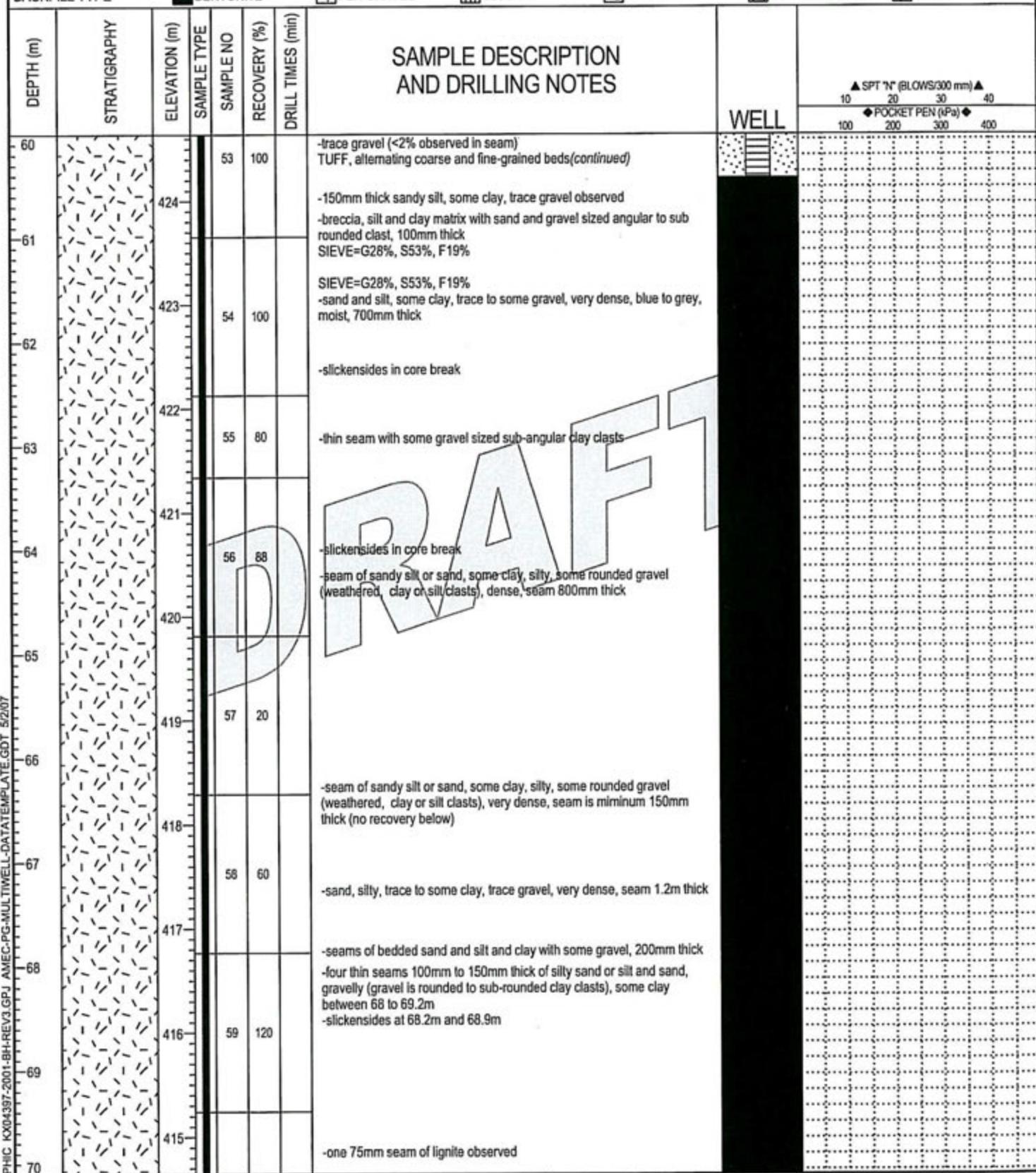








CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH2
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING	NORTHING: 5869340 EASTING: 532480	ELEVATION: 484.6 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



STRATIGRAPHIC KX04397-2001-BH-REV3.GPJ AMEC-PG-MULTIWELL-DATATEMPLATE.GDT 5/2/2007



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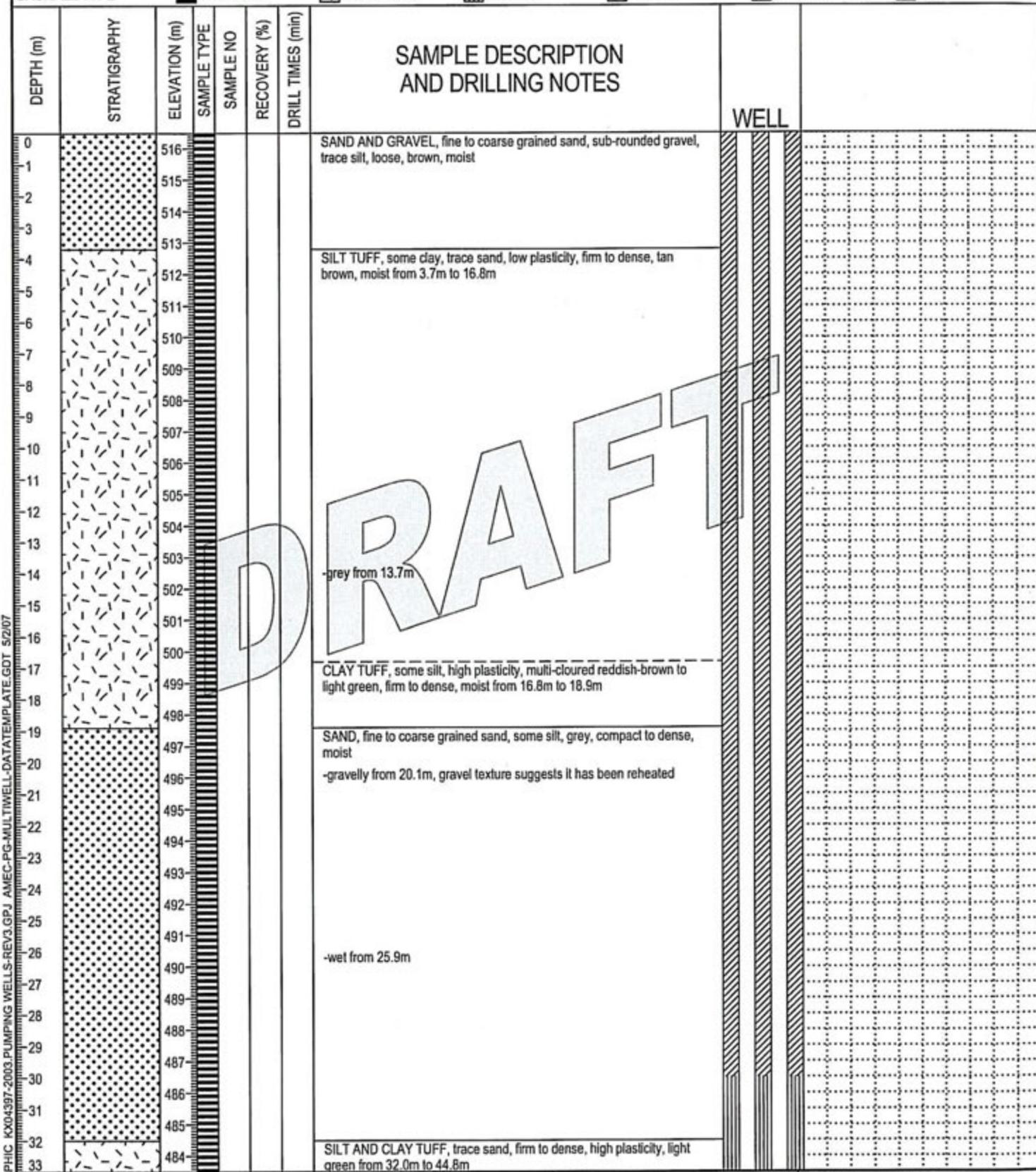
LOGGED BY: HA
ENTERED BY: SR

COMPLETION DEPTH: 80.1 m
COMPLETION DATE: 11/2/01

CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH2	
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING				NORTHING: 5869340 EASTING: 532480				ELEVATION: 484.6 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN			
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND			
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES	WELL	
70				60	177		-slickensides evident in broken core, swelling clay (1.5m run) TUFF, alternating coarse and fine-grained beds(continued) -400mm thick seam of sand, gravelly (gravel is mainly clasts sub-rounded to rounded of clay), silty, some clay, brown to grey, moist		
71		414		61	100		SIEVE=S46%, F54%		
72		413		61	100		SIEVE=S46%, F54% -700mm thick seam of silty fine sand with trace gravel, and trace clay, grey, very dense		
73		412		62	100		-zone of breccia, sand and gravel in silt and clay matrix, harder towards base, at 72.1m has rounded gravel sized clay clasts		
74		411		63	100		RAF		
75		410		63	100		-50mm thick seam of silty sand with some gravel (gravel is angular basal pieces with some rounded clay clasts)		
76		409		64	100		-zone of breccia as per 71.8m, bedded with sand and gravel, zone is 1m thick		
77		408		64	100		-occasional sandy or gravelly layer in slickensided clay, layers ranged from 70mm to 600mm thick		
78		407		65	100		SAND TUFF, (fine), silty, some gravel (mixture of rounded clay clasts and basalt clasts), trace to some clay, very dense, poorly graded, brown, moist to wet, layered with some seams of silt and clay from 76.8m to 80.1m SIEVE=S50%, F50%		
79		406		65	100		SIEVE=S50%, F50%		
80		405		66	103				
<p>The stratigraphic column diagram illustrates the borehole profile from 70m to 80m depth. It shows various geological features and sample locations. Key observations include: <ul style="list-style-type: none"> 70m: Slickensides evident in broken core, swelling clay (1.5m run). TUFF, alternating coarse and fine-grained beds (continued). 71m: 400mm thick seam of sand, gravelly (gravel is mainly clasts sub-rounded to rounded of clay), silty, some clay, brown to grey, moist. 72m: SIEVE=S46%, F54%. 73m: -700mm thick seam of silty fine sand with trace gravel, and trace clay, grey, very dense. 74m: -zone of breccia, sand and gravel in silt and clay matrix, harder towards base, at 72.1m has rounded gravel sized clay clasts. 75m: 50mm thick seam of silty sand with some gravel (gravel is angular basal pieces with some rounded clay clasts). 76m: -zone of breccia as per 71.8m, bedded with sand and gravel, zone is 1m thick. 77m: Occasional sandy or gravelly layer in slickensided clay, layers ranged from 70mm to 600mm thick. 78m: SAND TUFF, (fine), silty, some gravel (mixture of rounded clay clasts and basalt clasts), trace to some clay, very dense, poorly graded, brown, moist to wet, layered with some seams of silt and clay from 76.8m to 80.1m. 79m: SIEVE=S50%, F50%. 80m: SIEVE=S50%, F50%. The diagram also includes a legend for SPT "N" (BLOWS/300 mm) and Pocket Pen (kPa) values. </p>									
		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: HA	COMPLETION DEPTH: 80.1 m		
						ENTERED BY: SR	COMPLETION DATE: 11/2/01		
Page 8 of 9									

CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH2		
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397		
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING			NORTHING: 5869340 EASTING: 532480			ELEVATION: 484.6 m		
SAMPLE TYPE		<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE		<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	SAMPLE DESCRIPTION AND DRILLING NOTES					WELL	
		ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)		
80						End of Borehole at 80.1m, 25mm diameter standpipe was installed to approximately 60.4m below grade		
81								
82								
83								
84								
85								
86								
87								
88								
89								
90								
								
<small>STRATIGRAPHIC KX04397-2001-BH-REV3.GPJ AMEC-PG-MULTIWELL-DATA TEMPLATE.GDT 5/2/07</small>								
			Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243		LOGGED BY: HA ENTERED BY: SR		COMPLETION DEPTH: 80.1 m COMPLETION DATE: 11/2/01	
<small>▲ SPT 'N' (BLOWS/300 mm) ▲ 10 ◆ POCKET PEN (kPa) ◆ 40 100 200 300 400</small>								

CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH2AB
DRILLER: Cariboo Water Wells	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary	NORTHING: 5869742.43 EASTING: 532207.57	ELEVATION: 516.5 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



STRATIGRAPHIC KX04397-2003.PUMPING.WELLS-REV3.GPJ AMEC:PG-MULTIWELL-DATATEMPLATE.GDT 5/2/07



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LOGGED BY: SG

ENTERED BY: CD

COMPLETION DEPTH: 90.4 m

COMPLETION DATE: 8/7/03

STRATIGRAPHIC LOG KX04397-2003 PUMPING WELLS-REV3.GPJ AMEC-PG-MULTIWELL-DATA\WELL-DATA\WELL.DAT

CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH2AB					
DRILLER: Cariboo Water Wells		Quesnel, BC		PROJECT NO: KX04397					
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary		NORTHING: 5869742.43 EASTING: 532207.57		ELEVATION: 516.5 m					
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN				
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS				
					<input checked="" type="checkbox"/> CORE RETURN				
					<input checked="" type="checkbox"/> SAND				
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES		WELL
33		483					-drillers began adding water from 32.0m SILT AND CLAY TUFF, trace sand, firm to dense, high plasticity, light green from 32.0m to 44.8m(continued)		
34		482							
35		481							
36		480							
37		479					-trace to some semi-consolidated clasts from 37.2m		
38		478							
39		477							
40		476							
41		475							
42		474							
43		473							
44		472							
45		471					SILT AND CLAY TUFF, trace to some black sand, firm, high plasticity, dark brown from 44.8m to 46.5m		
46		470					LIGNITE, very weak, dark brown to black		
47		469					-Piezometer B set between 47.4m and 48.2m, static water level recorded at 40.88m (October 8, 2003)		
48		468					TUFF, alternating coarse and fine-grained beds SILT AND CLAY TUFF, firm to dense, high plasticity, green from 48.3m to 60.3m		
49		467							
50		466							
51		465					-light green from 51.8m		
52		464							
53		463							
54		462							
55		461							
56		460					-hard drilling from 56.4m		
57		459							
58		458					-trace sand from 57.9m		
59		457							
60		456					SILT TUFF, clayey, trace to some sand, trace gravel, green from 60.3m to 62.2m		
61		455							
62		454					SILT TUFF, sandy, trace semi-consolidated gravel clasts, dense, light grey from 62.2m to 65.5m -casing refusal at 62.4m, drilled open hole from 62.2m		
63		453							
64		452							
65		451							

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LOGGED BY: SG
ENTERED BY: CD

COMPLETION DEPTH: 90.4 m
COMPLETION DATE: 8/7/03

Page 2 of 3

STRATIGRAPHIC LOG PUMPING WELLS-REV3.GPJ AMEC-PG-MULTIWELL-DATA TEMPLATE.GDT 5/2007

CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH2AB					
DRILLER: Cariboo Water Wells		Quesnel, BC		PROJECT NO: KX04397					
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary		NORTHING: 5869742.43 EASTING: 532207.57		ELEVATION: 516.5 m					
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB				MUD RETURN <input type="checkbox"/>					
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT				CORE RETURN <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND					
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES		WELL
66		450					SAND TUFF, silty, trace gravel, dense, light grey from 65.5m to 67.1m		
67		449					TUFF, alternating coarse and fine-grained beds(continued)		
68		448					SILT TUFF, sandy, trace gravel, dense, light grey from 67.1m to 70.1m		
69		447							
70		446					SAND TUFF, silty, trace gravel, dense, light grey from 70.1m to 73.1m		
71		445							
72		444							
73		443					SILT TUFF, clayey, some sand, trace gravel, dense, light grey from 73.1m to 79.2m		
74		442							
75		441							
76		440							
77		439							
78		438							
79		437					SILT AND CLAY TUFF, trace sand, trace semi-consolidated gravel sized clasts, dense, medium plasticity, light grey from 79.2m to 85.3m		
80		436							
81		435							
82		434							
83		433							
84		432							
85		431					SAND TUFF, silty, trace to some gravel, dense, grey from 85.3m to 90.4m		
86		430							
87		429							
88		428					-trace to some clay from 88.4m		
89		427					-Piezometer A set between 88.4m and 86.9m, static water level recorded at 80.84m (October 8, 2003)		
90		426					End of Borehole 90.4m below grade		
91		425							
92		424							
93		423							
94		422							
95		421							
96		420							
97		419							
98		418							

amec

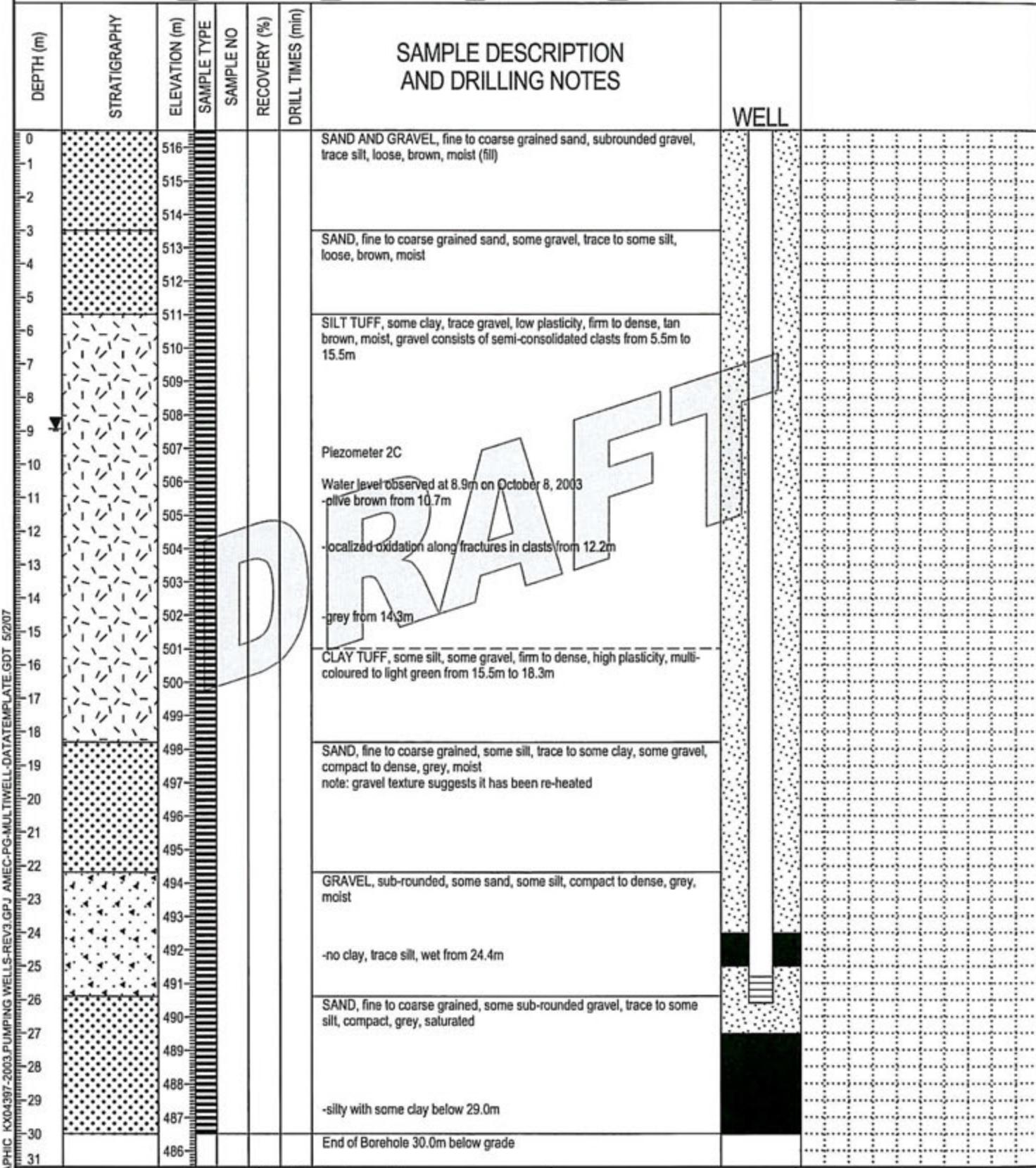
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: SG
ENTERED BY: CD

COMPLETION DEPTH: 90.4 m
COMPLETION DATE: 8/7/03

Page 3 of 3

CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH2C
DRILLER: Cariboo Water Wells	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary	NORTHING: 5869742.28 EASTING: 532210.04	ELEVATION: 516.5 m
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BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



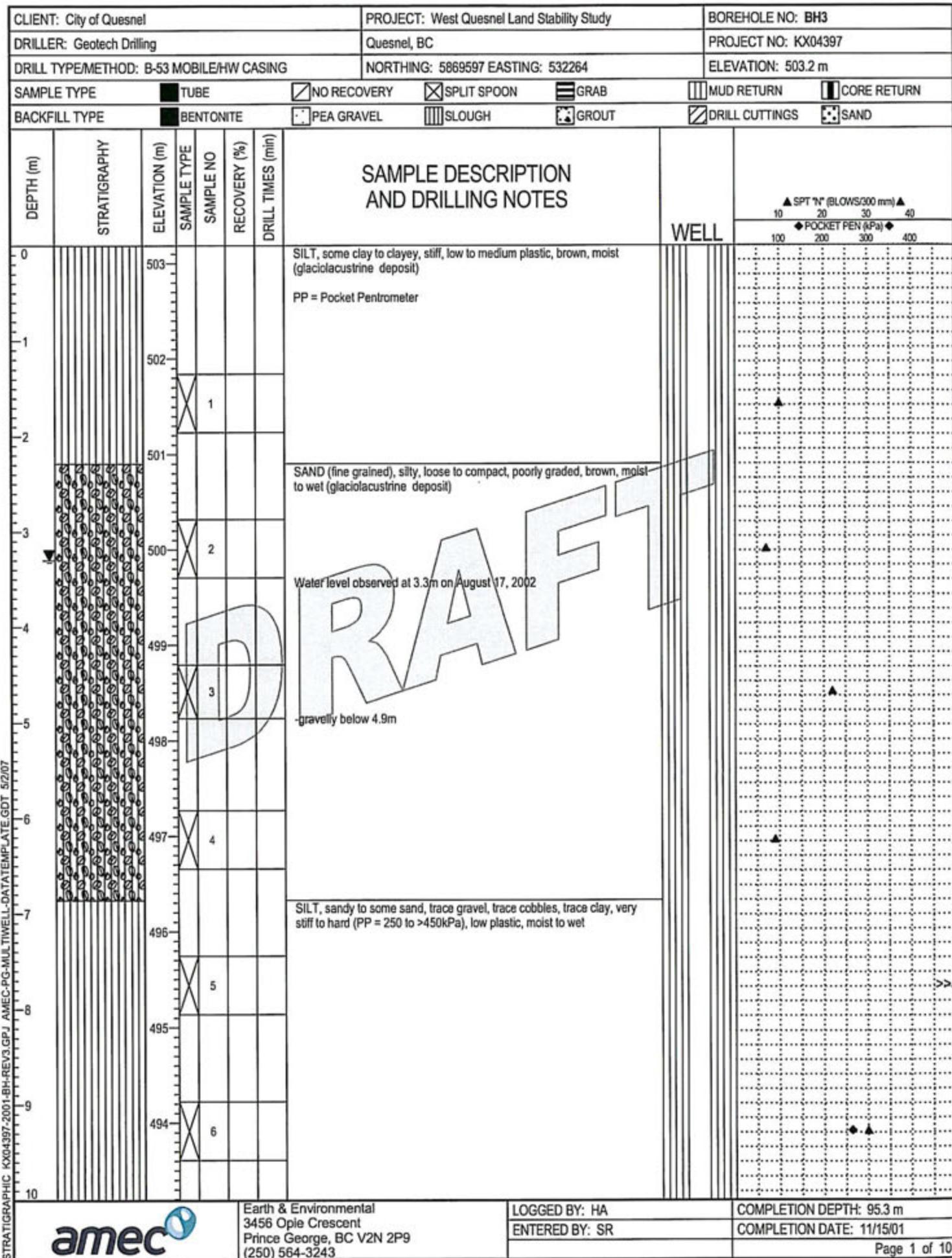
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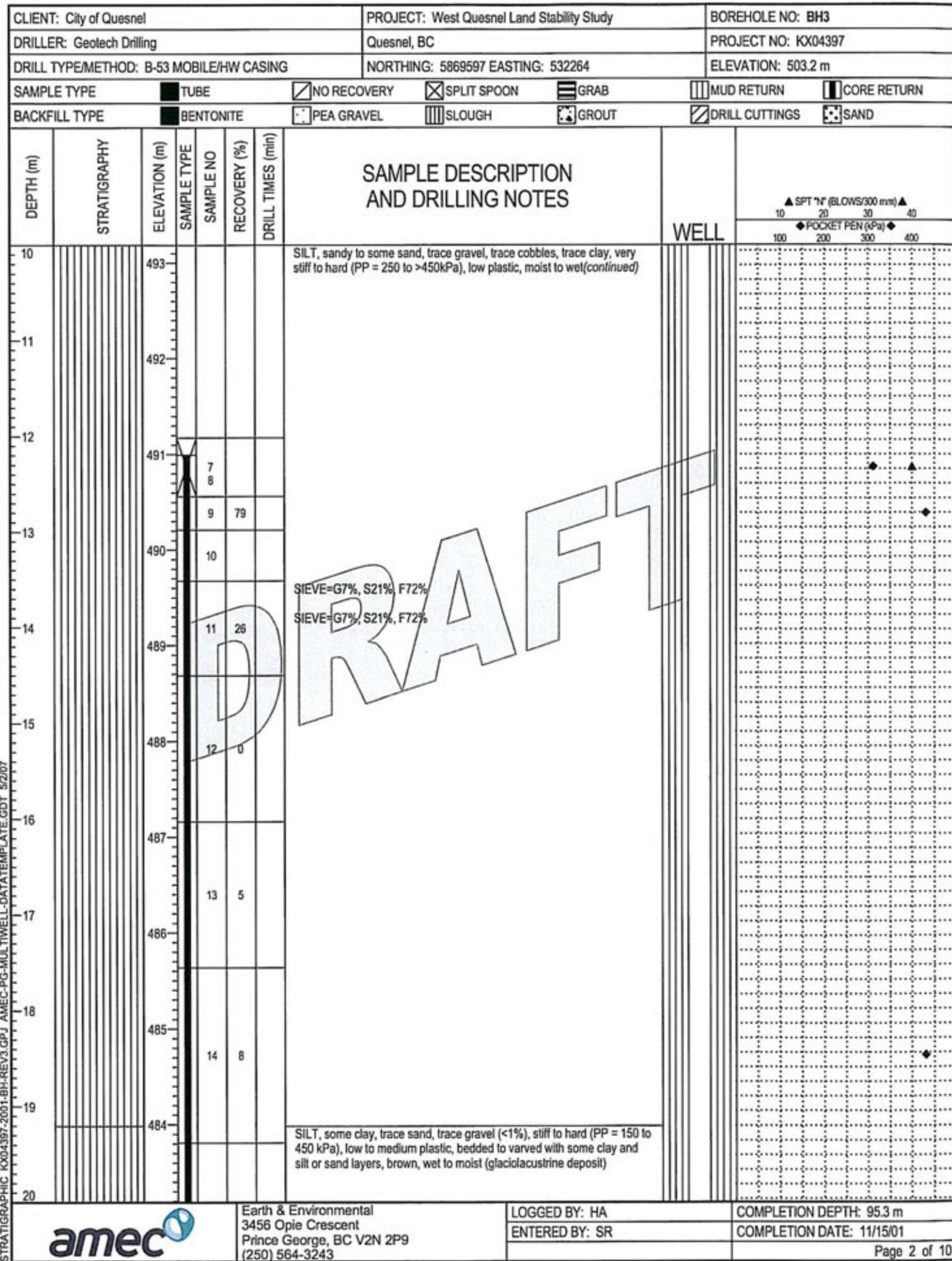
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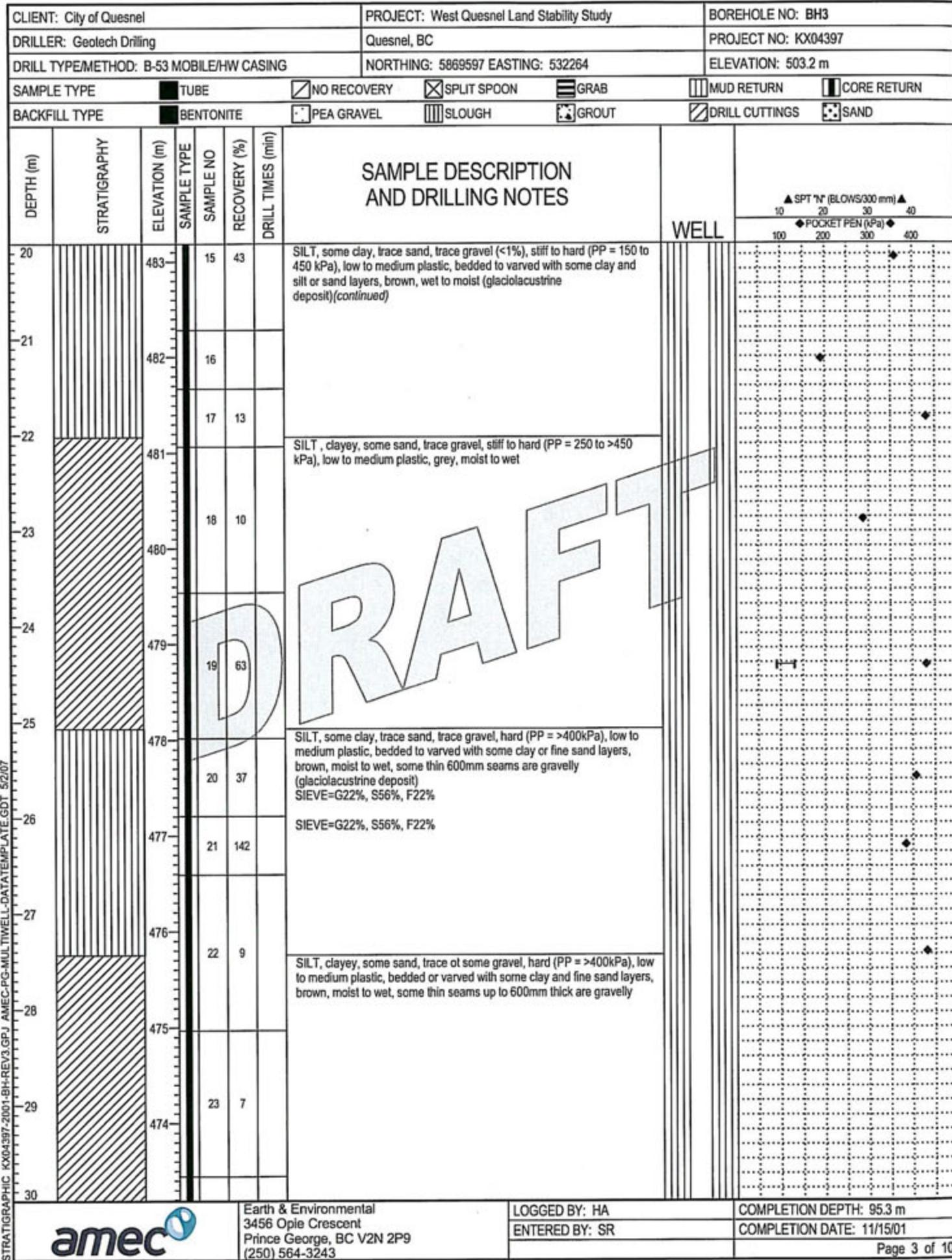
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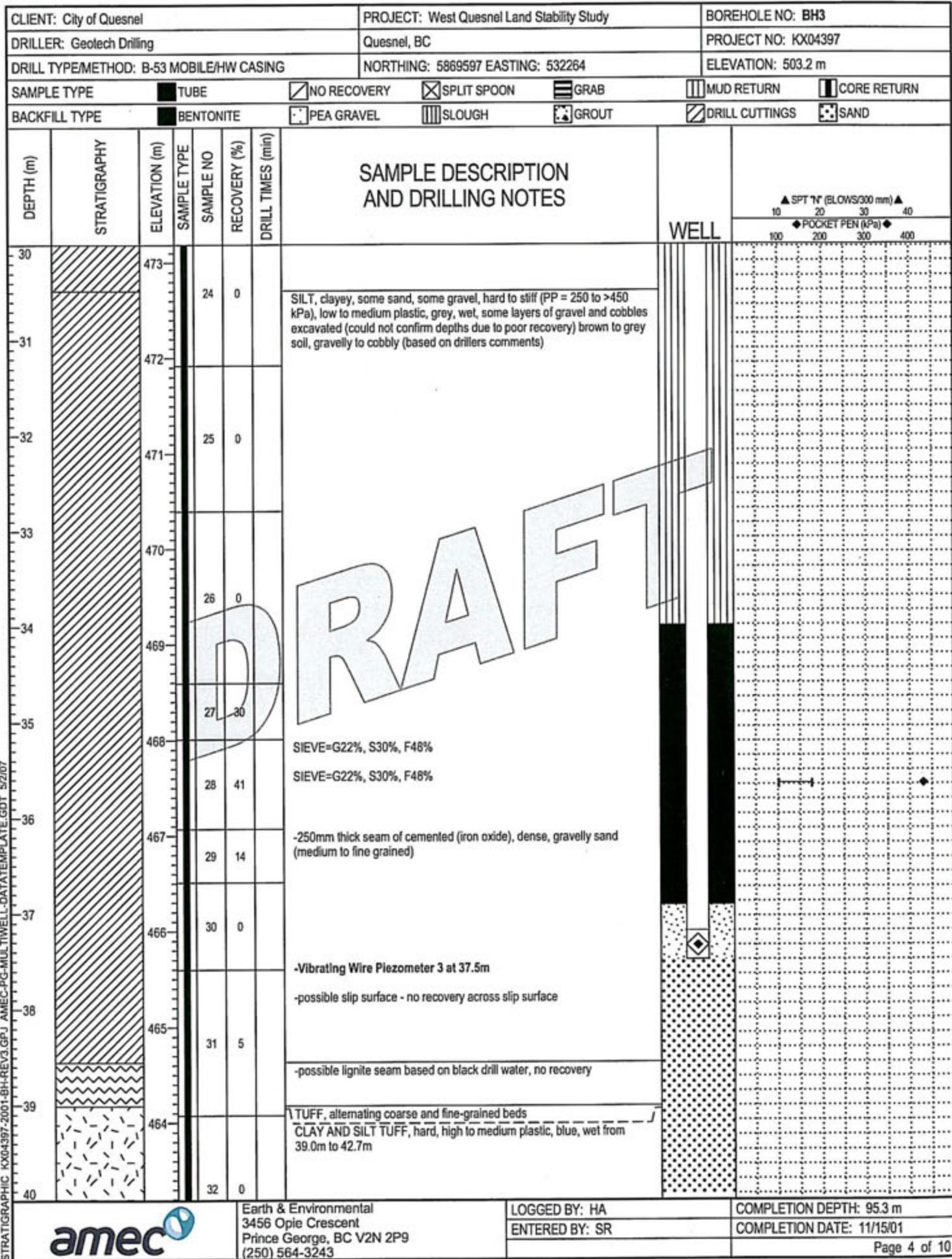
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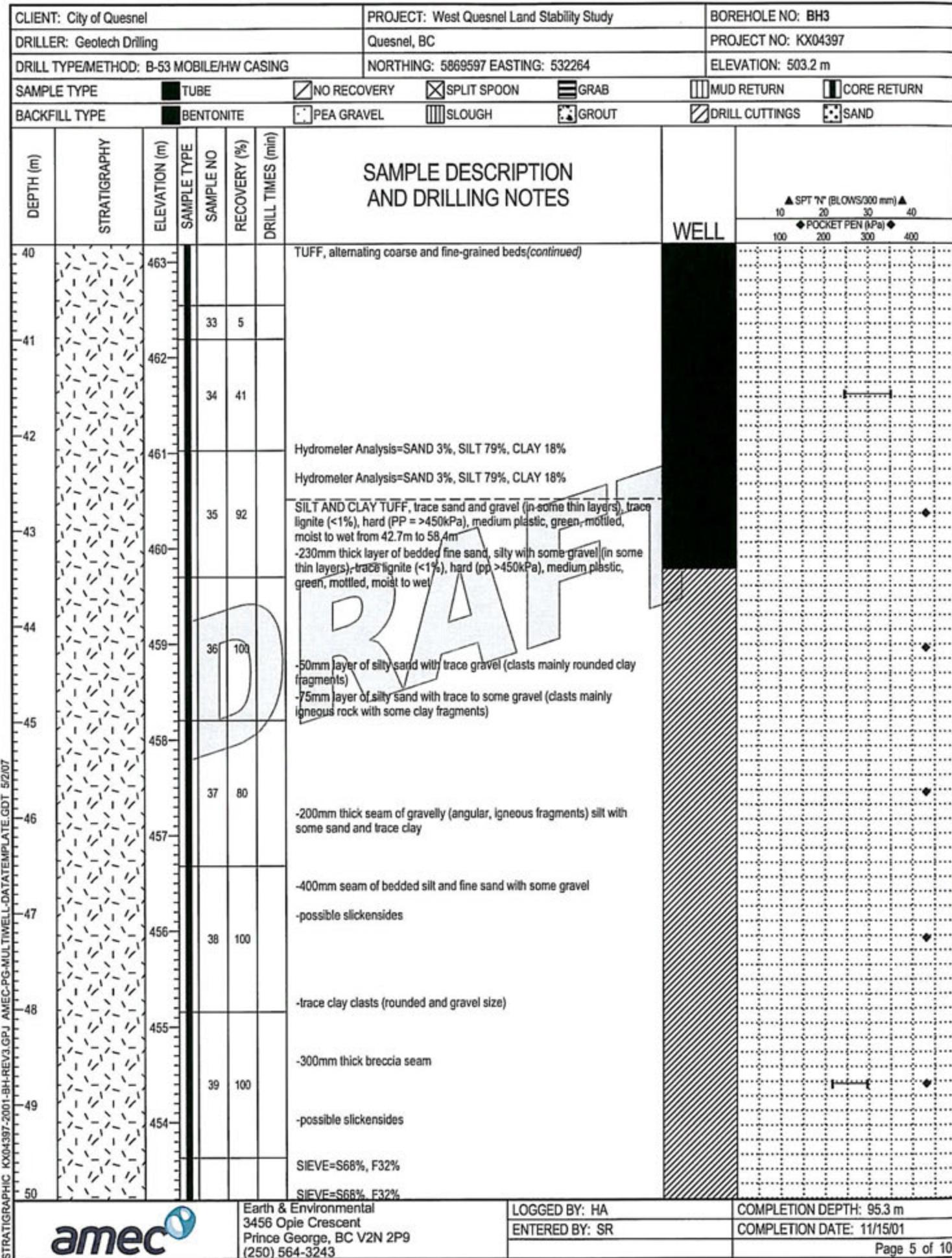
COMPLETION DATE: 8/8/03



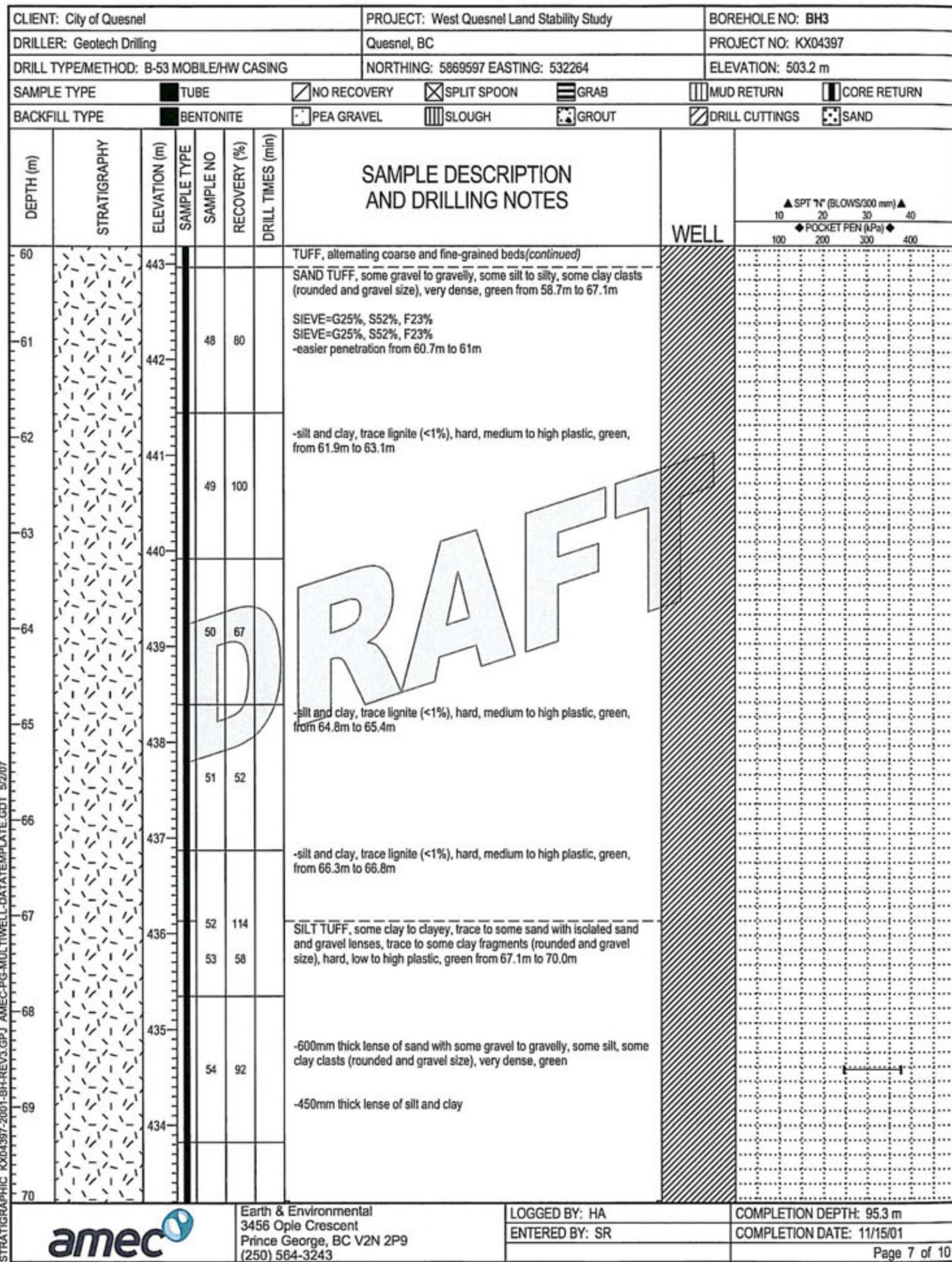


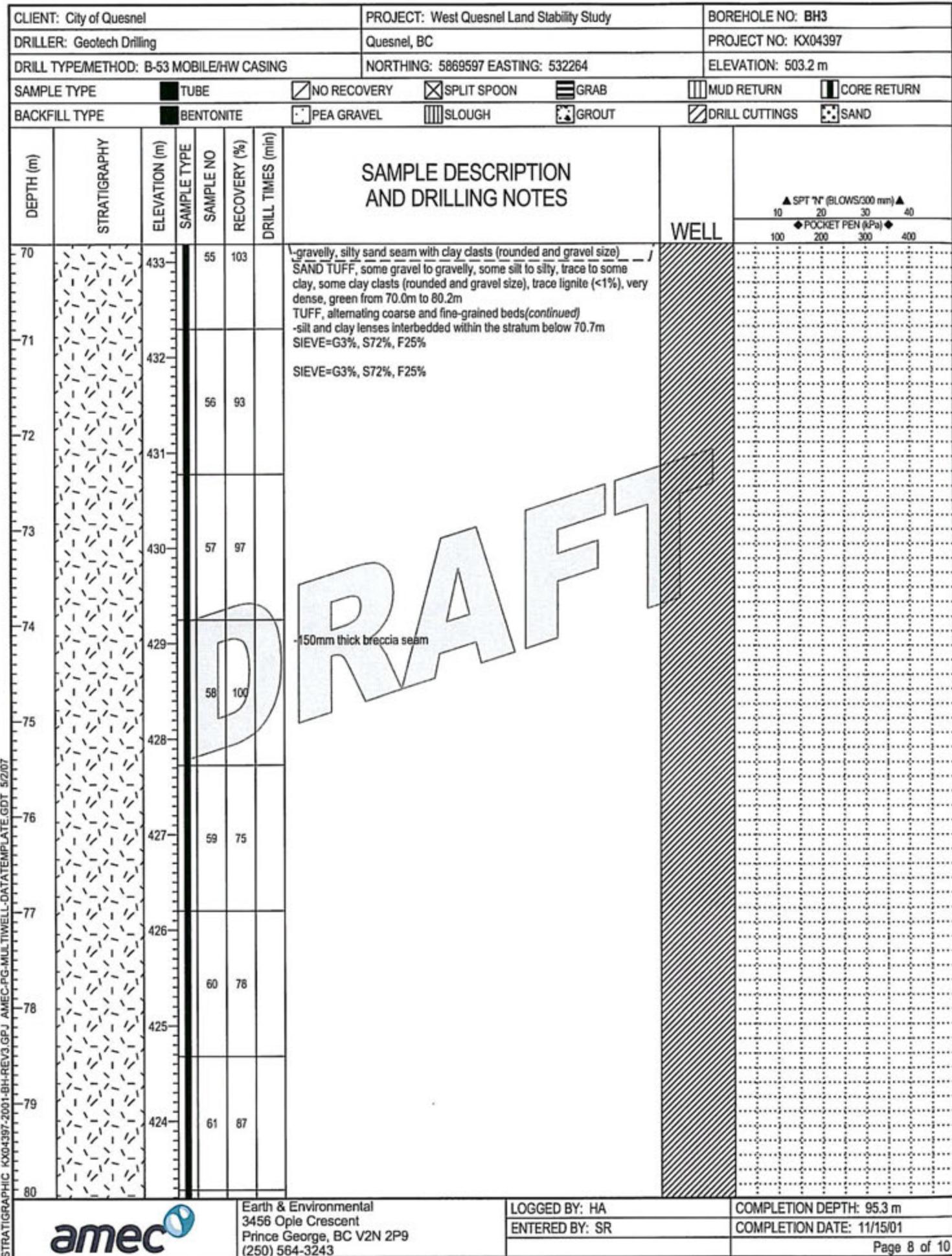






CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH3	
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING			NORTHING: 5869597 EASTING: 532264			ELEVATION: 503.2 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES
50		453		40	103		-600mm thick seam of trace to some sand, trace lignite (<1%) TUFF, alternating coarse and fine-grained beds(continued)
51		452		41	100		-100mm thick sandy silt lense -240mm thick silty sand lense
52		451		42	100		-70mm thick silty sand lense -240mm thick silty sand lense
53		450		43	99		-sand and gravel (some silt to silty) 0.4m thick Artesian water pressure (greater than 5.5m of head)
54		449		44	100		Artesian water pressure (greater than 5.5m of head above ground surface) -silt sand/sandy silt lense 1.5m thick Artesian water pressure (greater than 5.5m of head above ground surface)
55		448		45	100		Artesian water pressure (greater than 5.5m of head above ground surface) -150mm thick silty sand and gravel lense, trace gravel (rounded clay clasts) -trace lignite (<1%) Artesian water pressure (flow was approx. 0.5L/min)
56		447		46	100		Artesian water pressure (flow was approx. 0.5L/min) -possible slickensides -silty sand and gravel lense, trace gravel sized clay clasts, brown, 0.9m thick
57		446					
58		445					
59		444		47	100		-SANDSTONE, brown from 58.4m to 58.7m SILT AND CLAY TUFF, trace to some sand, trace to some clay fragments (gravel size), trace lignite (<1%), hard, medium to high plastic, green from 58.7m to 60.2m -300mm thick lense of silty sand with clay clasts (rounded and gravel size)
60							
STRATIGRAPHIC KX04397-2001-BH-REV3.GPJ AMEC:PG-MULTIWELL-DATATEMPLATE.GDT 5/2/07		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243			LOGGED BY: HA ENTERED BY: SR		COMPLETION DEPTH: 95.3 m COMPLETION DATE: 11/15/01 Page 6 of 10





CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH3
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING	NORTHING: 5869597 EASTING: 532264	ELEVATION: 503.2 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		

STRATIGRAPHY

ELEVATION (m)

SAMPLE TYPE

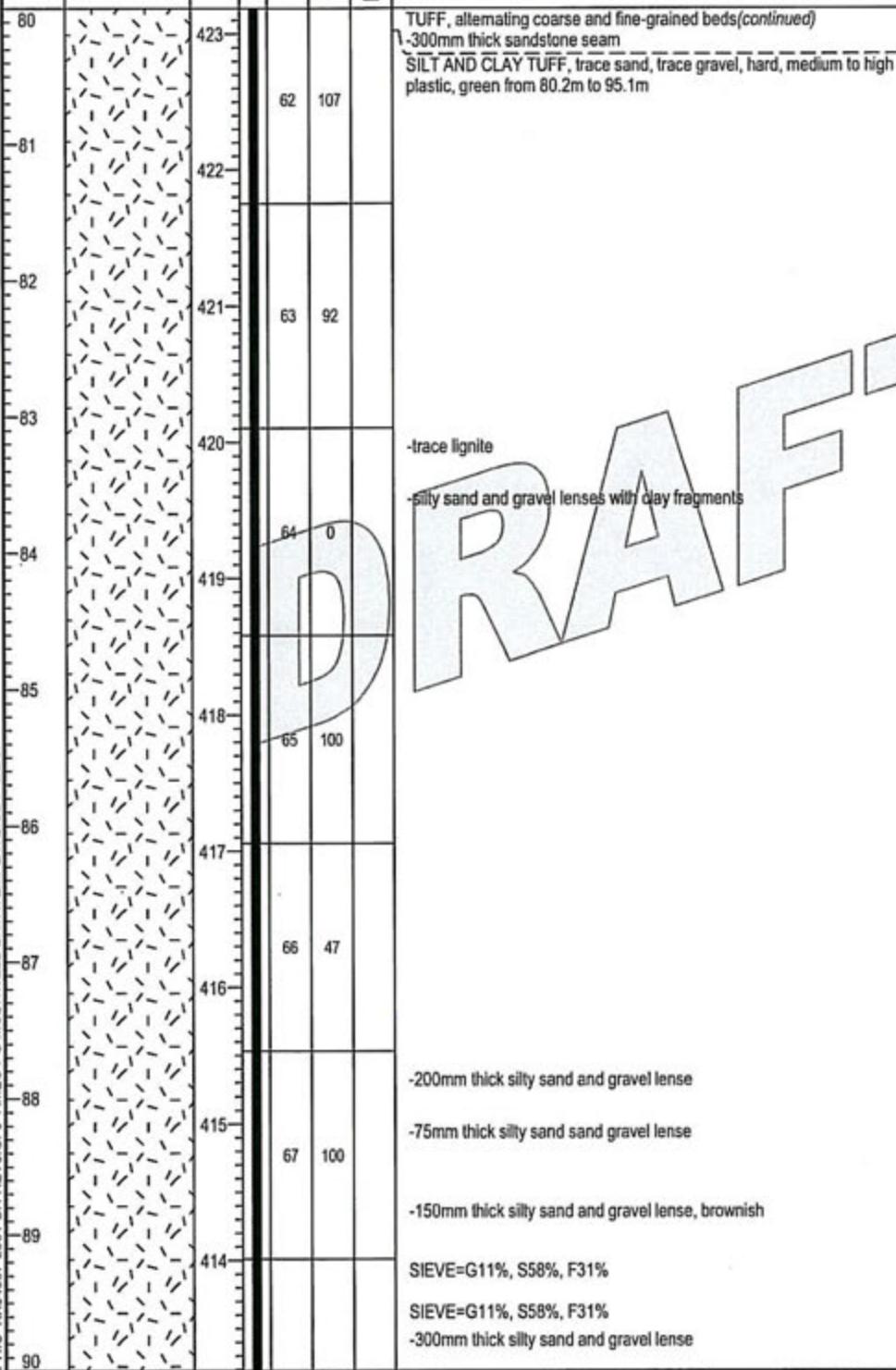
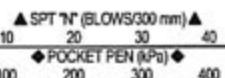
SAMPLE NO

RECOVERY (%)

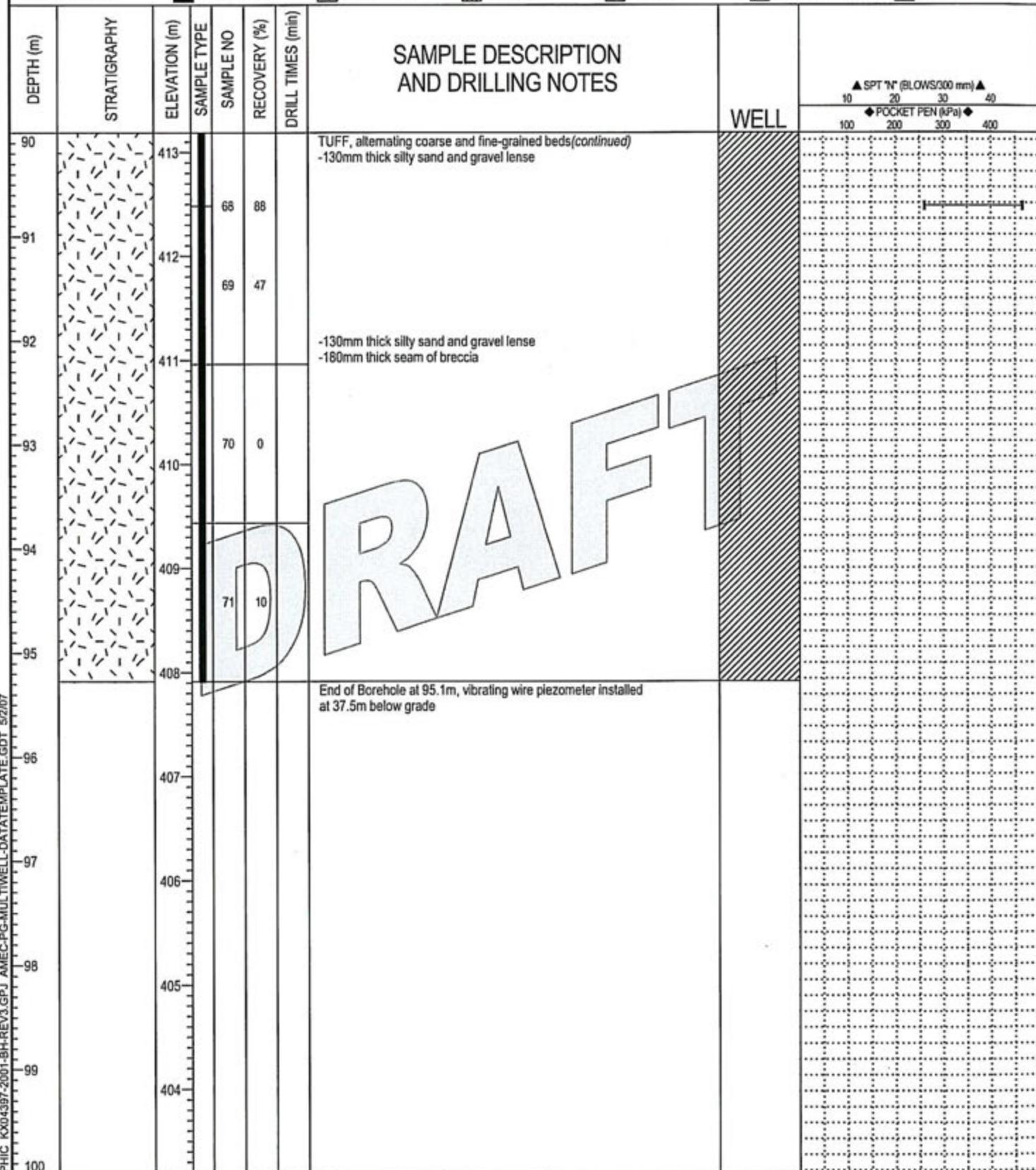
DRILL TIMES (min)

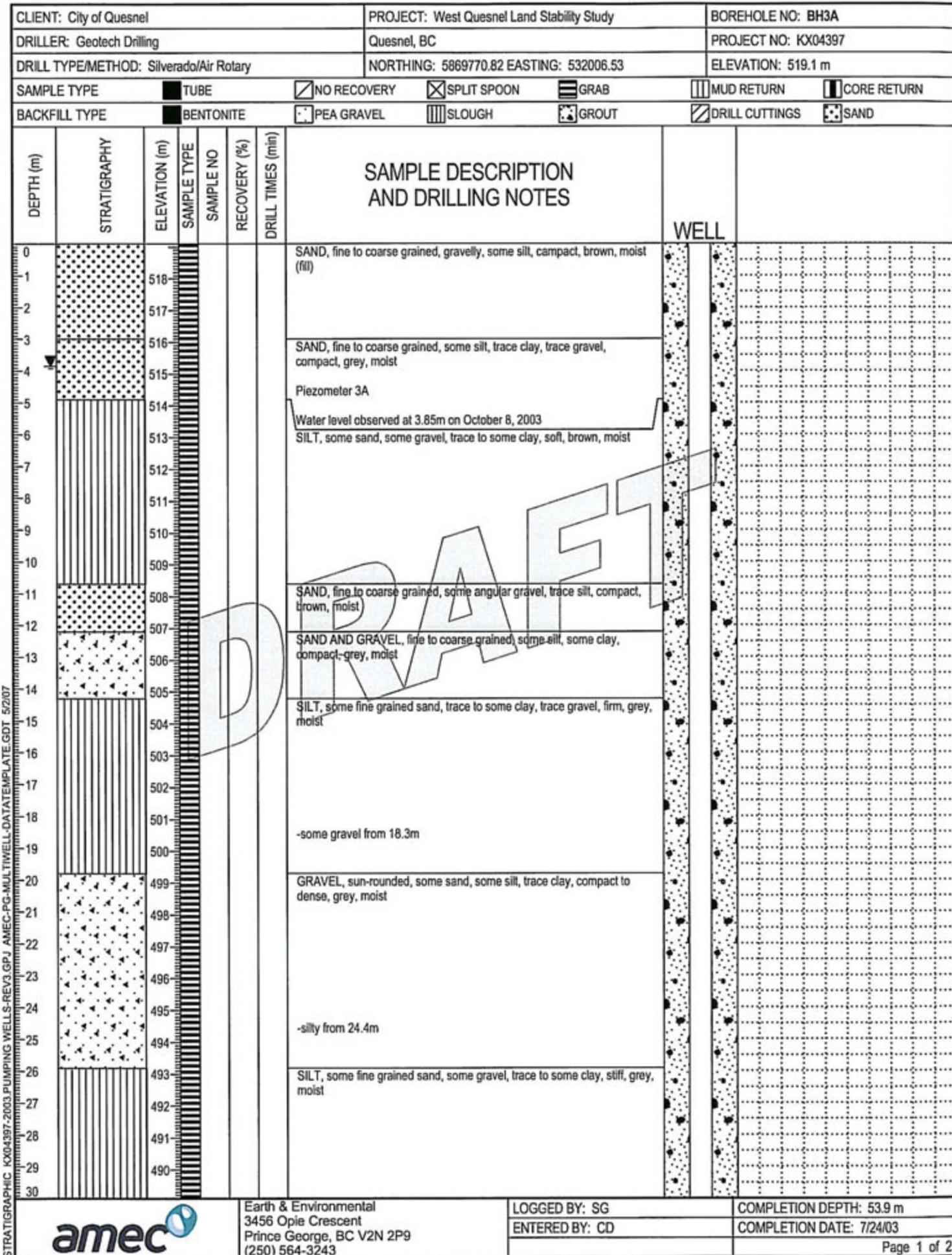
SAMPLE DESCRIPTION
AND DRILLING NOTES

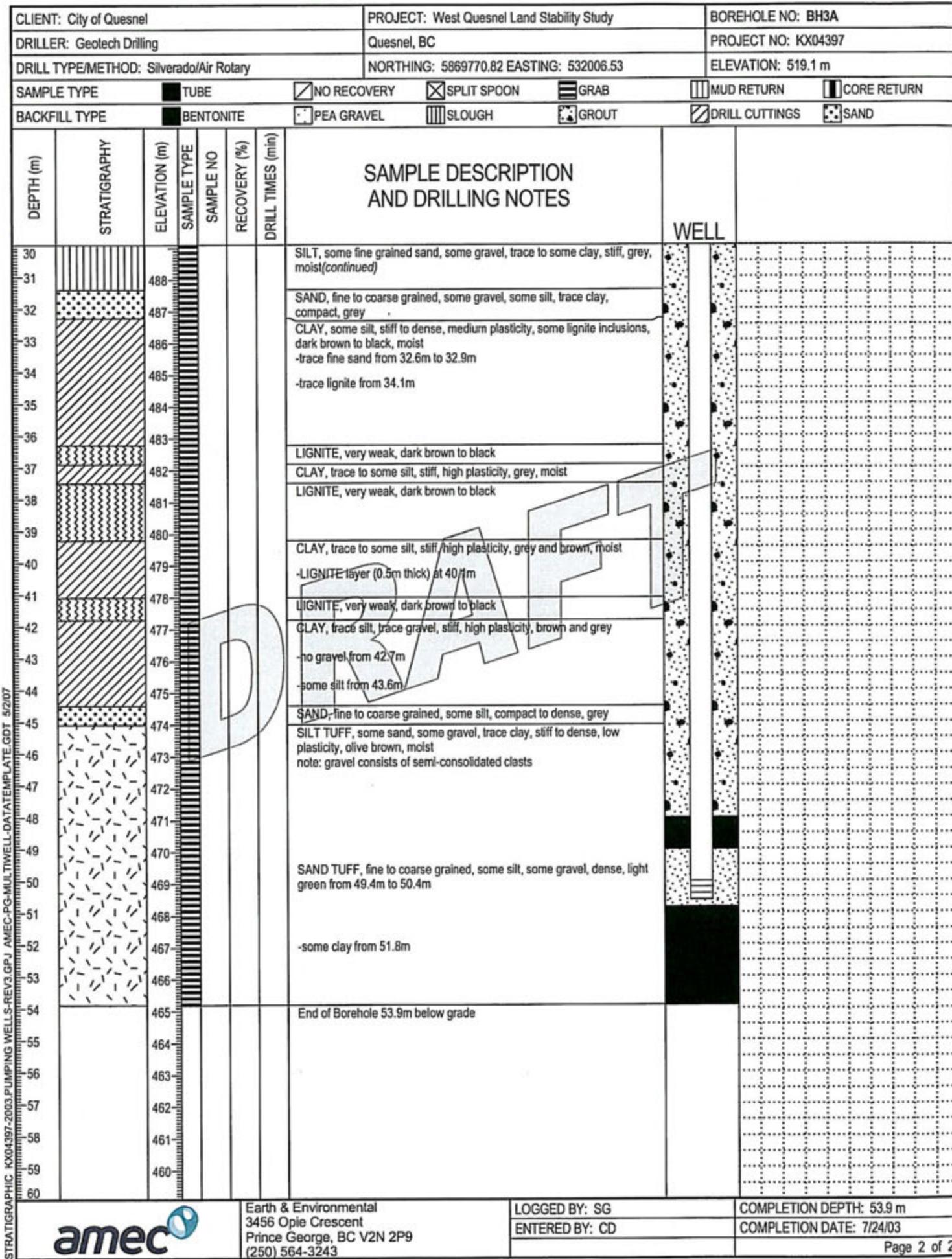
WELL



CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH3
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING	NORTHING: 5869597 EASTING: 532264	ELEVATION: 503.2 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		







CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH3BC	
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: Silverado/Air Rotary			NORTHING: 5869720.44 EASTING: 522005.24			ELEVATION: 519.1 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES
0		518					SAND AND GRAVEL, fine to coarse grained sand, trace silt, compact, brown, moist (fill)
1		517					SILT, some clay, some fine grained sand, trace to some gravel, soft, medium plasticity, brown, wet
2		516					-some sand to sandy, trace gravel, firm to stiff below 6.6m
3		515					
4		514					
5		513					
6		512					
7		511					
8		510					-0.3m thick layer of weak, friable, rock at 9.8m
9		509					
10		508					SAND AND GRAVEL, fine to coarse grained sand, some silt, trace clay, compact, brown, wet
11		507					
12		506					SILT, clayey, trace to some fine grained sand, trace gravel, firm, medium plasticity, grey, wet
13		505					
14		504					SAND, fine grained, silty, trace clay, compact to dense, grey, wet
15		503					
16		502					
17		501					
18		500					
19		499					SILT, clayey, trace gravel, firm, grey
20		498					
21		497					SAND, fine grained, silty, some gravel, trace clay, dense, brown
22		496					SILT, clayey, sandy, some gravel, stiff to hard, high plasticity, trace lignite inclusions, brown
23		495					
24		494					
25		493					SAND AND GRAVEL, trace to some silt, dense, brown
26		492					
27		491					
28		490					
29		490					
30		490					
		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243			LOGGED BY: SG ENTERED BY: CD	COMPLETION DEPTH: 44.8 m COMPLETION DATE: 7/22/03	Page 1 of 2

STRATIGRAPHIC LOG KX04397-2003 PUMPING WELLS-REV3 GPJ AMEC-PG-MULTIWELL-DATAFILE.GDT 5/2007

CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH3BC					
DRILLER: Geotech Drilling		Quesnel, BC		PROJECT NO: KX04397					
DRILL TYPE/METHOD: Silverado/Air Rotary		NORTHING: 5869720.44 EASTING: 522005.24		ELEVATION: 519.1 m					
SAMPLE TYPE	TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN				
BACKFILL TYPE	BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS				
					<input checked="" type="checkbox"/> CORE RETURN				
					<input checked="" type="checkbox"/> SAND				
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES		WELL
30									
31		488					GRAVEL, sub-rounded, some sand, trace silt, dense, grey		
32		487							
33		486					-Piezometer C set between 32.0m and 33.5m, static water level measured at 1.82m below grade (October 8, 2003)		
34		485					LIGNITE, very weak, dark brown to black		
35		484					CLAY, some silt, trace sand, hard, high plasticity, grey and black		
36		483					LIGNITE, very weak, dark brown to black		
37		482					CLAY, some silt, hard, high plasticity, grey to light brown		
38		481							
39		480					LIGNITE, very weak, dark brown to black		
40		479					-Piezometer B set between 38.7m and 40.2m, static water level measured at 1.14m below grade (October 8, 2003)		
41		478					-0.3m thick clay layer from 39.3m to 39.6m		
42		477					CLAY, some silt, hard, high plasticity, grey to light brown		
43		476					LIGNITE, very weak, dark brown to black		
44		475					CLAY, trace to some silt, hard, high plasticity, grey		
45		474					CLAY TUFF, some silt, trace sand, dense, high plasticity, grey		
46		473					End of Borehole 44.8m below grade		
47		472							
48		471							
49		470							
50		469							
51		468							
52		467							
53		466							
54		465							
55		464							
56		463							
57		462							
58		461							
59		460							

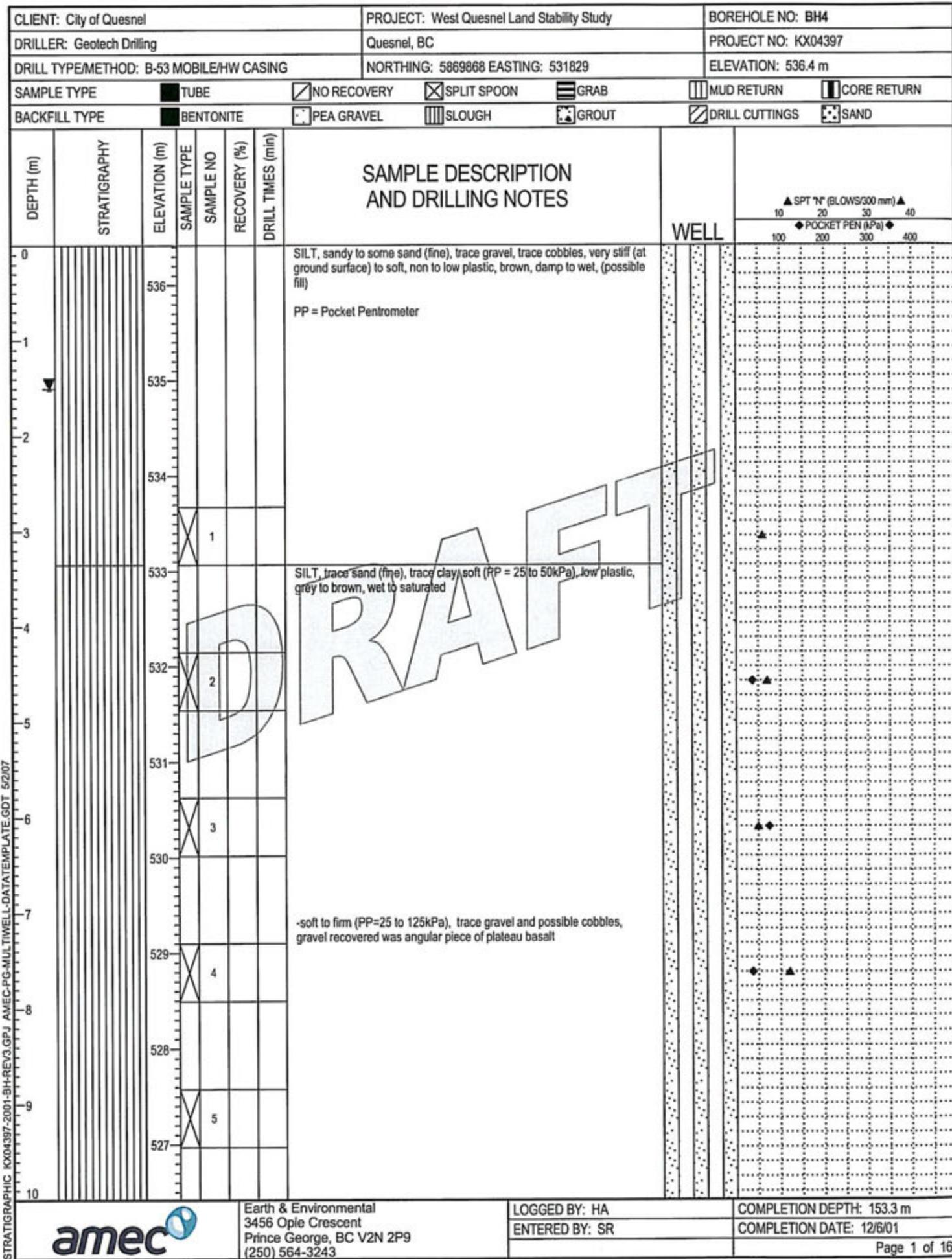
SAMPLE DESCRIPTION
AND DRILLING NOTES

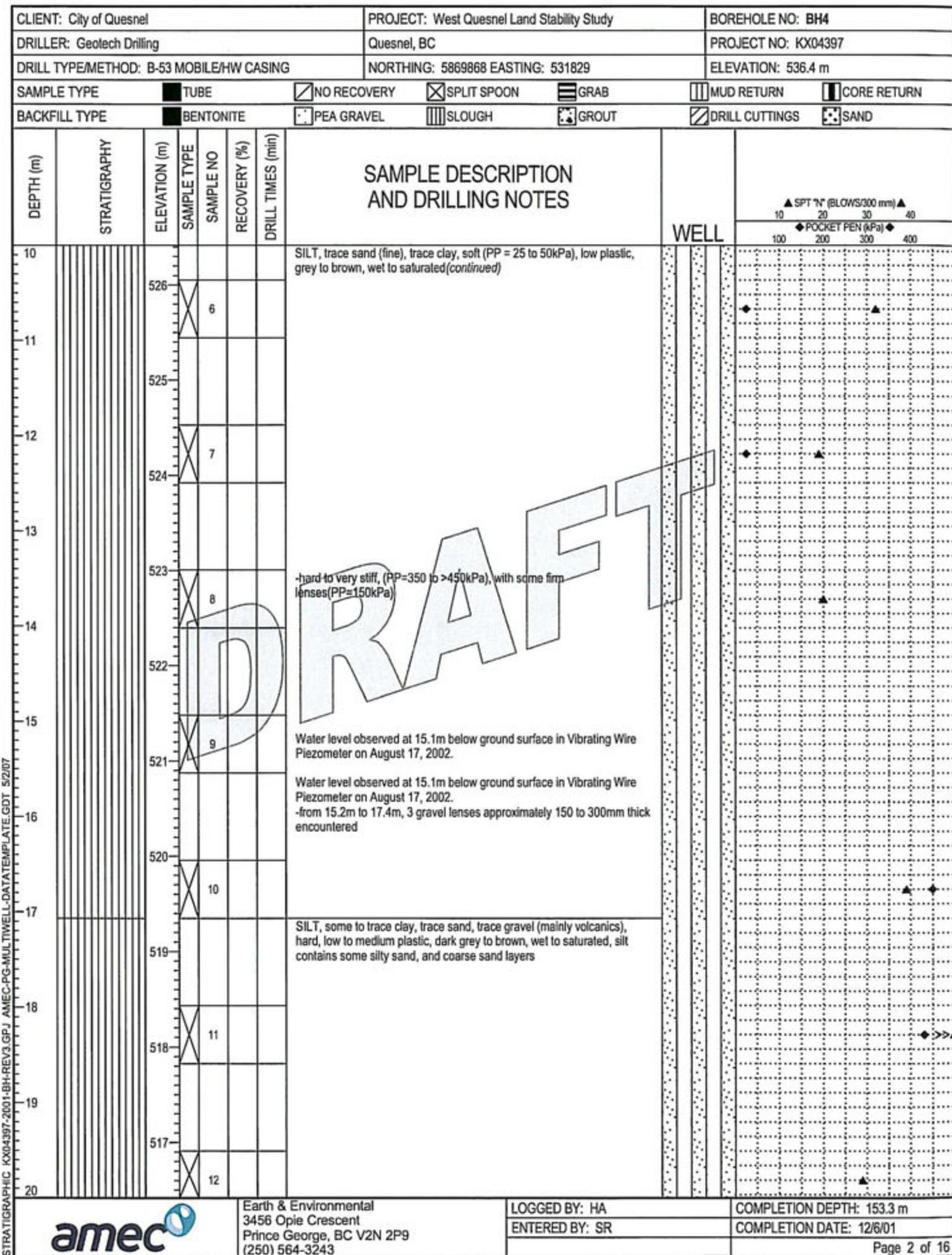
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: SG
ENTERED BY: CD

COMPLETION DEPTH: 44.8 m
COMPLETION DATE: 7/22/03

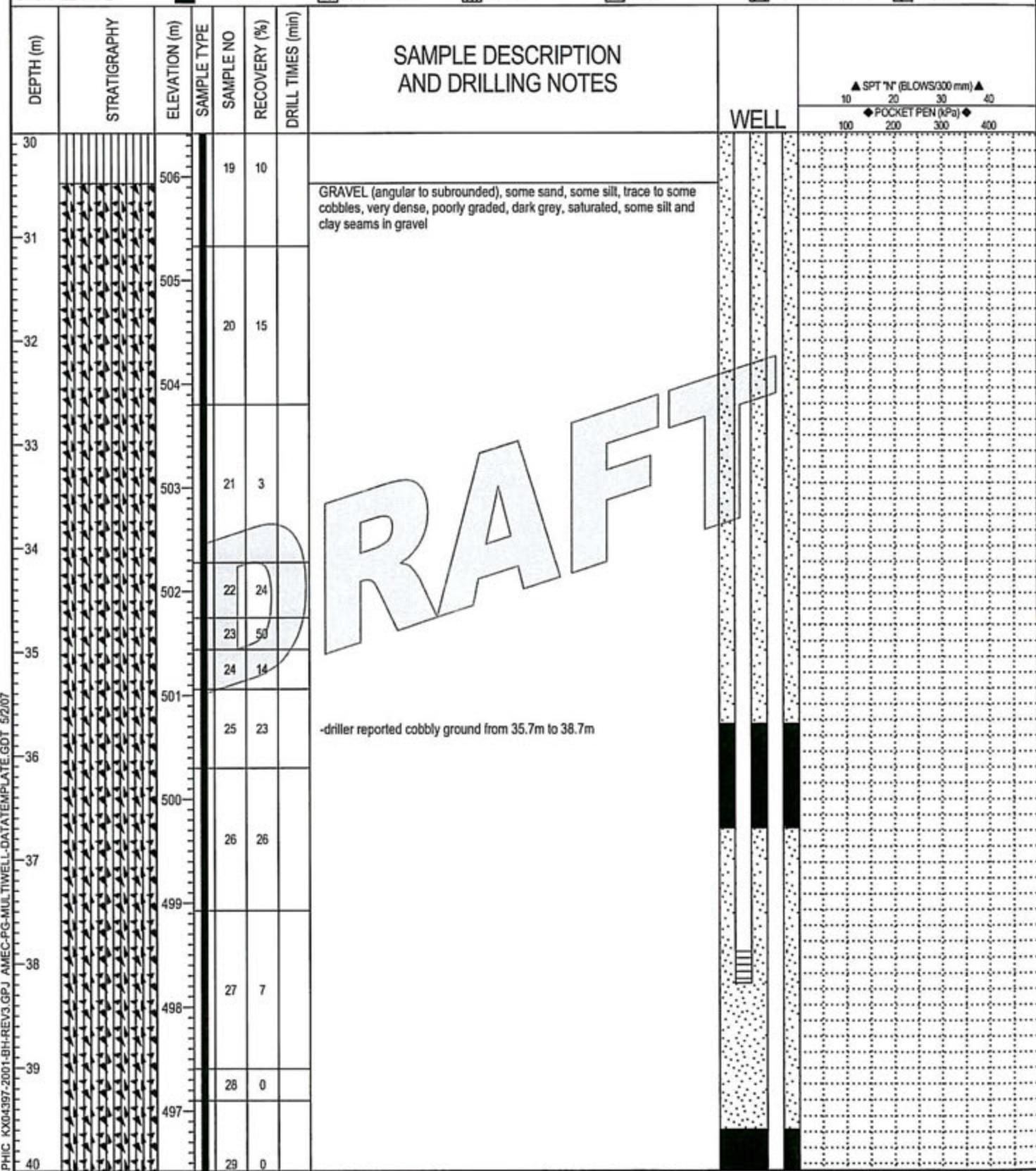
Page 2 of 2





CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH4		
DRILLER: Geotech Drilling		Quesnel, BC		PROJECT NO: KX04397		
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING		NORTHING: 5869868 EASTING: 531829		ELEVATION: 536.4 m		
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB				MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT				DRILL CUTTINGS <input type="checkbox"/> SAND		
DEPTH (m)	STRATIGRAPHY	SAMPLE DESCRIPTION AND DRILLING NOTES				WELL
		ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	
20						SILT, some to trace clay, trace sand, trace gravel (mainly volcanics), hard, low to medium plastic, dark grey to brown, wet to saturated, silt contains some silty sand, and coarse sand layers(continued)
21						
22						
23						-150mm thick gravel layer encountered
24						-150mm thick gravel layer encountered
25						-trace lignite in soil
26						
27						
28						
29						
30						
STRATIGRAPHIC LOG SHEET KX04397-2001BH-REV3.GPJ AMEC-PG-MULTIWELL.DATATEMPLATE.GDT 5/2/07						▲ SPT 'N' (BLOWS/300 mm) ▲ 10 20 30 40 ◆ POCKET PEN (kPa) ◆ 100 200 300 400
amec		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243		LOGGED BY: HA ENTERED BY: SR		COMPLETION DEPTH: 153.3 m COMPLETION DATE: 12/6/01
						Page 3 of 16

CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH4
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING	NORTHING: 5869868 EASTING: 531829	ELEVATION: 536.4 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



STRATIGRAPHIC KX04397-2001-BH-REV3 GPU AMEC-PG-MULTIWELL-DATATEMPLATE.GDT 5/2/07



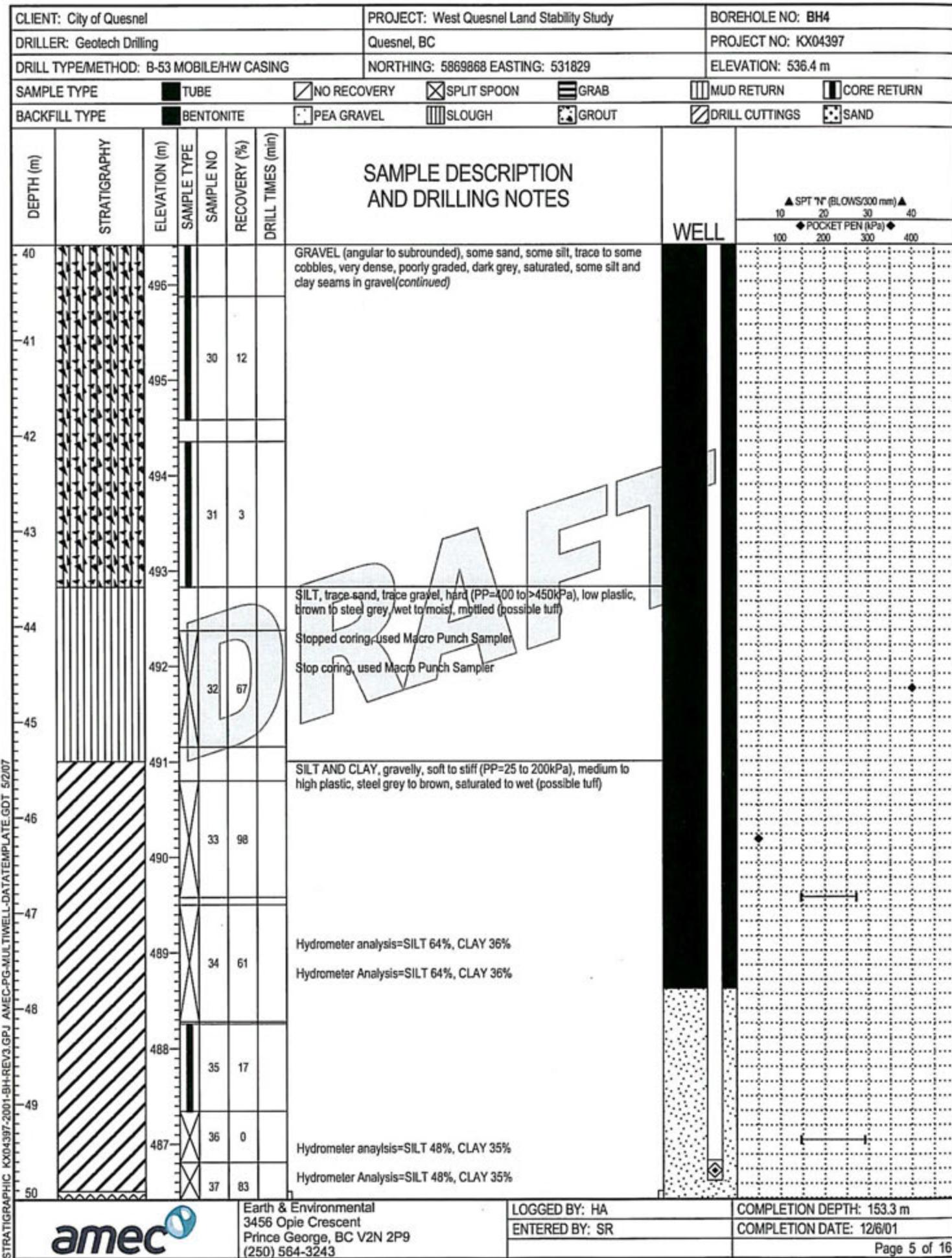
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

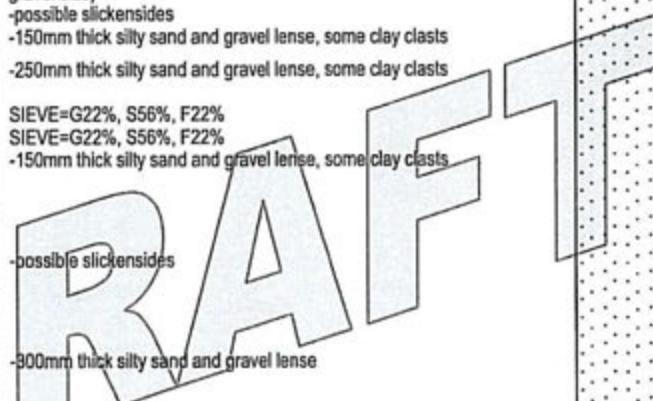
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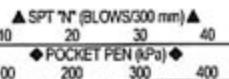
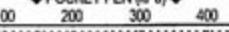
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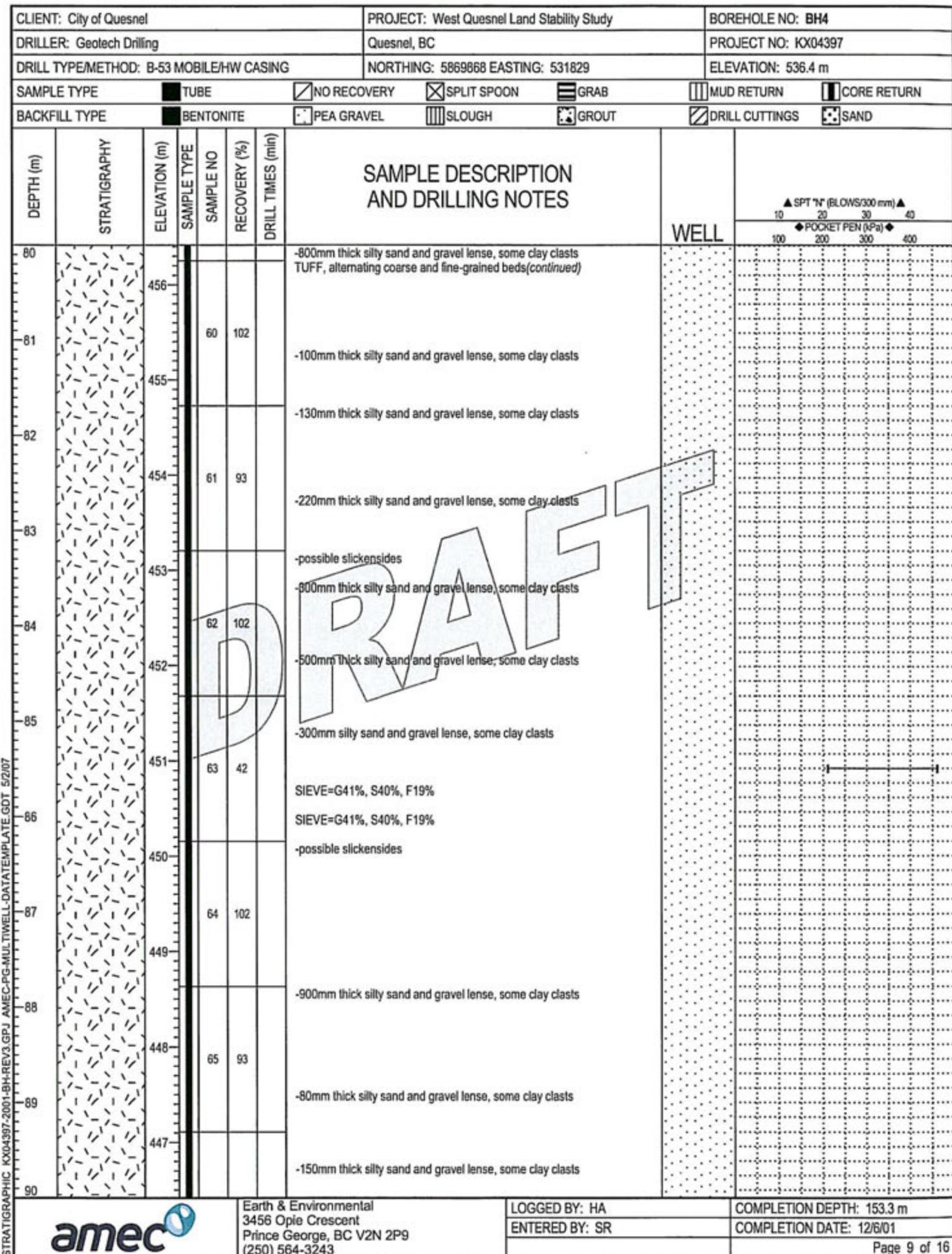
COMPLETION DEPTH: 153.3 m

COMPLETION DATE: 12/6/01

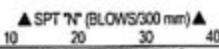
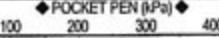


CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH4	
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING				NORTHING: 5869868 EASTING: 531829				ELEVATION: 536.4 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN			
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND			
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES		
50							-X-Ray Diffraction test at 49.68m X-Ray Diffraction Test at 49.68m -Vibrating Wire Piezometer 4 at 49.8m Started coring Start Coring PEAT, soft, fibrous(continued) TUFT, alternating coarse and fine-grained beds SILT AND CLAY TUFT, trace to some sand, trace gravel, hard, medium to high plastic, green from 50.1m to 92.6m -silty sand lenses with some gravel and some clay clasts (rounded and gravel size) -possible slickensides -150mm thick silty sand and gravel lense, some clay clasts -250mm thick silty sand and gravel lense, some clay clasts SIEVE=G22%, S56%, F22% SIEVE=G22%, S56%, F22% -150mm thick silty sand and gravel lense, some clay clasts -possible slickensides -300mm thick silty sand and gravel lense -150mm thick silty sand and gravel lense, some clay clasts -150mm thick silty sand and gravel lense, some clay clasts, trace lignite (<1%) -possible slickensides		
51		486		38	41				
52		485		39	100				
53		484		40	112				
54		483		41	68				
55		482		42	107				
56		481		43	58				
57		480		44	98				
58		479					-possible slickensides		
59		478							
60		477							
									
				Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: HA ENTERED BY: SR	
								COMPLETION DEPTH: 153.3 m COMPLETION DATE: 12/6/01	
Page 6 of 16									

CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH4				
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397				
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING				NORTHING: 5869868 EASTING: 531829				ELEVATION: 536.4 m				
SAMPLE TYPE	TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN						
BACKFILL TYPE	BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND						
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES					
60		476										
61		475		46	77		TUFF, alternating coarse and fine-grained beds(continued) -possible slickensides					
62		474		47	90		-80mm thick silty sand and gravel lens, some clay clasts -800mm thick silty sand and gravel lens, some clay clasts					
63		473					-possible slickensides					
64		472		48	100		-900mm thick silty sand and gravel lens, some clay clasts					
65		471		49	103		-300mm thick silty sand and gravel lens, some clay clasts					
66		470		50	100		-possible slickensides /possible lignite					
67		469		51	93		-black drill water return and soft drilling conditions were reported from 64.9m to 65.8m, may indicate lignite layer					
68		468					-300mm thick silty sand and gravel lens, some clay clasts					
69		467					-possible slickensides					
70		466										
WELL												
												
												
												

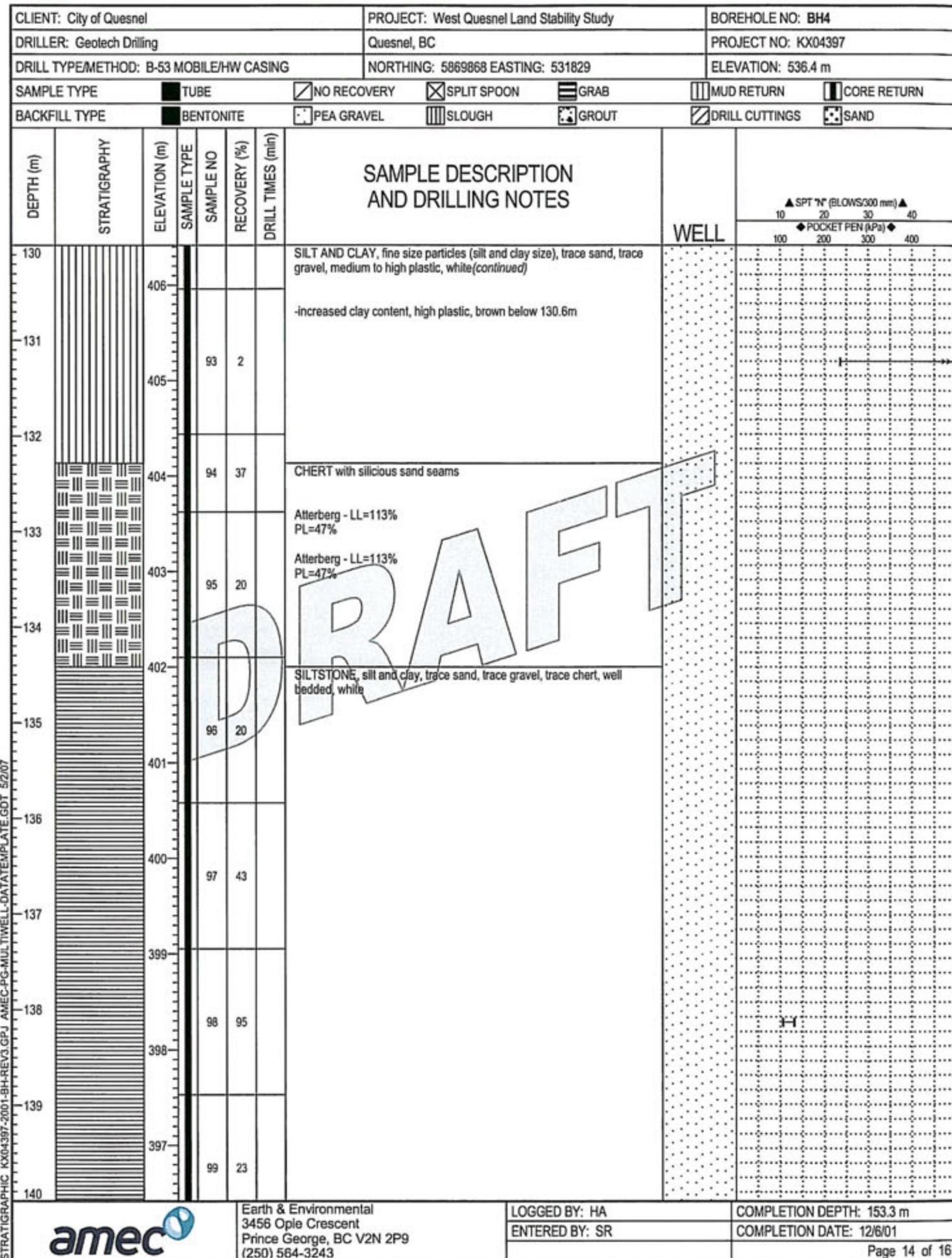


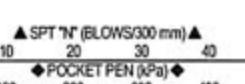
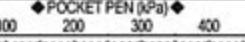
CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH4	
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING			NORTHING: 5869868 EASTING: 531829			ELEVATION: 536.4 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES
90				66	103		TUFF, alternating coarse and fine-grained beds (continued) -100mm thick silty sand and gravel lens, some clay clasts
91		446		67	105		-500mm thick silty sand and gravel lens, some clay clasts
92		445					-50mm sandstone piece encountered
93		444					SAND AND GRAVEL TUFF, silty, some clay clasts (rounded and gravel size), poorly graded, brown from 92.6m to 124.7m
94		443		68	100		
95		442		69	53		
96		441					
97		440					-300mm thick clayey silt lens, trace sand
98		439		70	70		
99		438		71	13		
100		437					
STRATIGRAPHIC KX04397-2001-BH-REV3.GPJ AMEC-PG-MULTIWELL-DATATEMPLATE.GDT 5/2/07							
amec	Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243	LOGGED BY: HA ENTERED BY: SR	COMPLETION DEPTH: 153.3 m COMPLETION DATE: 12/6/01 Page 10 of 16				

CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH4	
DRILLER: Geotech Drilling		Quesnel, BC		PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING		NORTHING: 5869868 EASTING: 531829		ELEVATION: 536.4 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> CORE RETURN
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)
					DRILL TIMES (min)
SAMPLE DESCRIPTION AND DRILLING NOTES					
100		436			TUFF, alternating coarse and fine-grained beds(continued)
101		435		72	13
102		434			-300mm thick clayey silt lens, trace sand
103		433			-430mm thick clayey silt lens, trace sand
104		432		73	98
105		431		74	12
106		430		75	13
107		429			SIEVE=G22%, S66%, F12%
108		428			SIEVE=G22%, S66%, F12%
109		427		76	63
110					-300mm thick clayey silt lens, trace sand
				77	46
				78	23
WELL					
					
 					
STRATIGRAPHIC KX04397-2001-BH-REV3 GPJ AMEC-PG-MULTIWELL-DATAFILE.G01 5/2/2001					
		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243		LOGGED BY: HA	COMPLETION DEPTH: 153.3 m
				ENTERED BY: SR	COMPLETION DATE: 12/6/01
Page 11 of 16					

CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH4				
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397				
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING				NORTHING: 5869868 EASTING: 531829				ELEVATION: 536.4 m				
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN						
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input checked="" type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND						
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES					
110		426					TUFF, alternating coarse and fine-grained beds (continued)					
111		425		79	43							
112		424										
113		423		80	100		-200mm thick clayey silt lens, trace sand					
114		422		81	32		-450mm thick clayey silt lens, trace sand					
115		421		82	53							
116		420										
117		419		83	15							
118		418										
119		417		84	64		-150mm thick possible sandstone encountered					
120												
▲ SPT "N" (BLOWS/300 mm) ▲												
10 20 30 40												
◆ POCKET PEN (kPa) ◆												
100 200 300 400												
DRAFT												

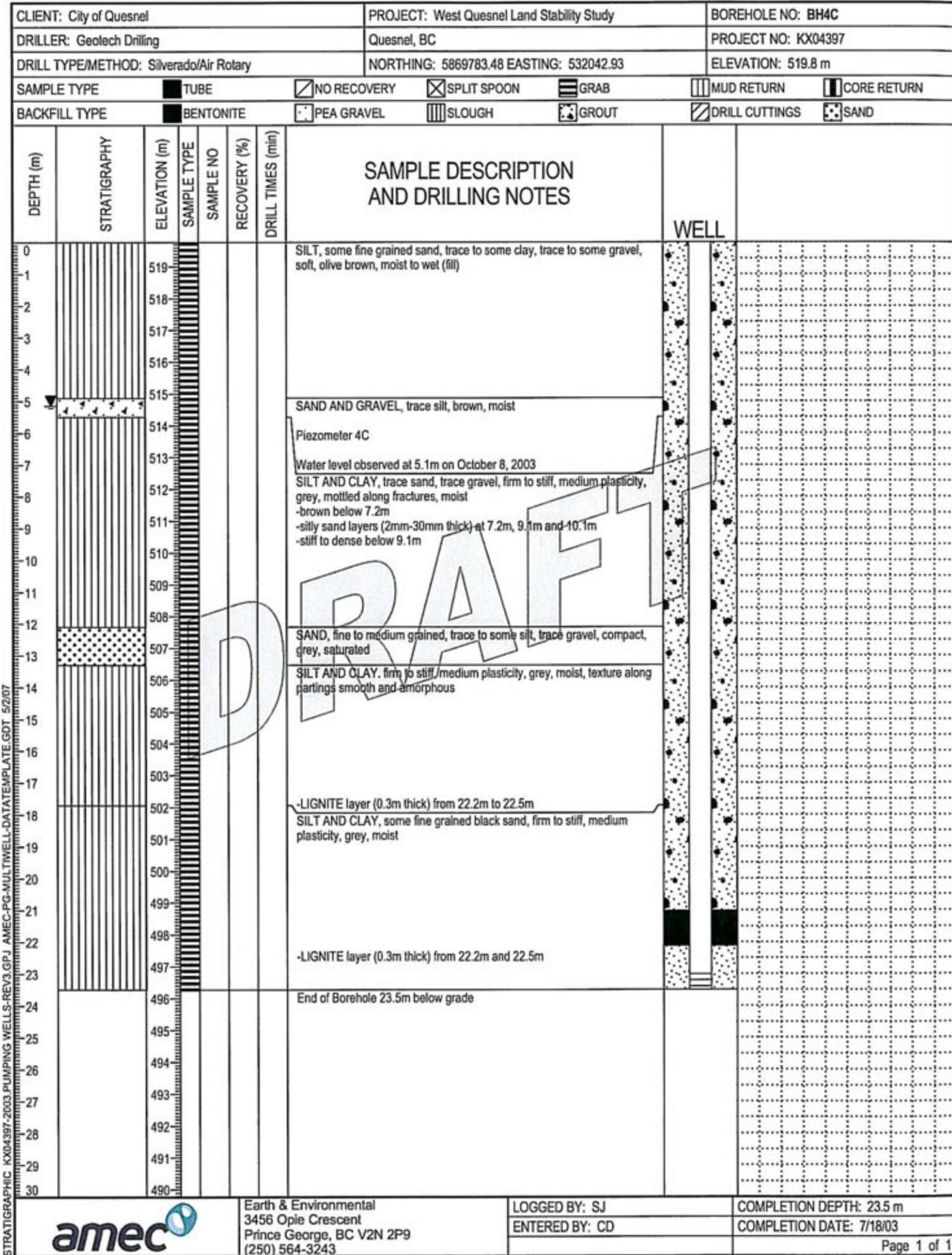
CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH4	
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING			NORTHING: 5869868 EASTING: 531829			ELEVATION: 536.4 m	
SAMPLE TYPE		<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN
BACKFILL TYPE		<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND
DEPTH (m)	STRATIGRAPHY	SAMPLE DESCRIPTION AND DRILLING NOTES				WELL	
		ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)		
120			85	54		TUFF, alternating coarse and fine-grained beds(continued)	
			86	47		-50mm diameter vesicular clast of tephra, brown on inside, weathered to white on outside	
121			87	23			
122			88	100		-possible breccia layer (approx. 0.1m thick)	
123			89	103		RAFT	
124			90	100		SILT AND CLAY, fine size particles (silt and clay size), trace sand, trace gravel, medium to high plastic, white	
125			91	88			
126			92	75		-layers of chert below 126.2m depth	
127							
128							
129							
130							
STRATIGRAPHIC LOG KX04397-2001-BH-REV3.GPJ AMEC-PG-MULTIWELL.DATEMPLATE.GDT 5/2/07							
		Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243			LOGGED BY: HA ENTERED BY: SR		COMPLETION DEPTH: 153.3 m COMPLETION DATE: 12/6/01
Page 13 of 16							



CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH4				
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397				
DRILL TYPE/METHOD: B-53 MOBILE/HW CASING				NORTHING: 5869868 EASTING: 531829				ELEVATION: 536.4 m				
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN						
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND						
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES					
140		396					SILTSTONE, silt and clay, trace sand, trace gravel, trace chert, well bedded, white (continued)					
141		395		100	8							
142		394		101	100							
143		393		102	52							
144		392		103	53							
145		391										
146		390		104	102							
147		389										
148		388		105	102							
149		387		106	60							
150												
WELL												
												
 ▲ SPT "N" (BLOWS/300 mm) ▲ 10 20 30 40  ◆ POCKET PEN (kPa) ◆ 100 200 300 400												
STRATIGRAPHIC KX04397-2001-BH-REV3.GPJ AMEC-PG-MULTIWELL-DATATEMPLATE.GDT 5/2/07												
			Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: HA	COMPLETION DEPTH: 153.3 m				
							ENTERED BY: SR	COMPLETION DATE: 12/6/01				
								Page 15 of 16				

CLIENT: City of Quesnel		PROJECT: West Quesnel Land Stability Study		BOREHOLE NO: BH4AB				
DRILLER: Geotech Drilling		Quesnel, BC		PROJECT NO: KX04397				
DRILL TYPE/METHOD: Silverado/Air Rotary		NORTHING: 5869782.79 EASTING: 532040.98		ELEVATION: 519.8 m				
SAMPLE TYPE	TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN			
BACKFILL TYPE	BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS			
<input type="checkbox"/> CORE RETURN					<input type="checkbox"/> SAND			
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES	WELL
0		519					SILT, some fine grained sand, trace to some clay, trace to some gravel, soft, medium plasticity, olive brown, moist to wet	
1		518						
2		517						
3		516						
4		515					SAND AND GRAVEL, trace silt, brown, moist	
5		514					SILT AND CLAY, trace sand, trace gravel, firm to stiff, medium plasticity, grey, mottled along fractured, moist	
6		513					-brown below 7.2m	
7		512					-silty sand layers (2mm-30mm thick) at 7.2m, 9.1m and 10.1m	
8		511					-stiff to dense below 9.1m	
9		510						
10		509						
11		508						
12		507					SAND, fine to medium grained, trace to some silt, trace gravel, compact, grey, saturated	
13		506					SILT AND CLAY, firm to stiff/medium plasticity, grey, moist, texture along partings smooth and amorphous	
14		505						
15		504						
16		503						
17		502					SILT AND CLAY, some fine grained black sand, firm to stiff, medium plasticity, grey, moist	
18		501						
19		500						
20		499						
21		498						
22		497					-LIGNITE layer (0.3m thick) between 22.2m and 22.5m	
23		496					LIGNITE, very weak, dark brown to black	
24		495					SILT AND CLAY, trace fine grained black sand, firm to stiff, medium plasticity, grey, moist	
25		494					-no sand from 25.9m	
26		493						
27		492						
28		491						
29		490						
STRATIGRAPHIC LOG KX04397-2003.PUMPING.WELLS-REV3.GPJ AMEC-PG-MULTIWELL-DATAFILE TEMPLATE GDT 5/2007								
Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243			LOGGED BY: SJ		COMPLETION DEPTH: 49.7 m			
			ENTERED BY: CD		COMPLETION DATE: 7/17/03			
					Page 1 of 2			

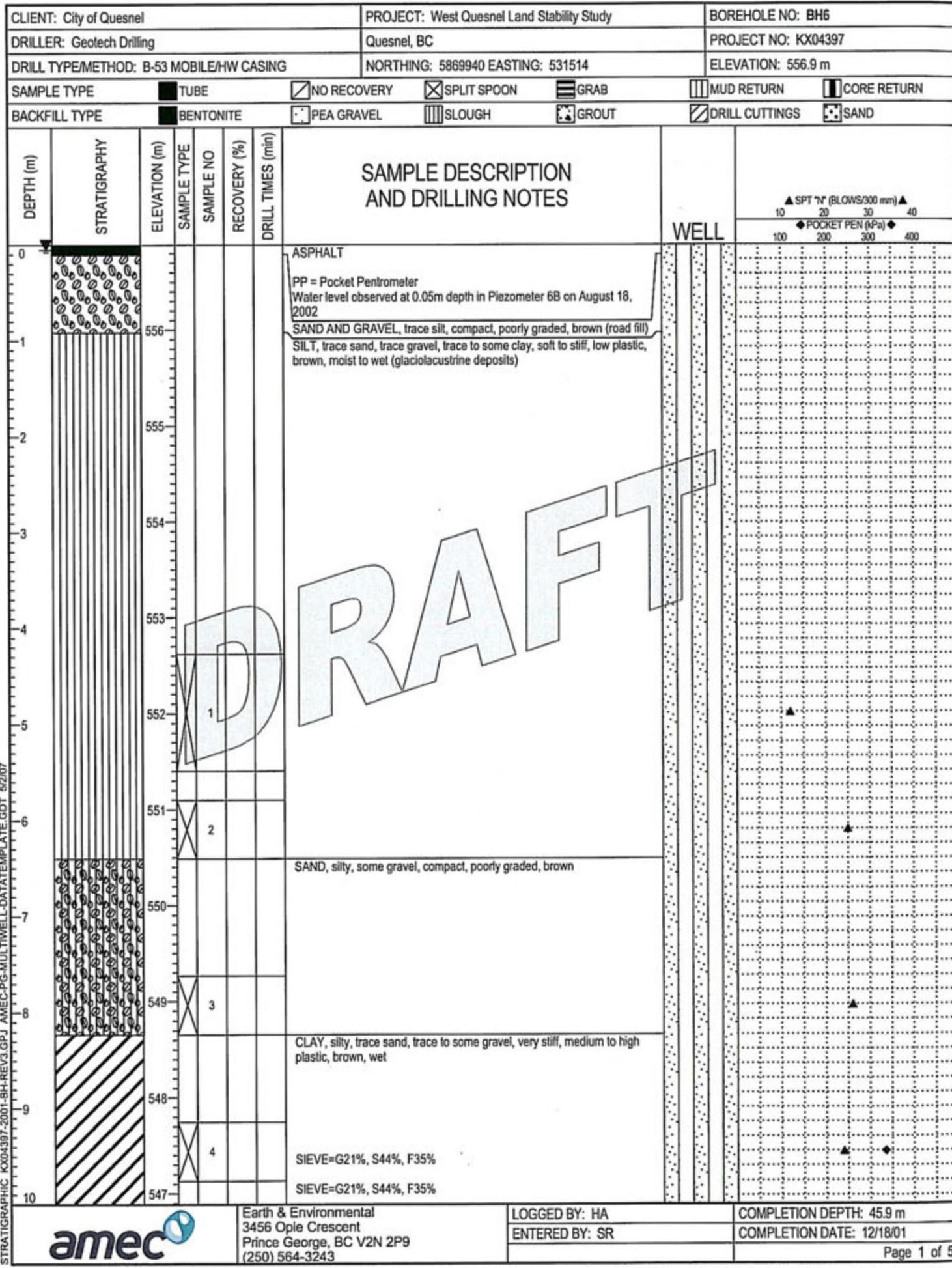
CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: BH4AB	
DRILLER: Geotech Drilling			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: Silverado/Air Rotary			NORTHING: 5869782.79 EASTING: 532040.98			ELEVATION: 519.8 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES
30		489					SILT AND CLAY, trace fine grained black sand, firm to stiff, medium plasticity, grey, moist (continued) -LIGNITE layer (0.3m thick) from 30.5m to 30.8m
31		488					
32		487					
33		486					
34		485					
35		484					
36		483					-driller began adding water below 36.3m -LIGNITE, very weak, dark brown to black
37		482					SILT AND CLAY, some fine grained black sand, firm to stiff, medium plasticity, grey, moist
38		481					
39		480					-Piezometer B set between 38.7m and 40.2m, static water level measured at 3.79m below grade (October 8, 2003) -LIGNITE layer (0.3m thick) between 39.0m and 39.3m
40		479					
41		478					
42		477					
43		476					
44		475					
45		474					
46		473					
47		472					-LIGNITE layer (0.3m thick) between 47.5m and 47.8m
48		471					CLAY TUFF, trace to some sand, dense, high plasticity, light green, moist to wet -trace gravel from 48.8m
49		470					
50		469					-Piezometer A set between 48.8m and 49.4m, static water level measured at 32.38m below grade (October 8, 2003)
51		468					End of Borehole at 49.7m below grade due to broken drive shoe
52		467					
53		466					
54		465					
55		464					
56		463					
57		462					
58		461					
59		460					
STRATIGRAPHIC KX04397-2003.PUMPING.WELLS.REV3.G91 AMEC PG MULTIWELL.DATATEMPLATE.GDT 5/2/07		Earth & Environmental 3455 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243			LOGGED BY: SJ ENTERED BY: CD	COMPLETION DEPTH: 49.7 m COMPLETION DATE: 7/17/03	

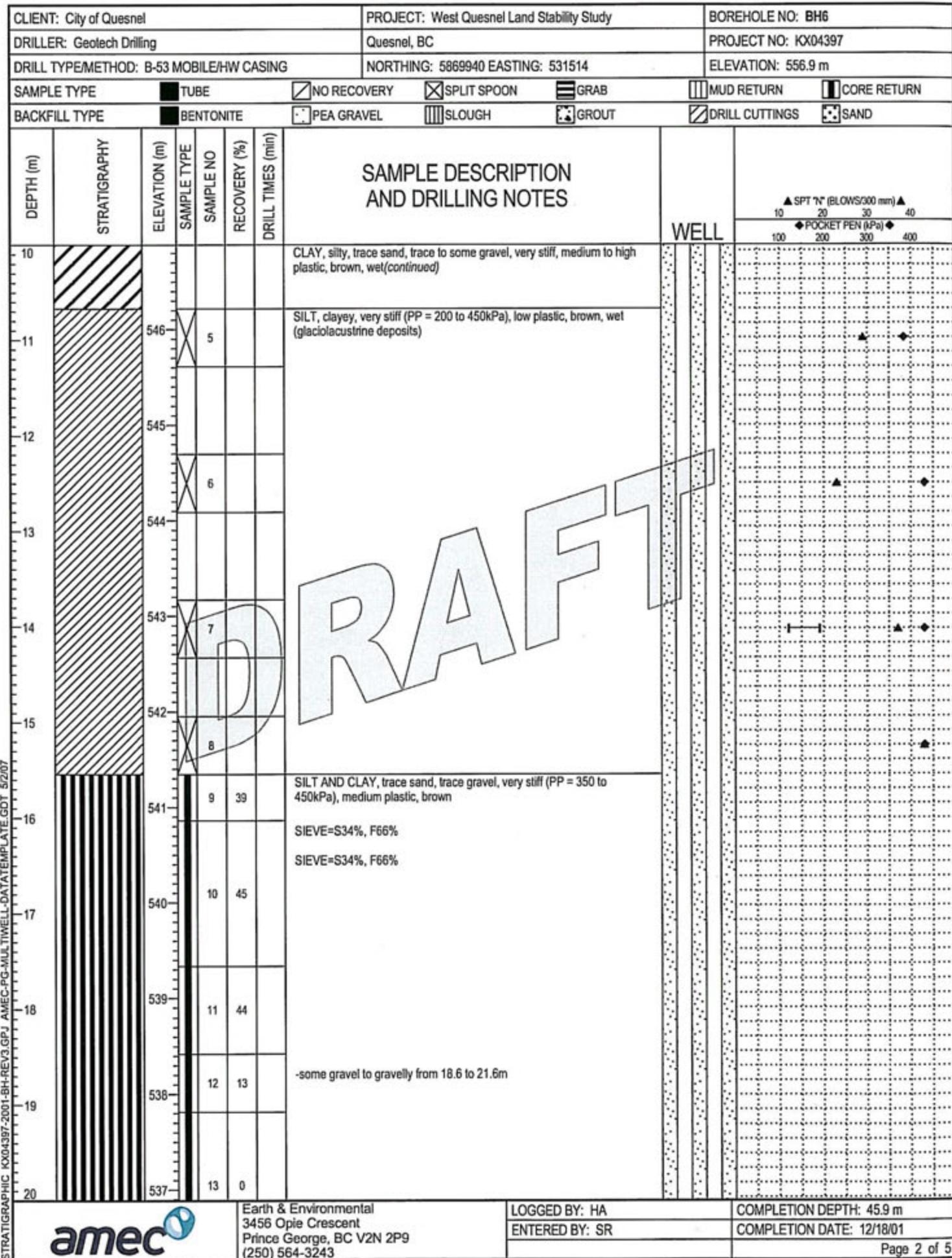


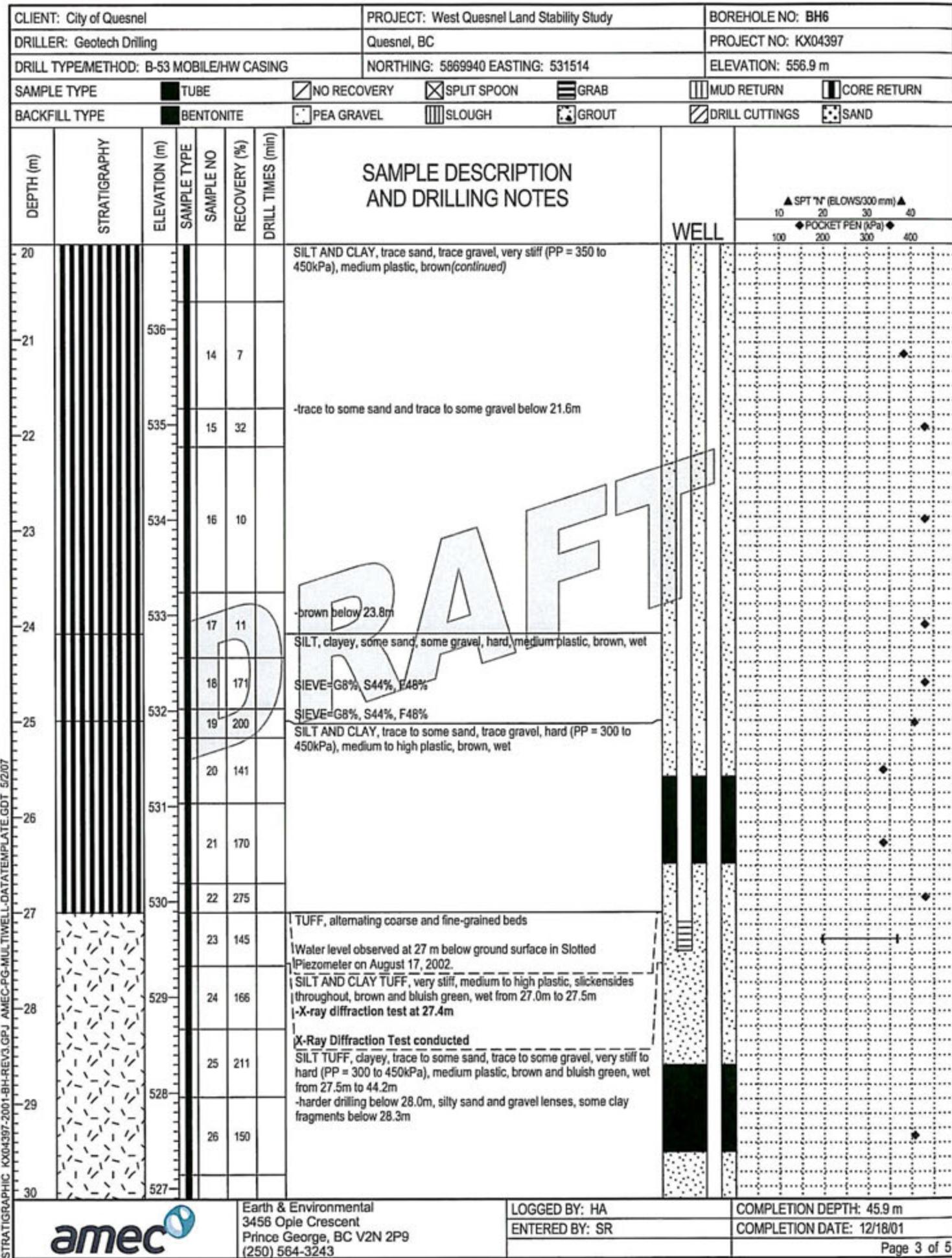
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

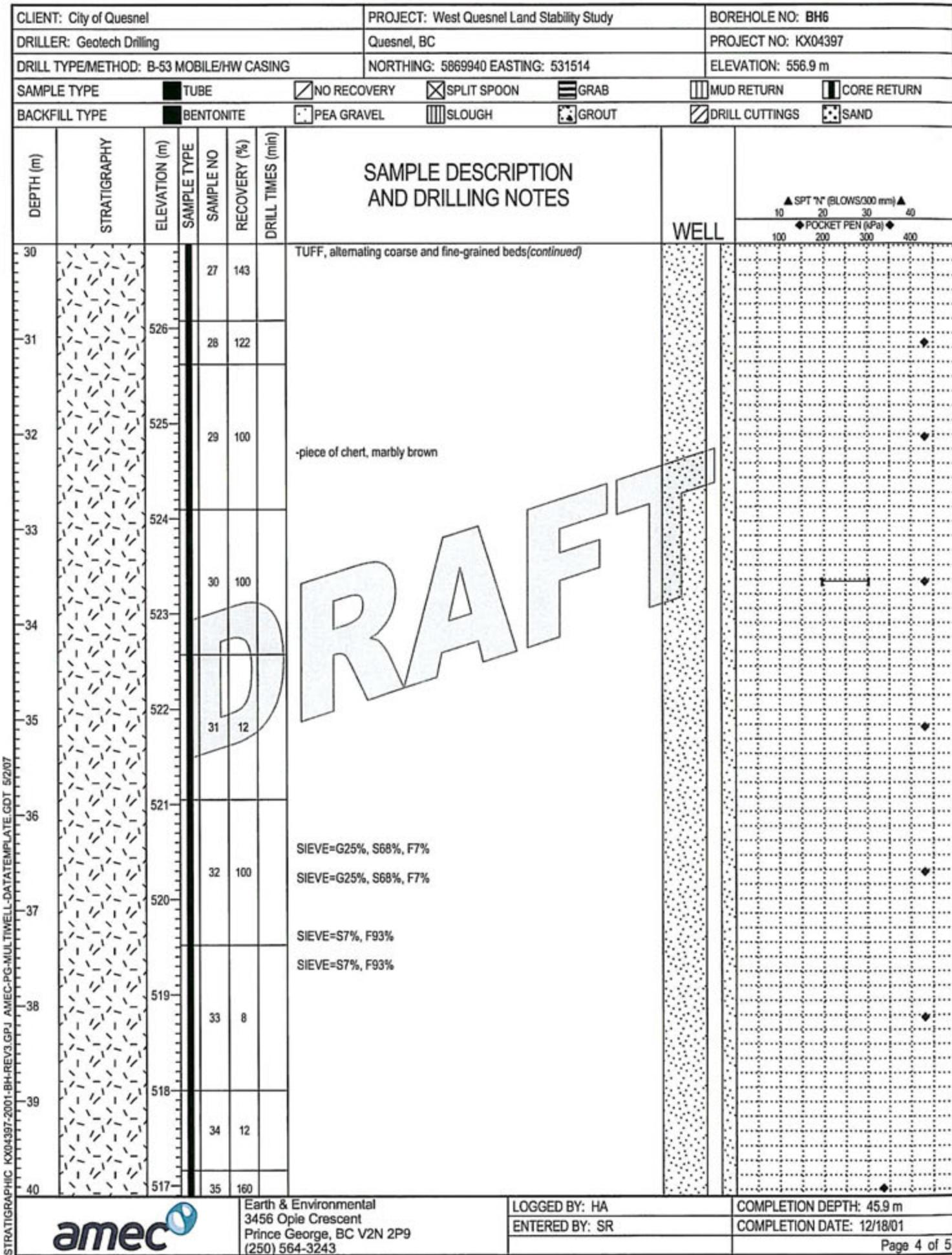
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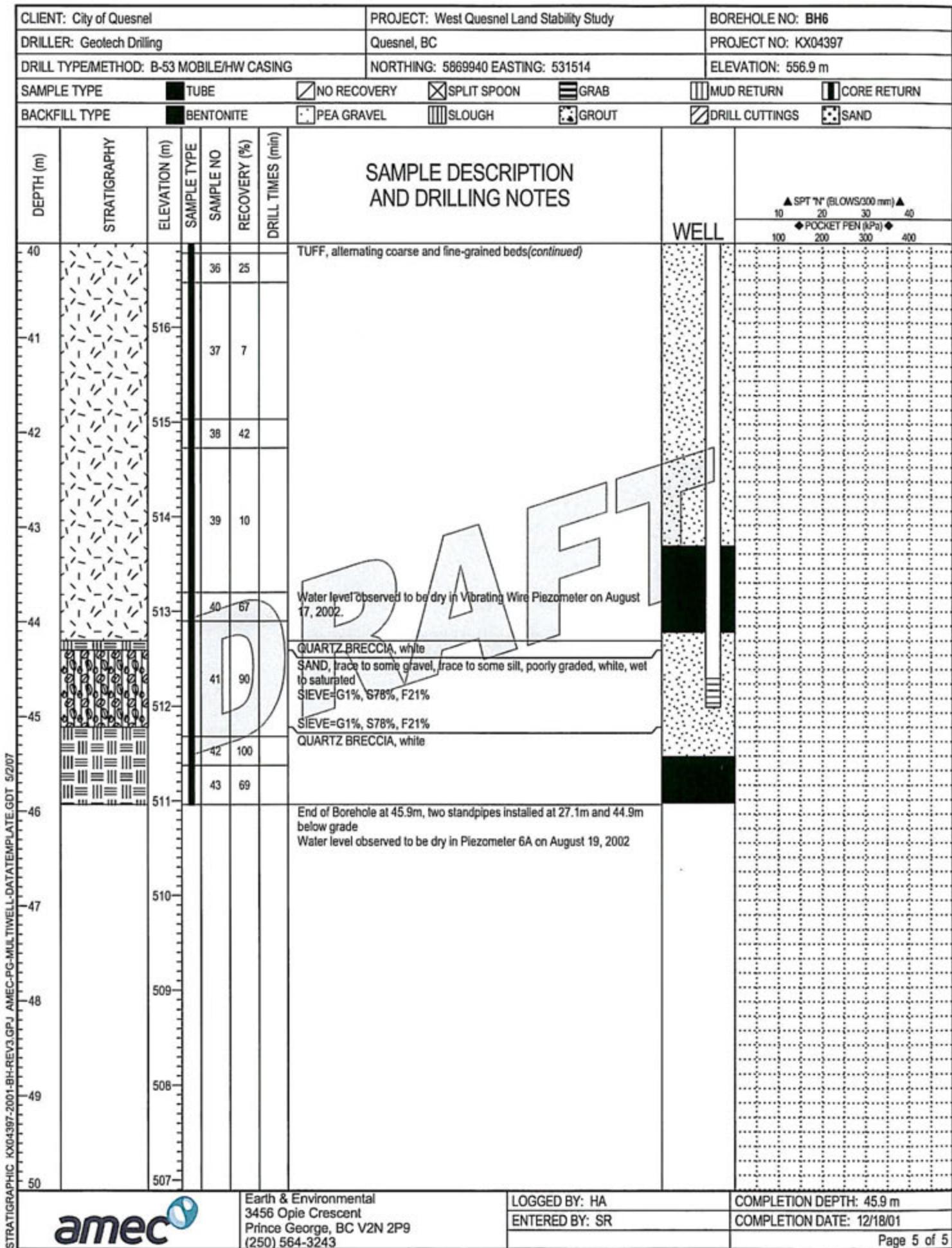
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COMPLETION DATE: 7/18/03

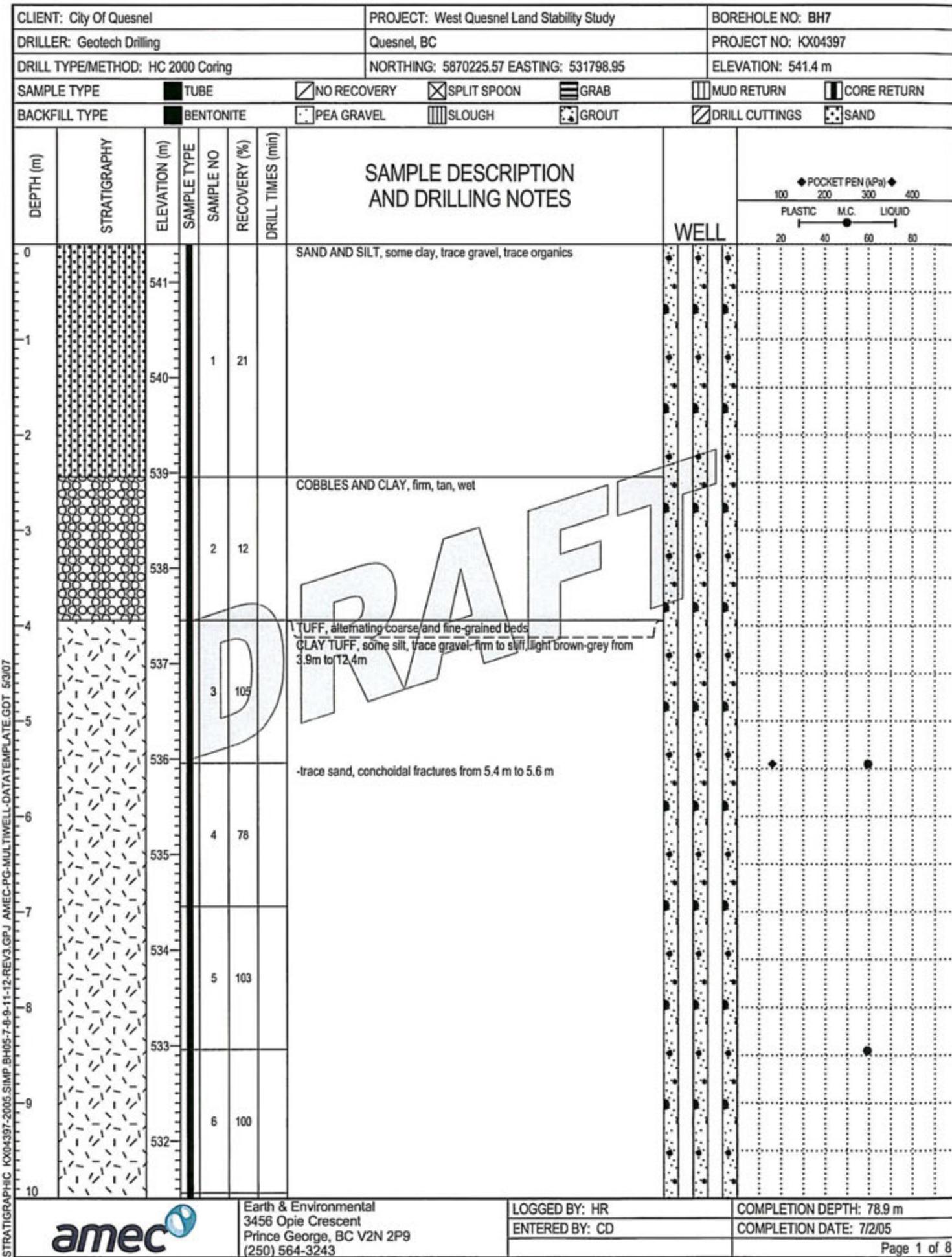


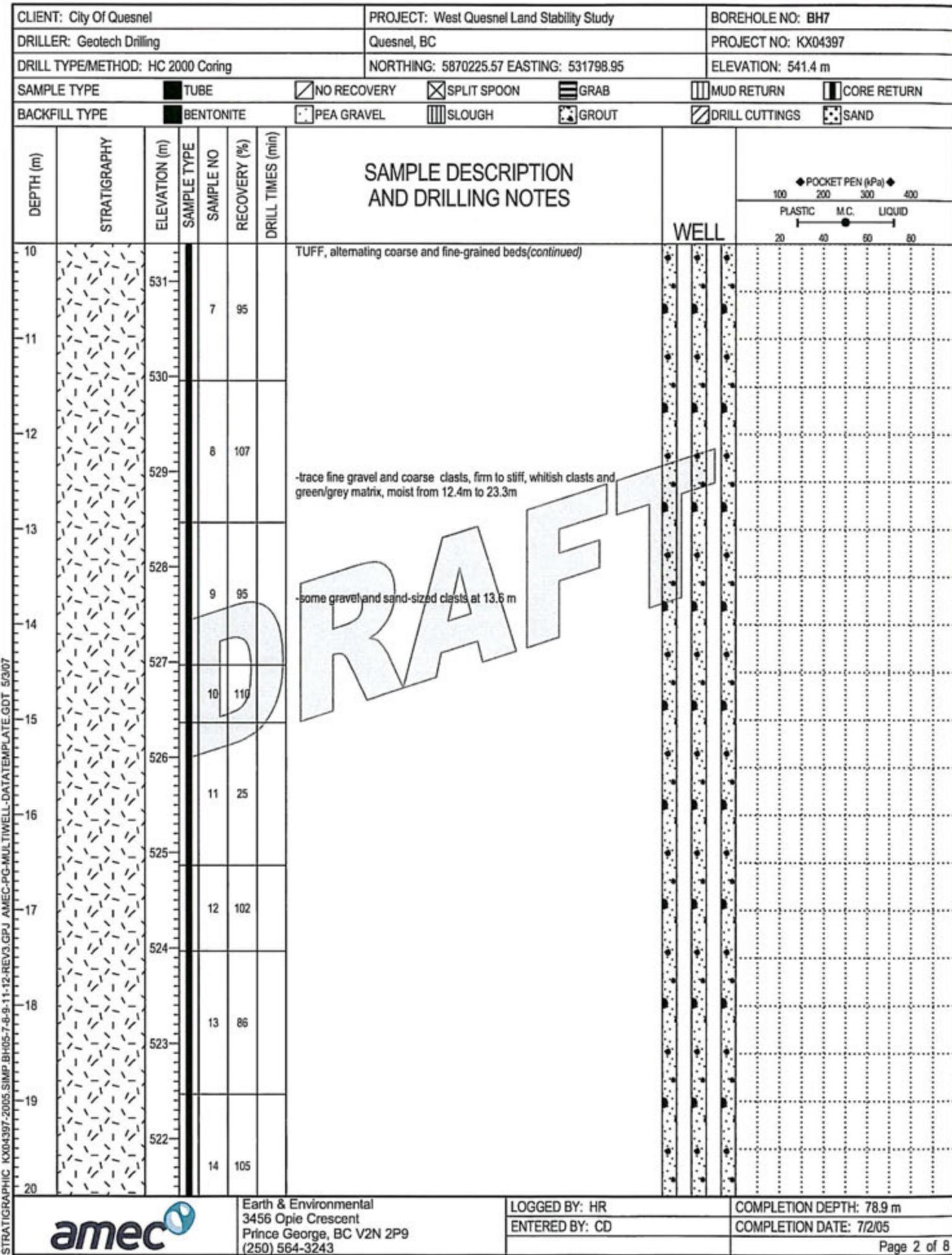




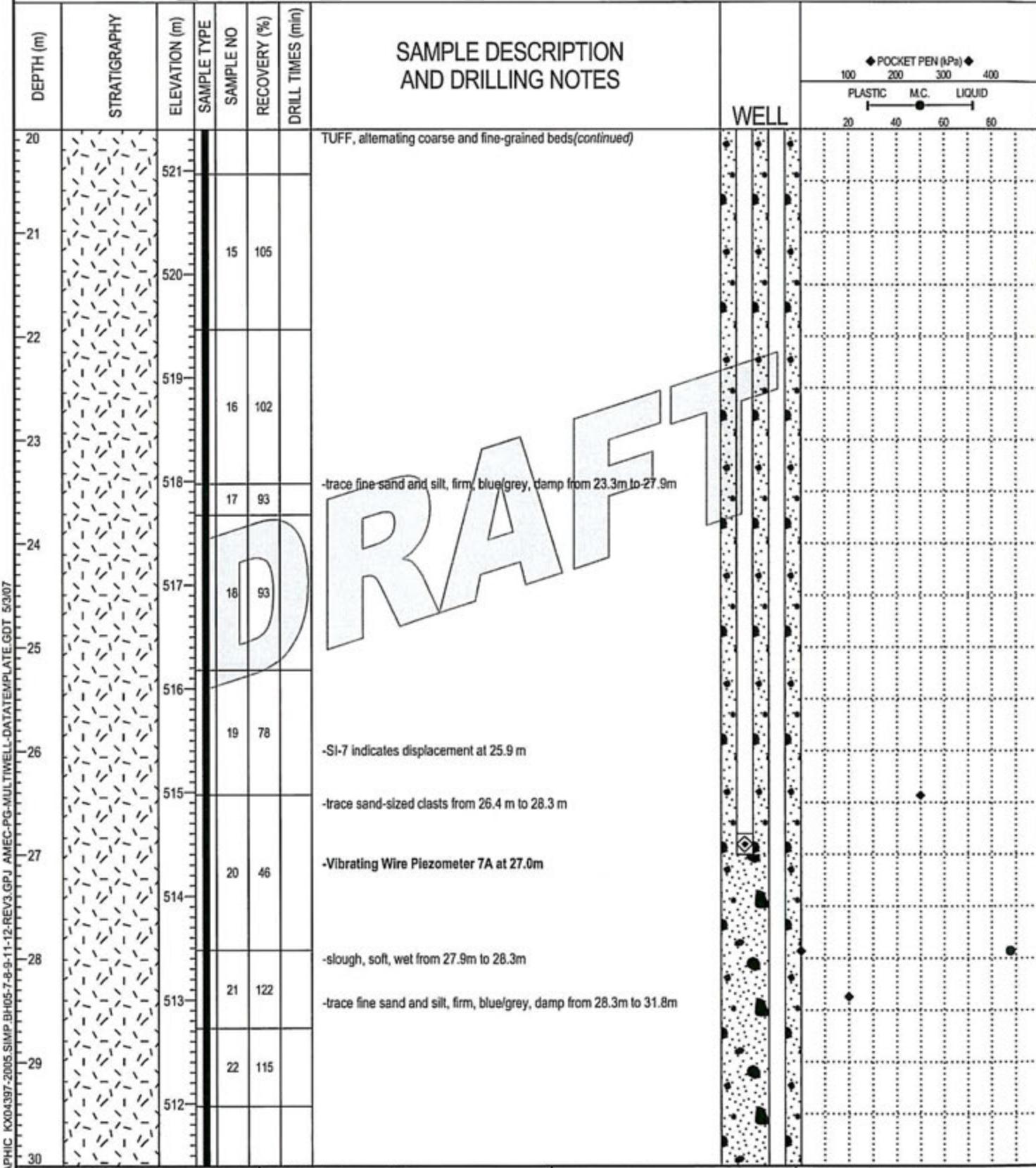




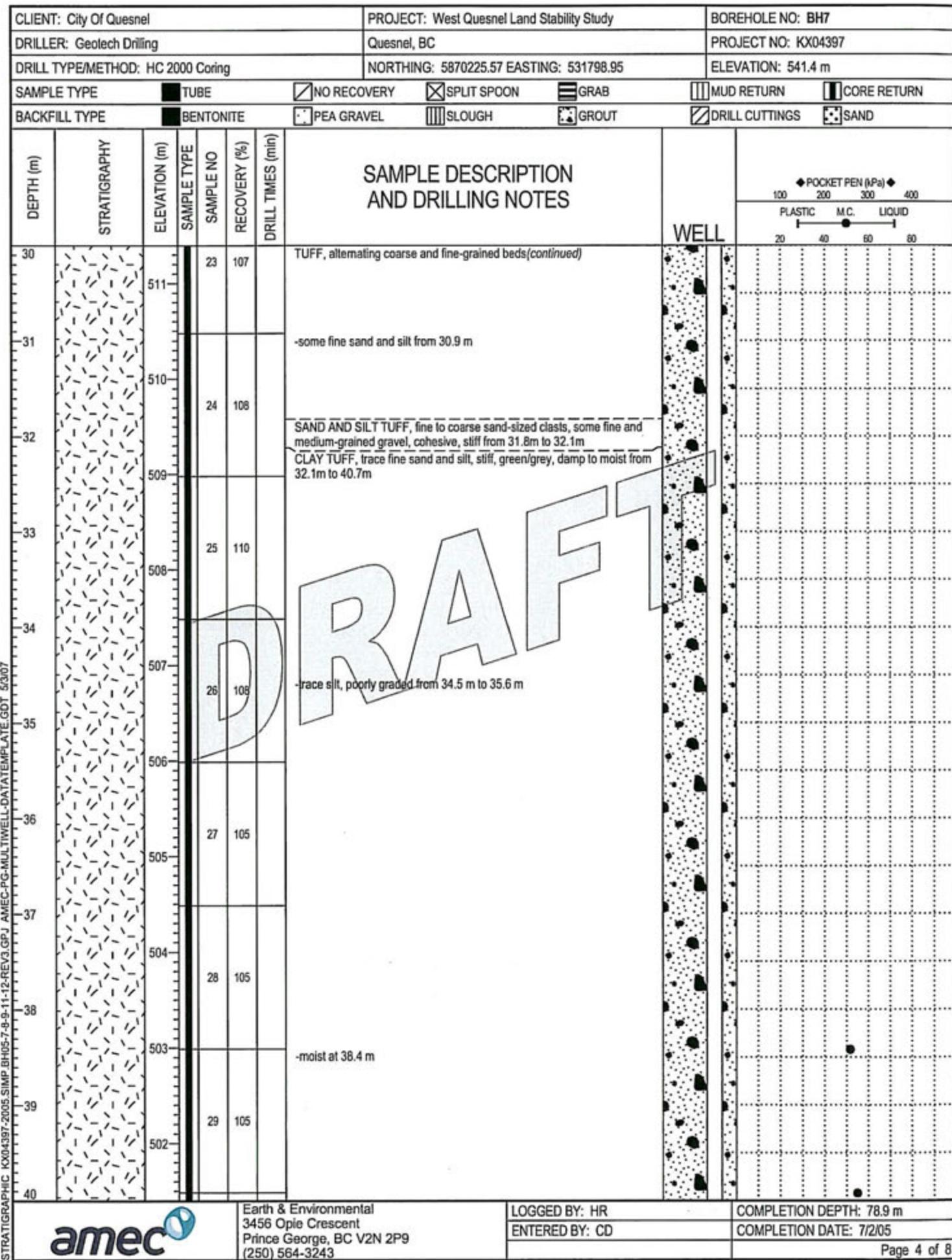




CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH7
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870225.57 EASTING: 531798.95	ELEVATION: 541.4 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



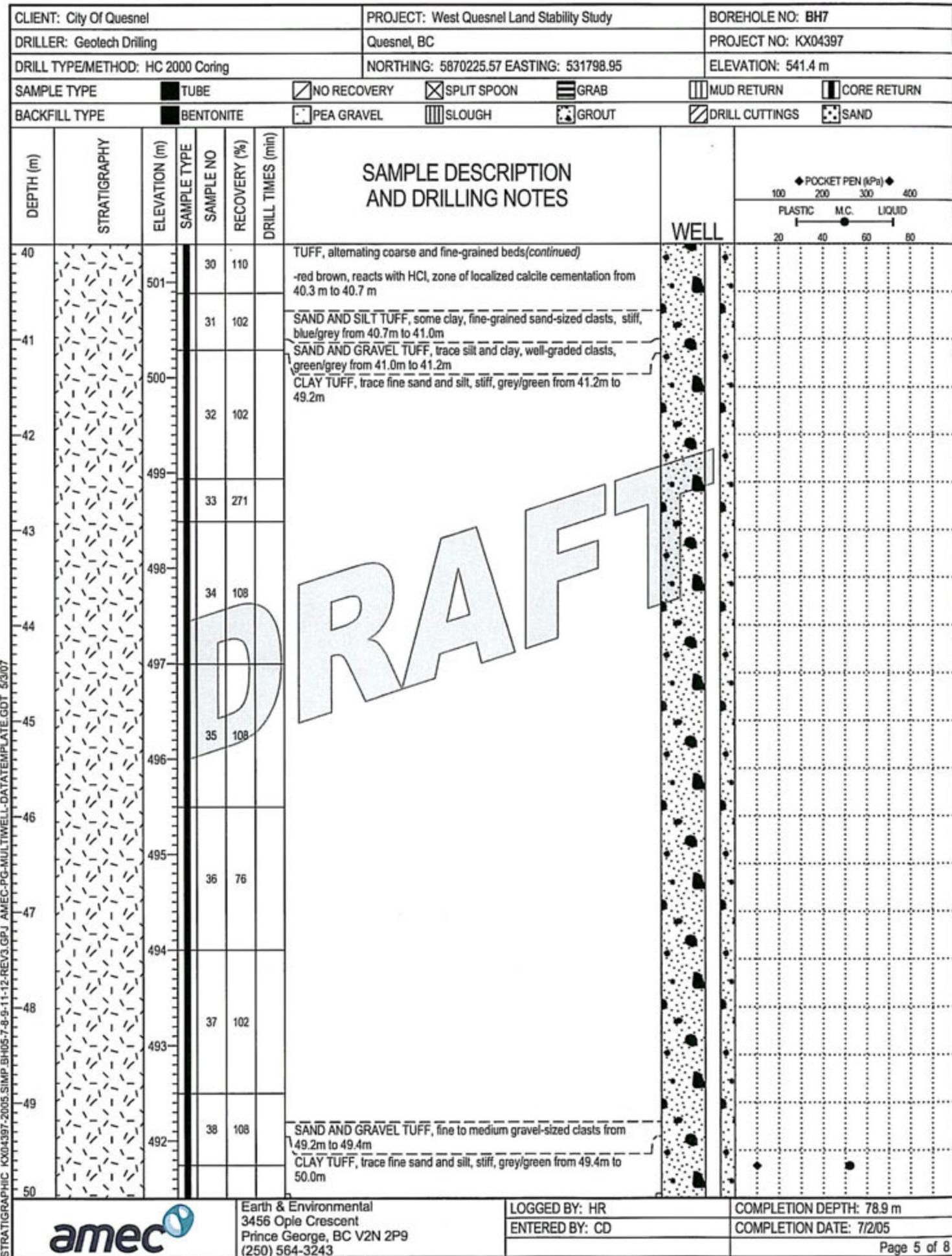
amec	Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243	LOGGED BY: HR ENTERED BY: CD	COMPLETION DEPTH: 78.9 m COMPLETION DATE: 7/2/05
			Page 3 of 8

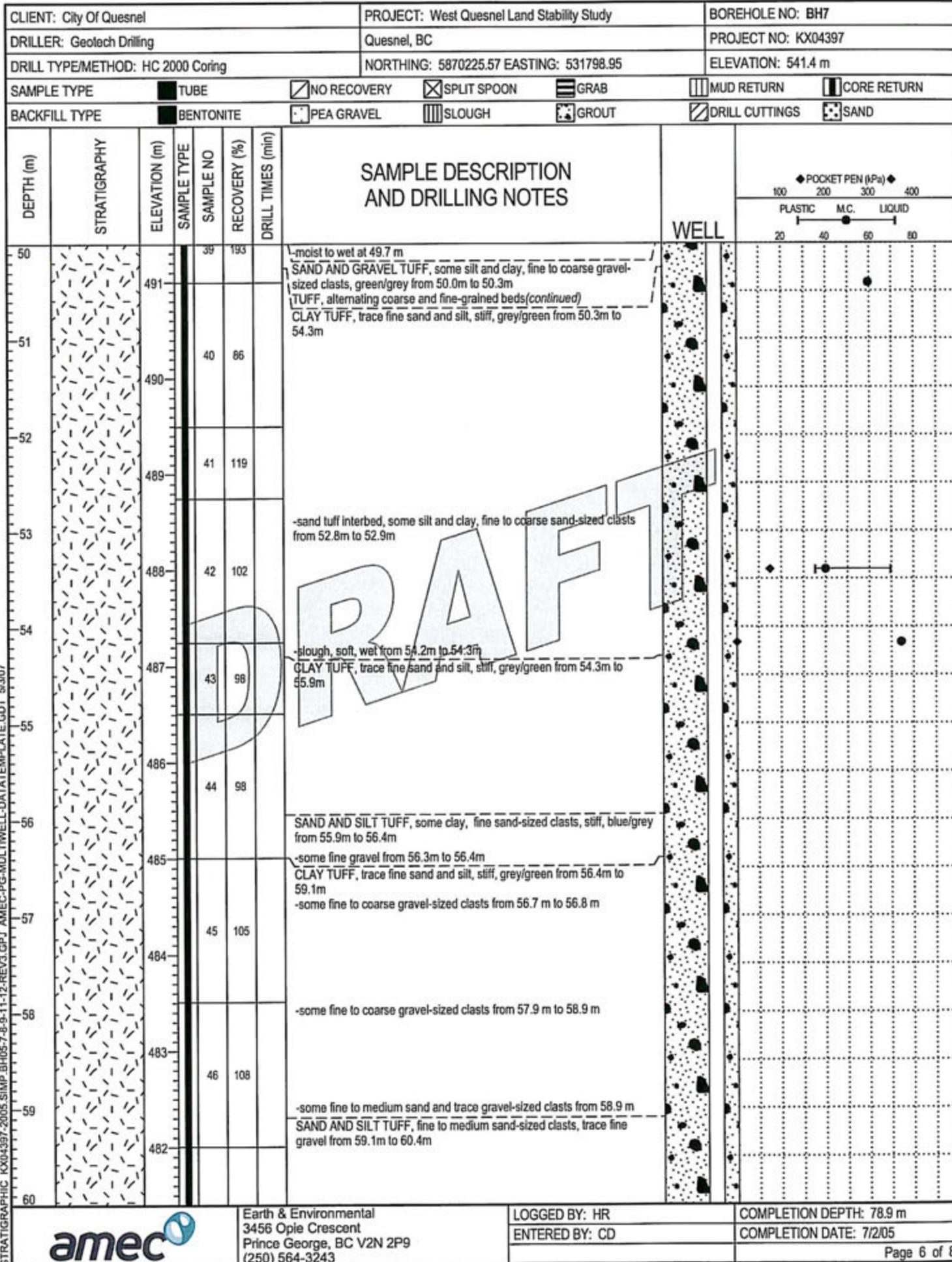


Earth & Environmental
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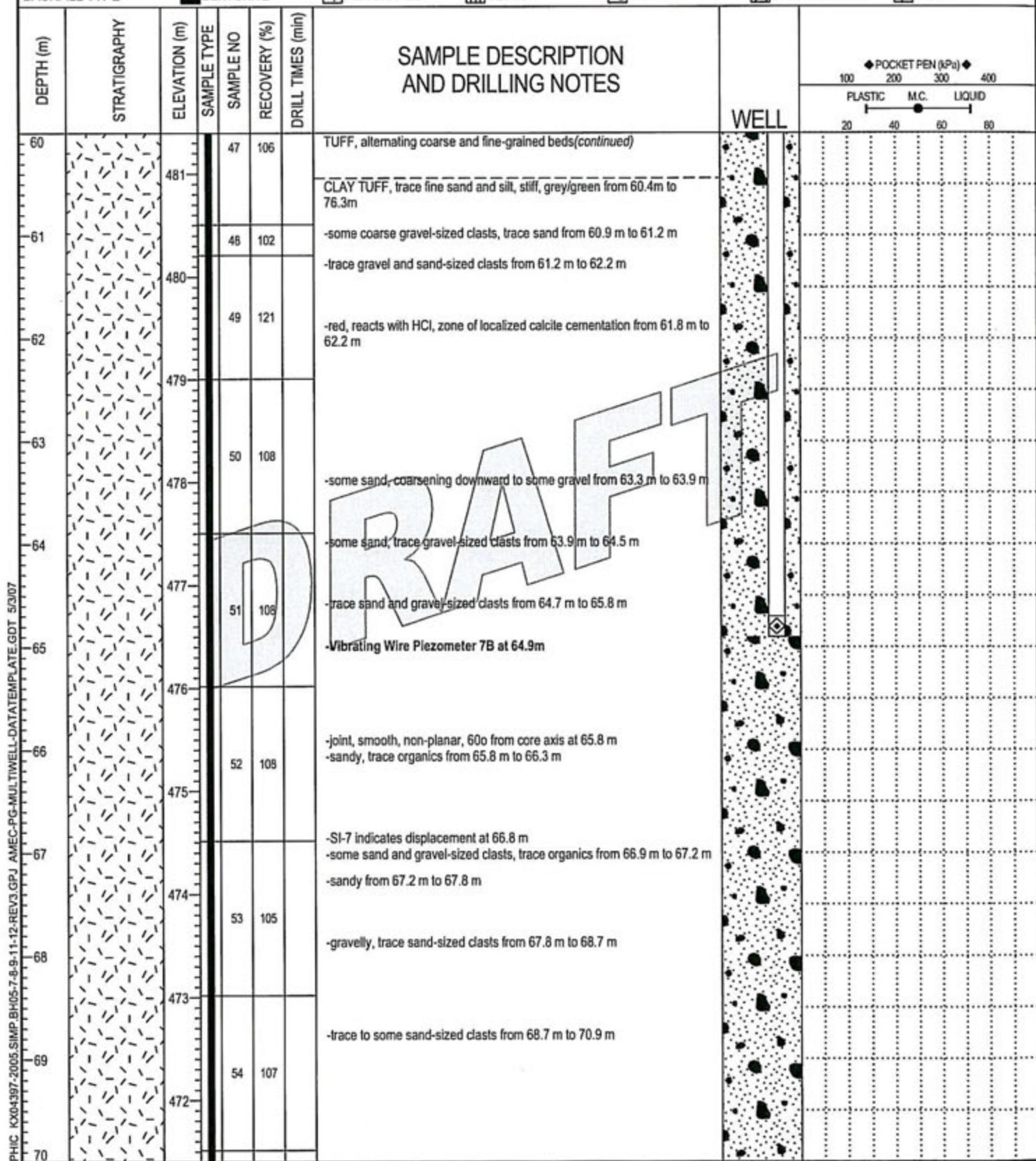
LOGGED BY: HR
 ENTERED BY: CD

COMPLETION DEPTH: 78.9 m
 COMPLETION DATE: 7/2/05





CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH7
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870225.57 EASTING: 531798.95	ELEVATION: 541.4 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



STRATIGRAPHIC KX04397-2005 SIMP BH057-8-9-11-12-REV3 GPJ AMEC-PG-MULTIWELL-DATAWELL.GDT 5/3/07



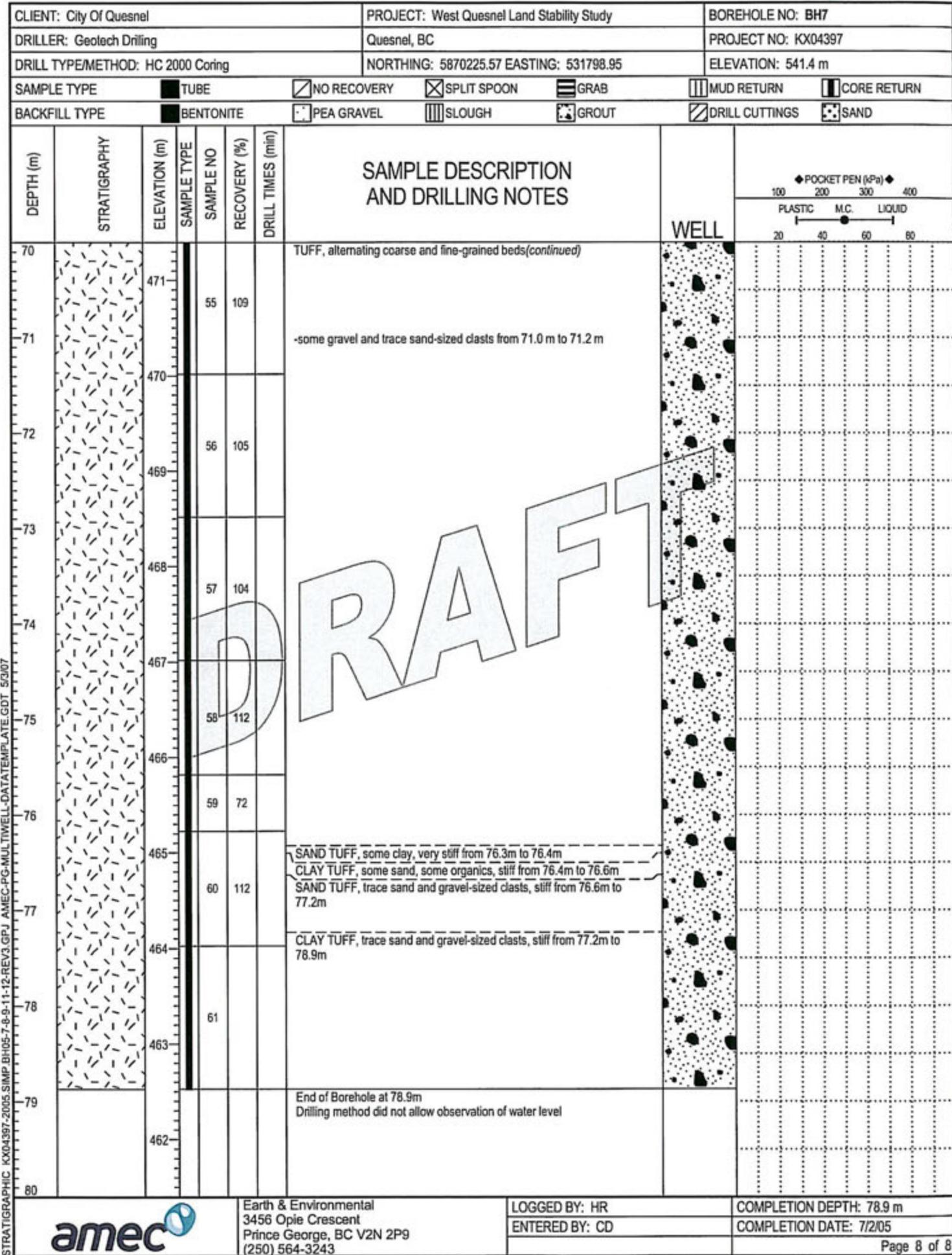
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: HR

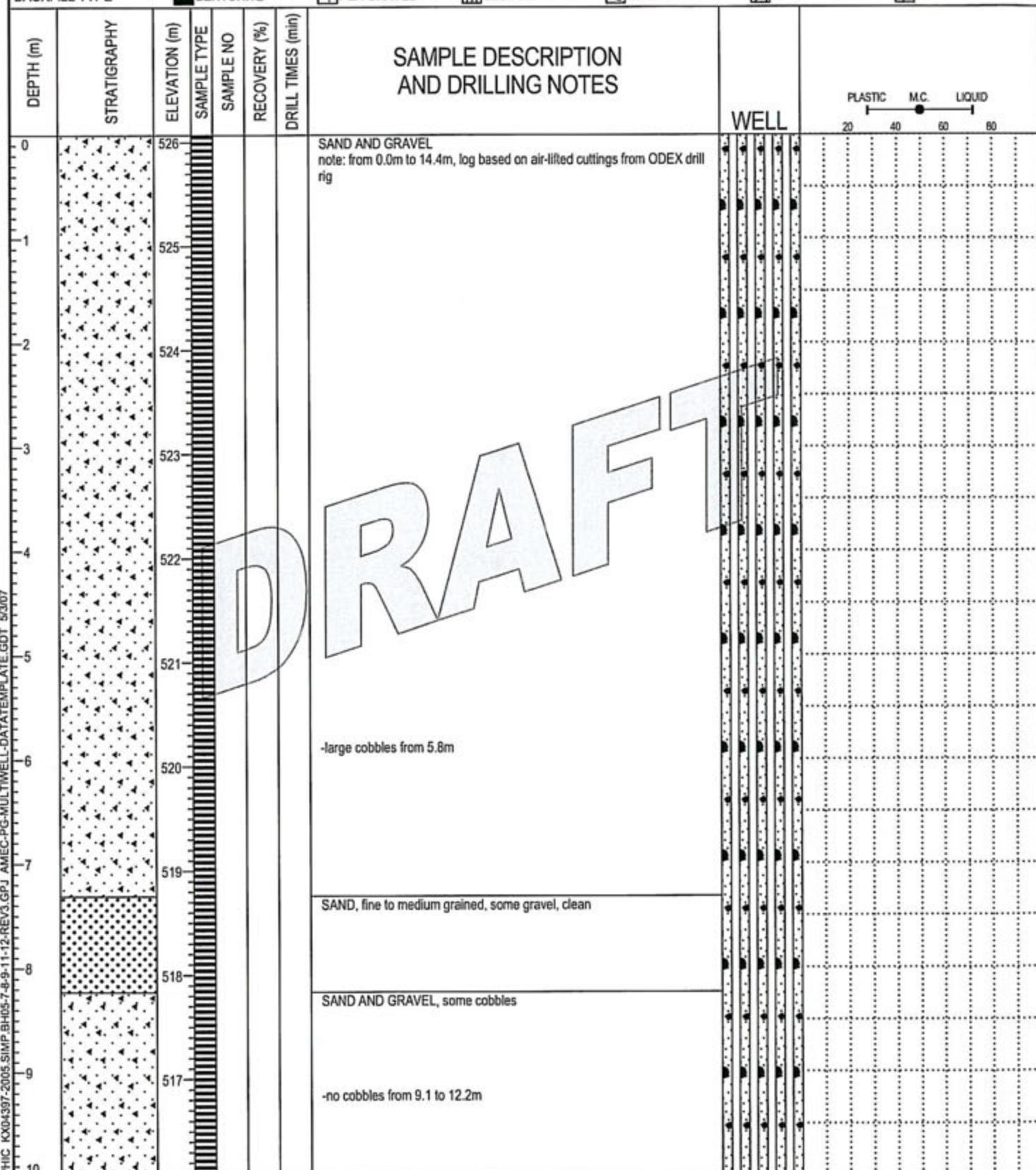
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COMPLETION DEPTH: 78.9 m

COMPLETION DATE: 7/2/05



CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH8
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870042.69 EASTING: 532161.17	ELEVATION: 526.1 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



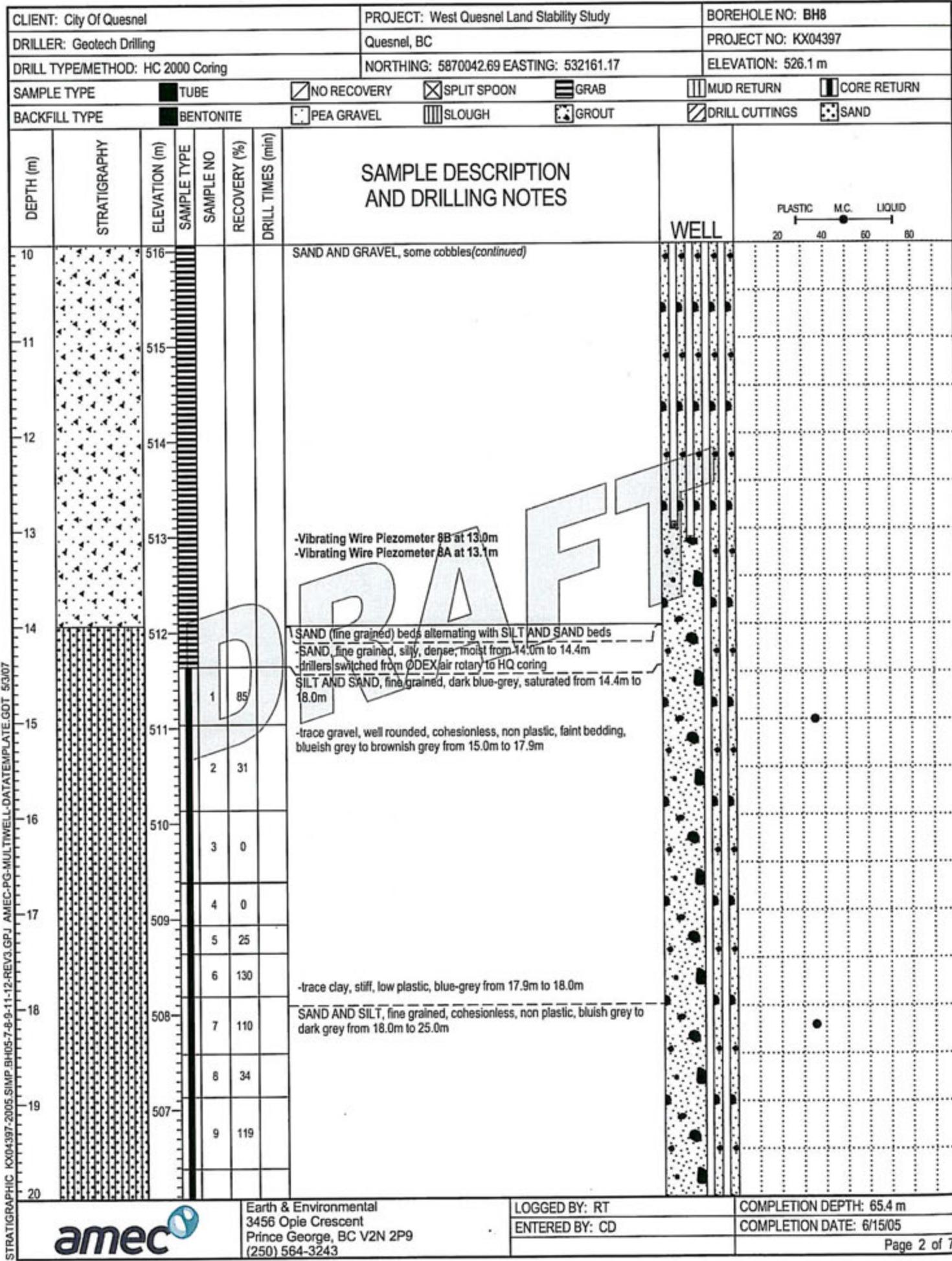
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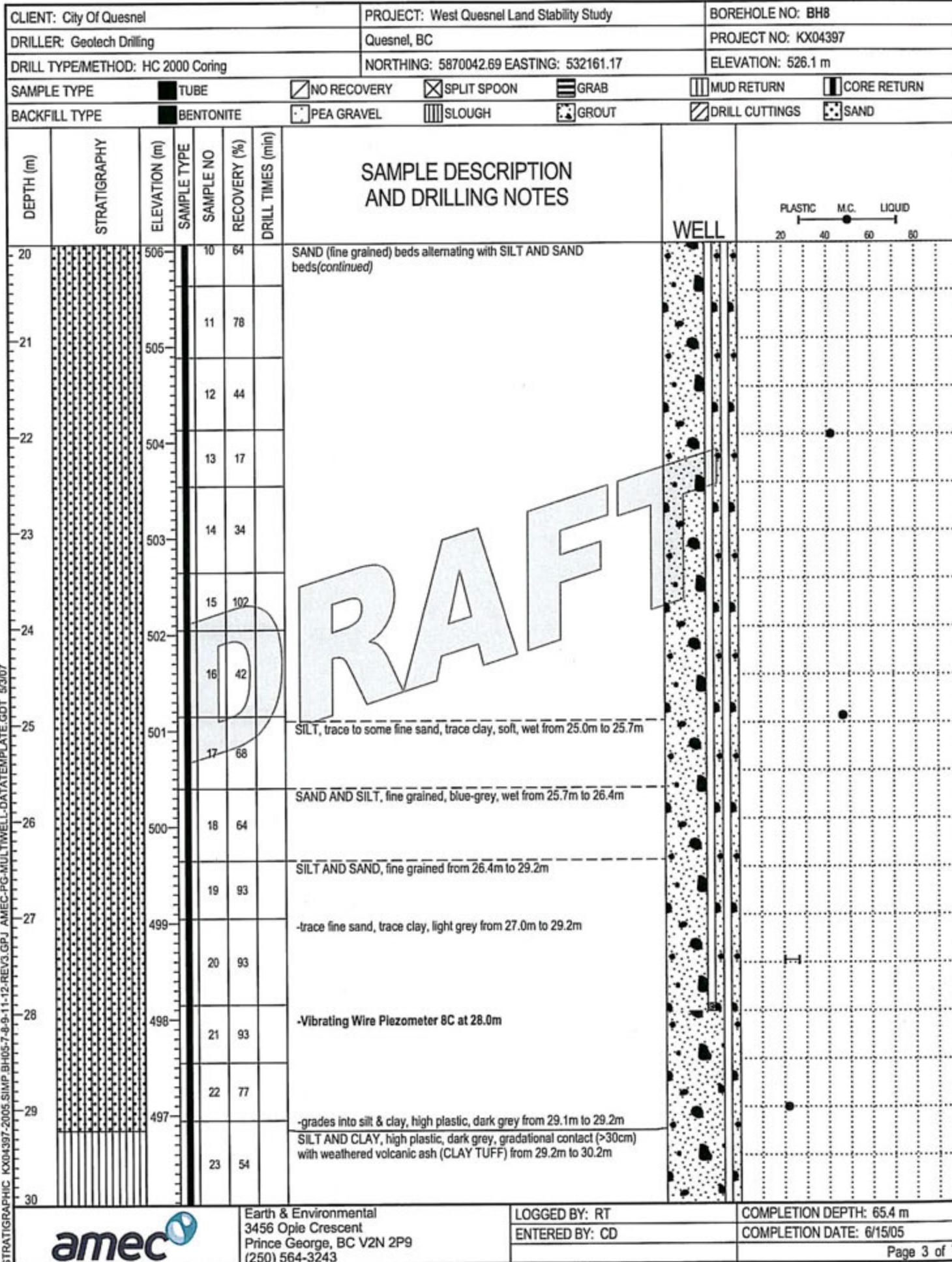


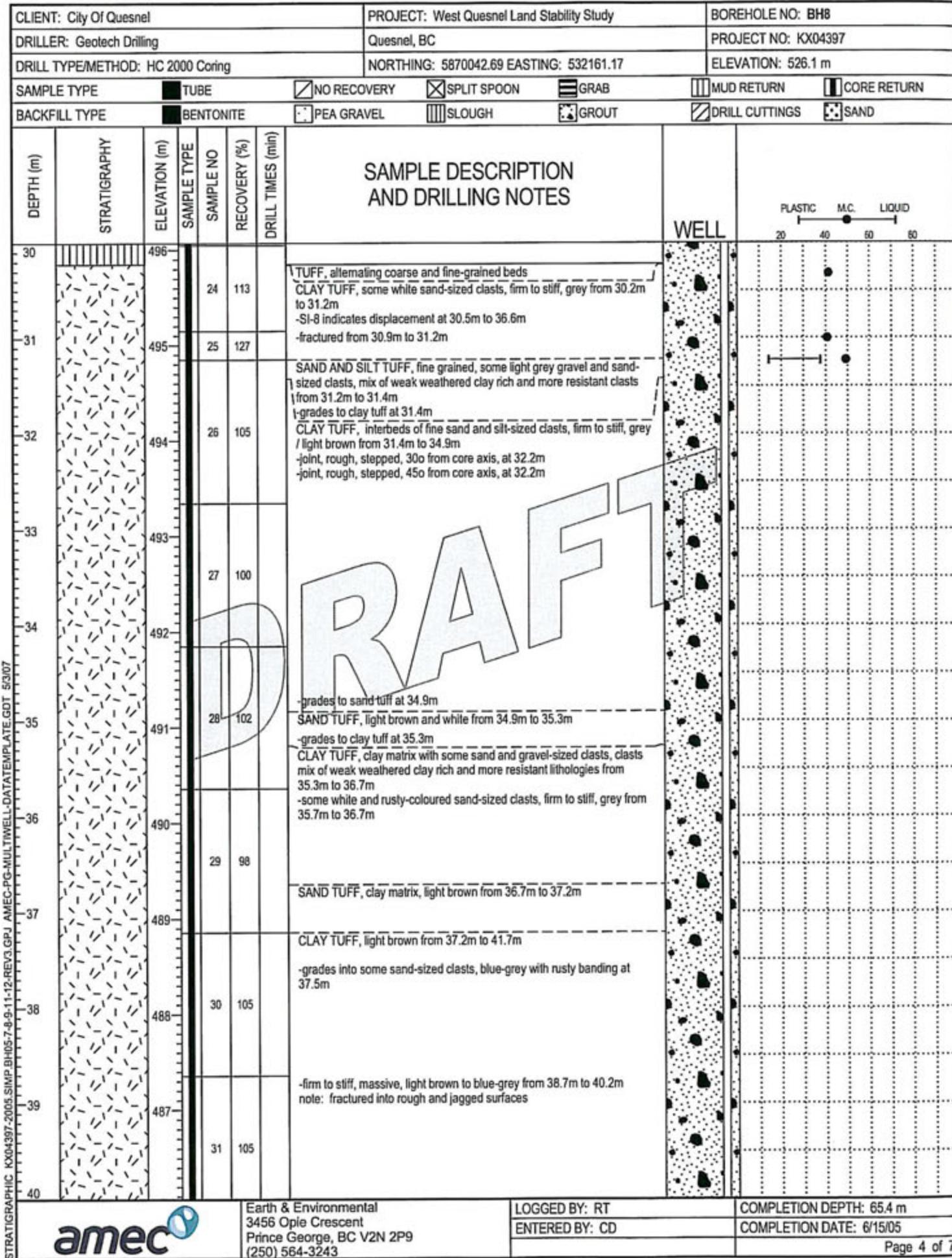
Earth & Environmental
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(250) 564-3243

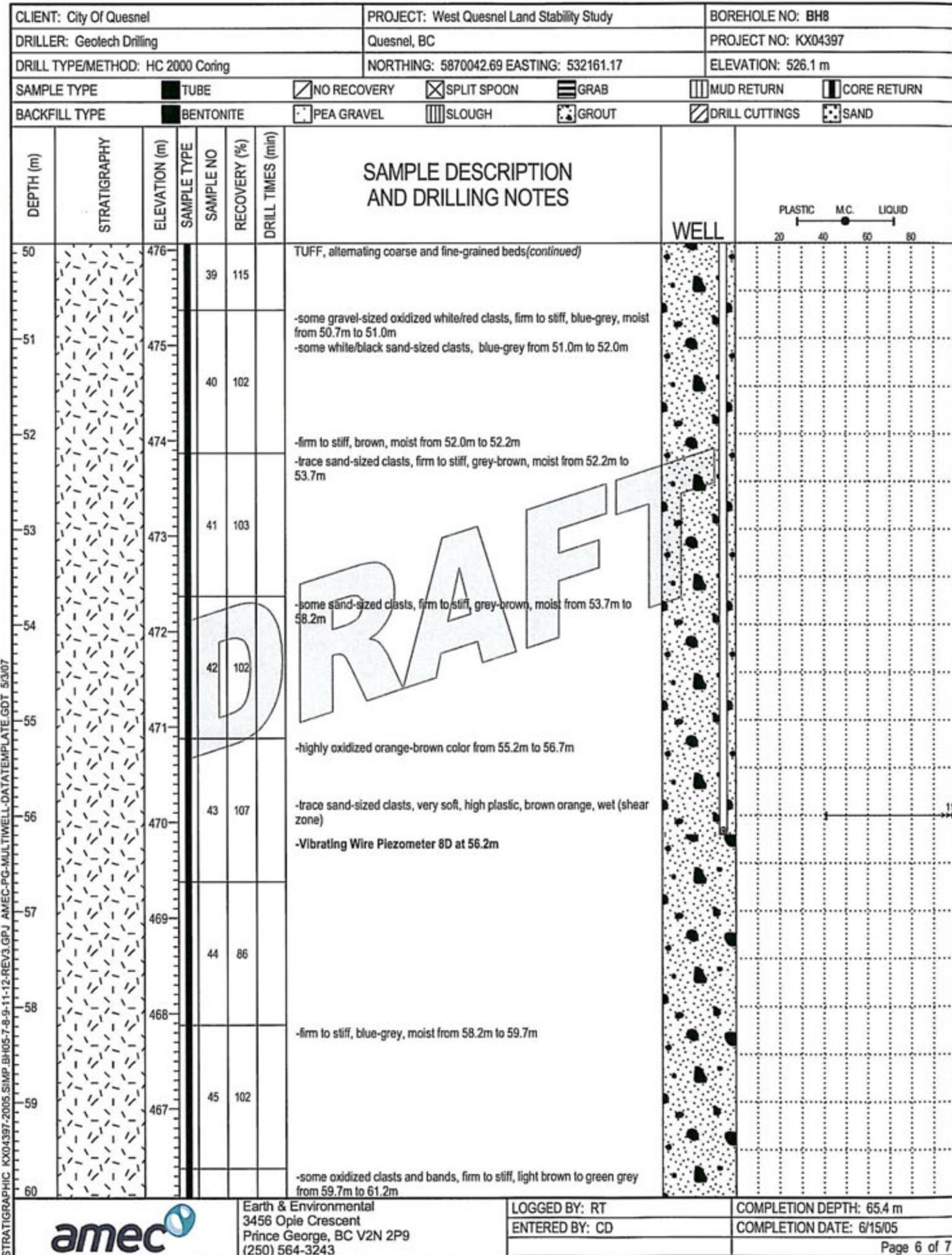
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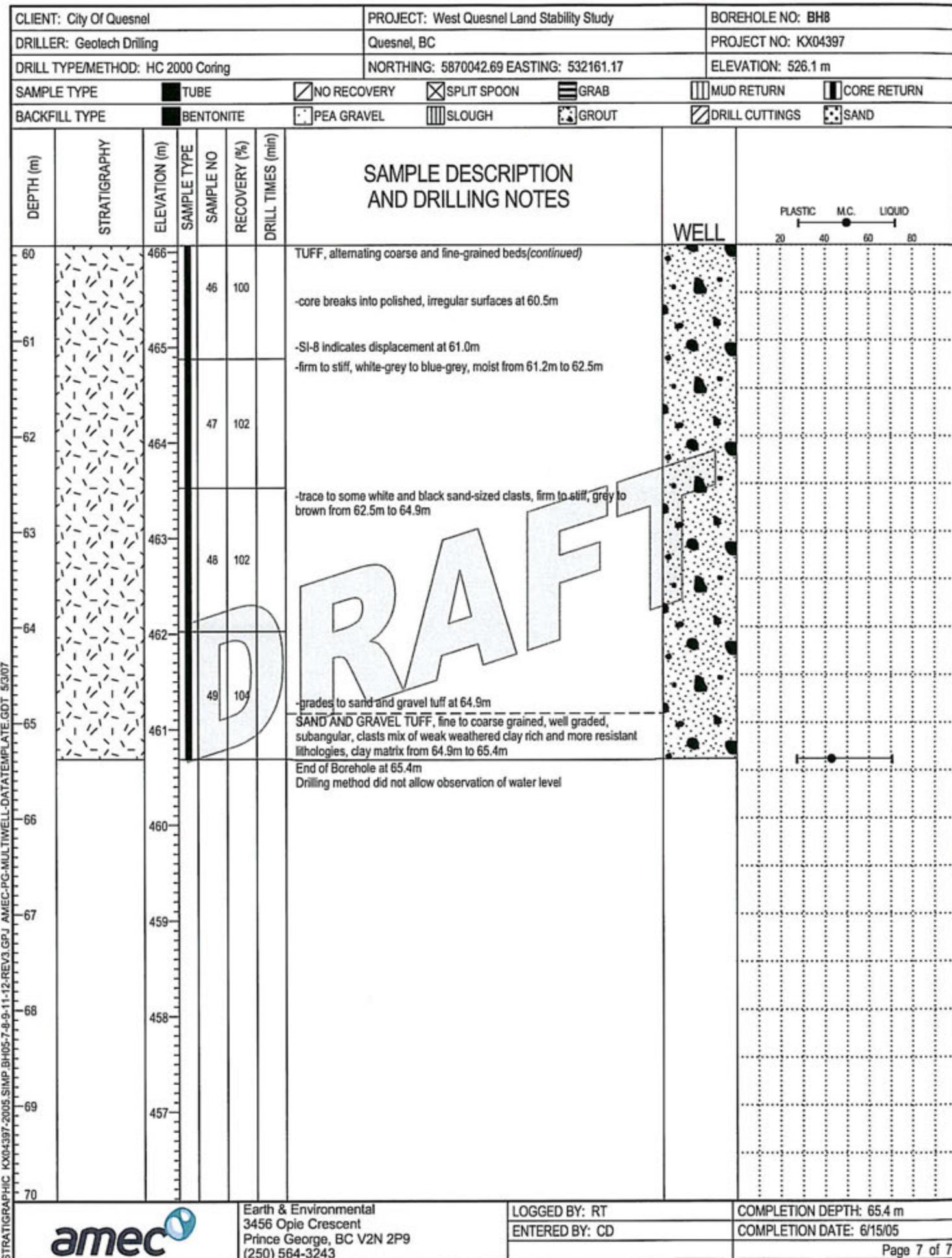
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COMPLETION DATE: 6/15/05

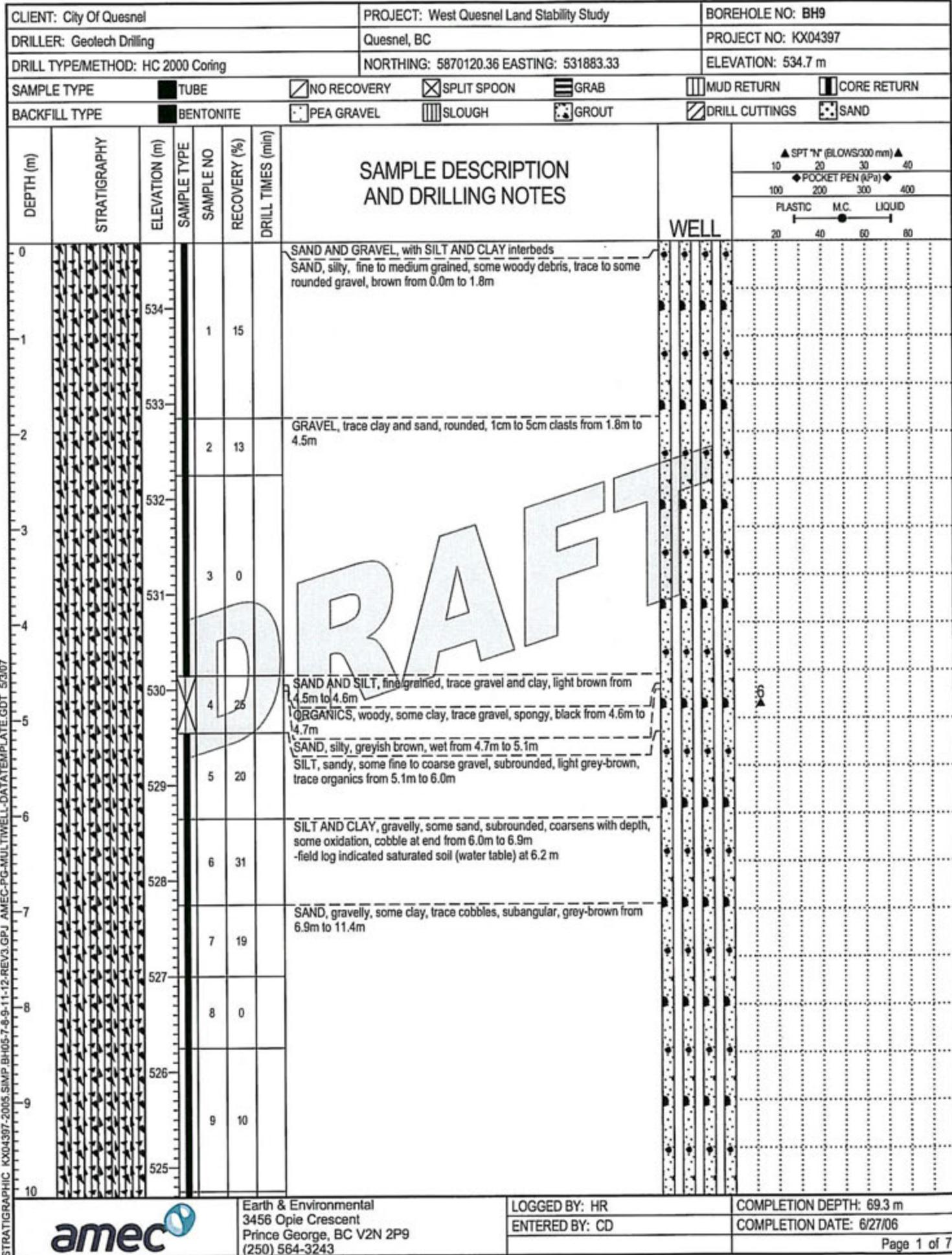


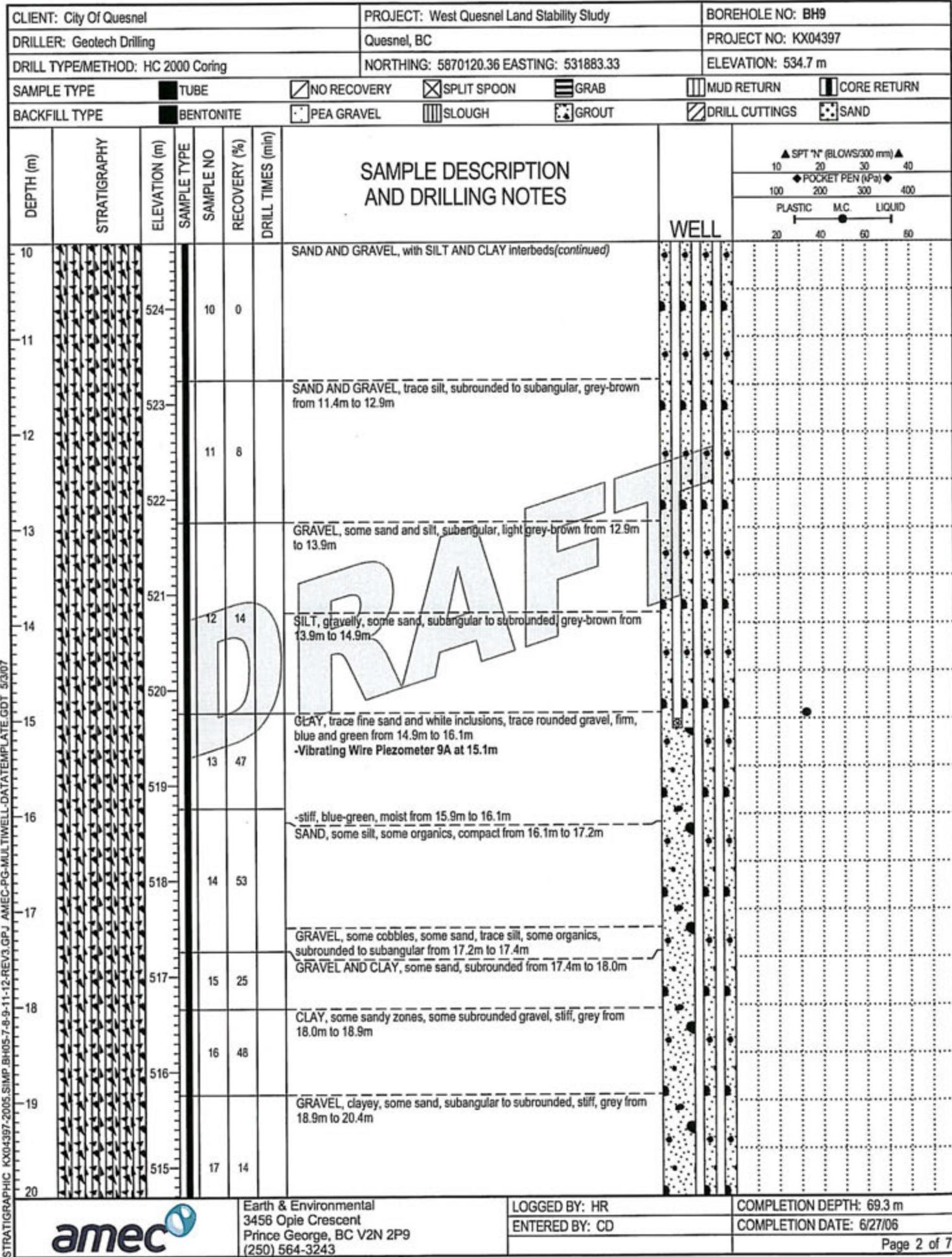


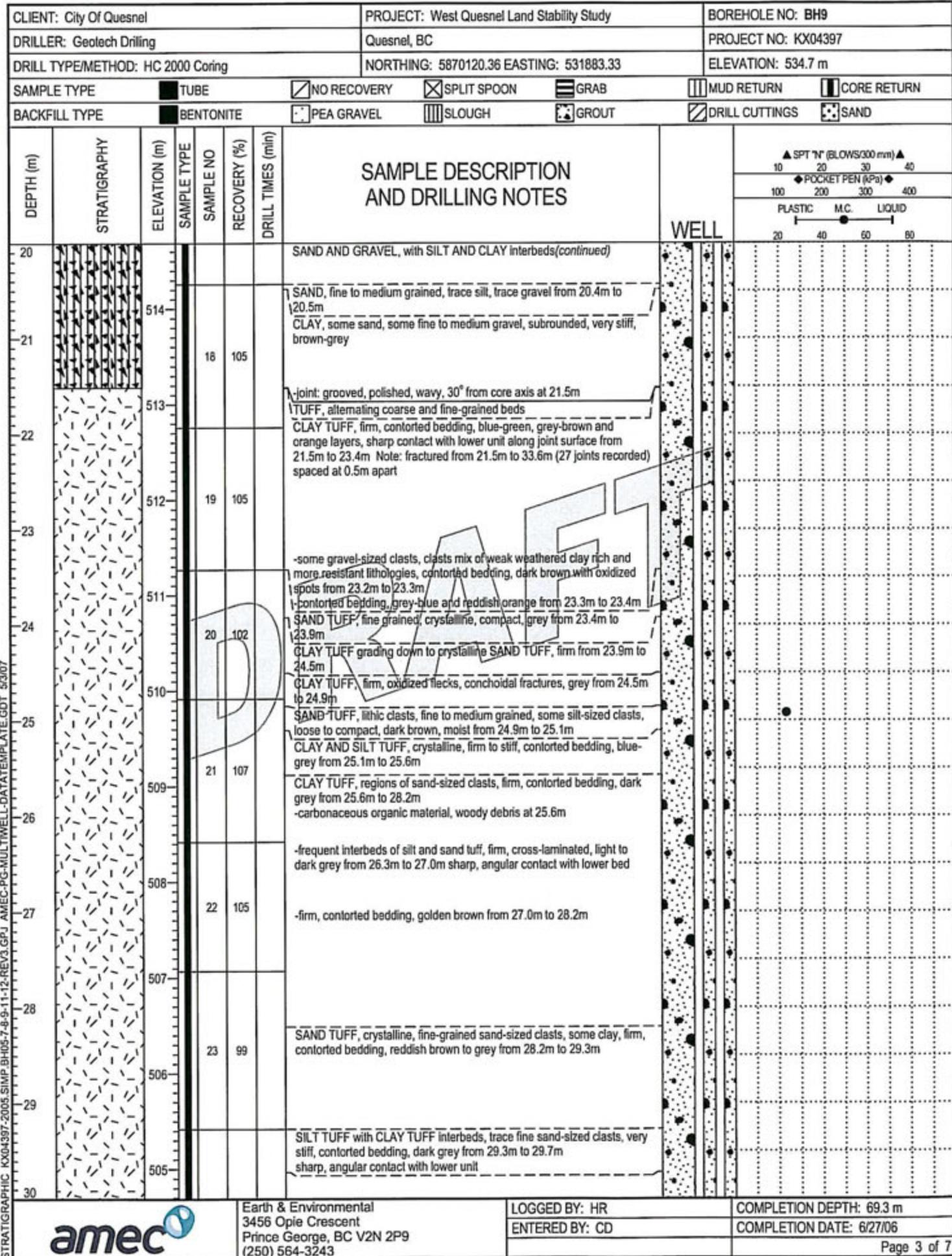


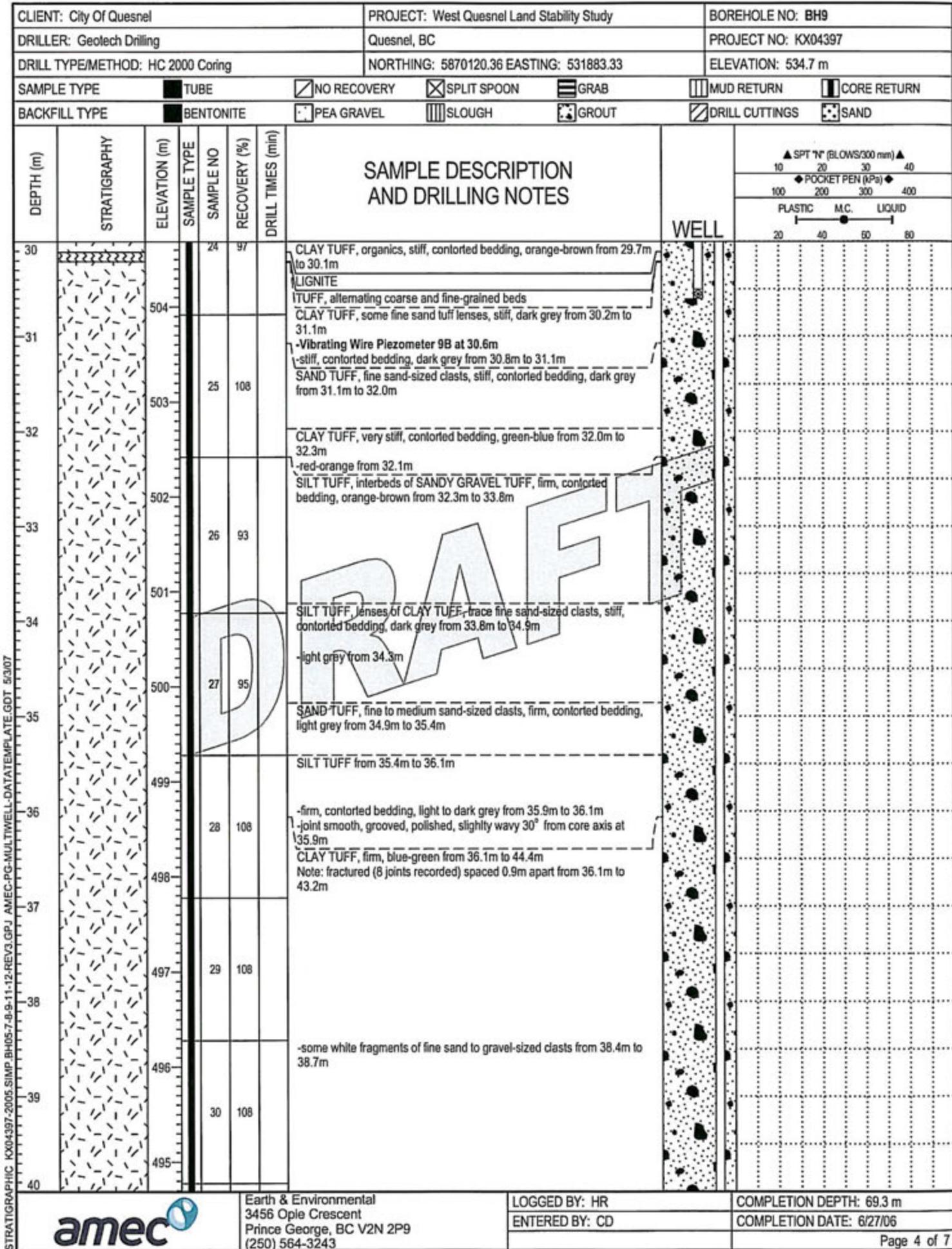


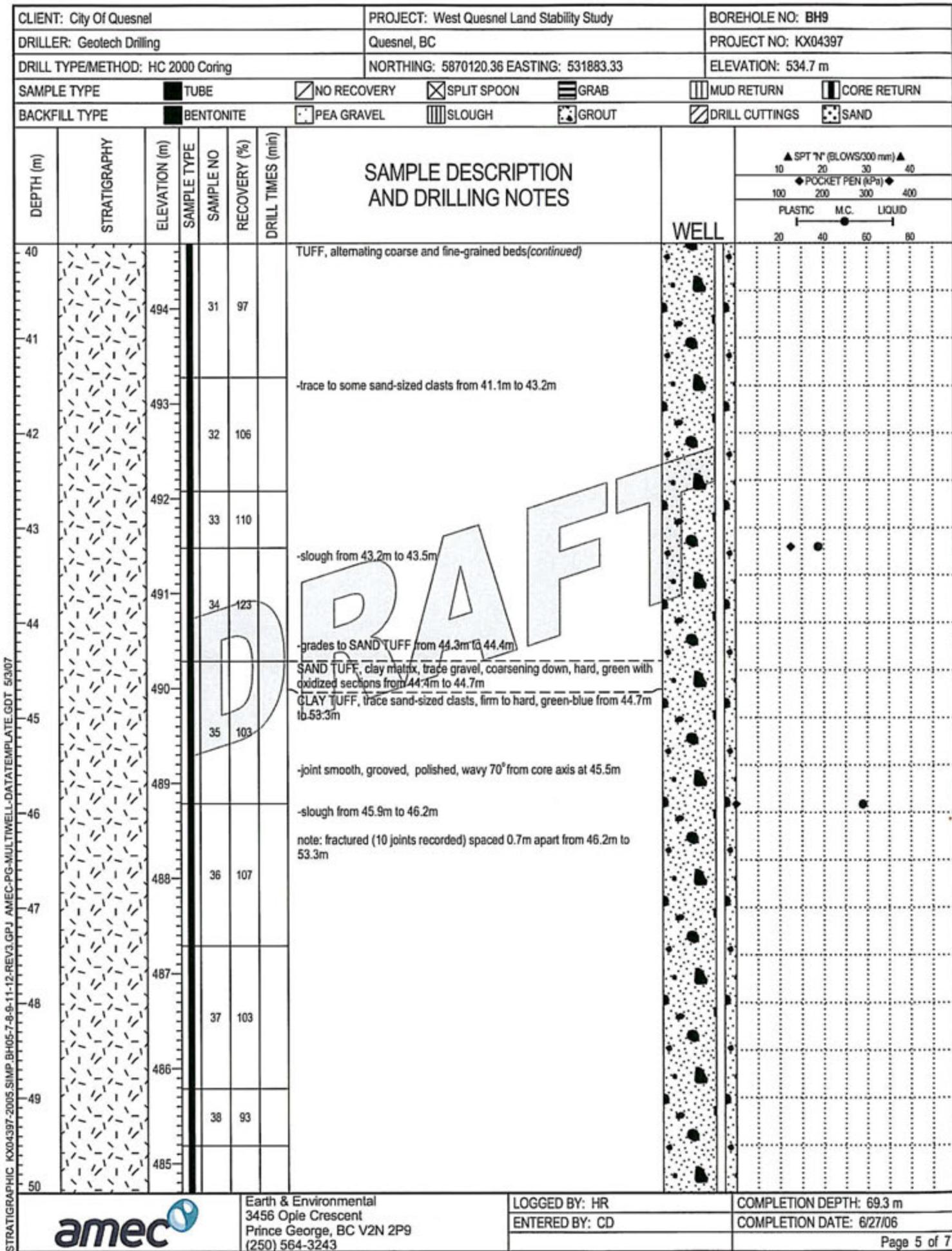


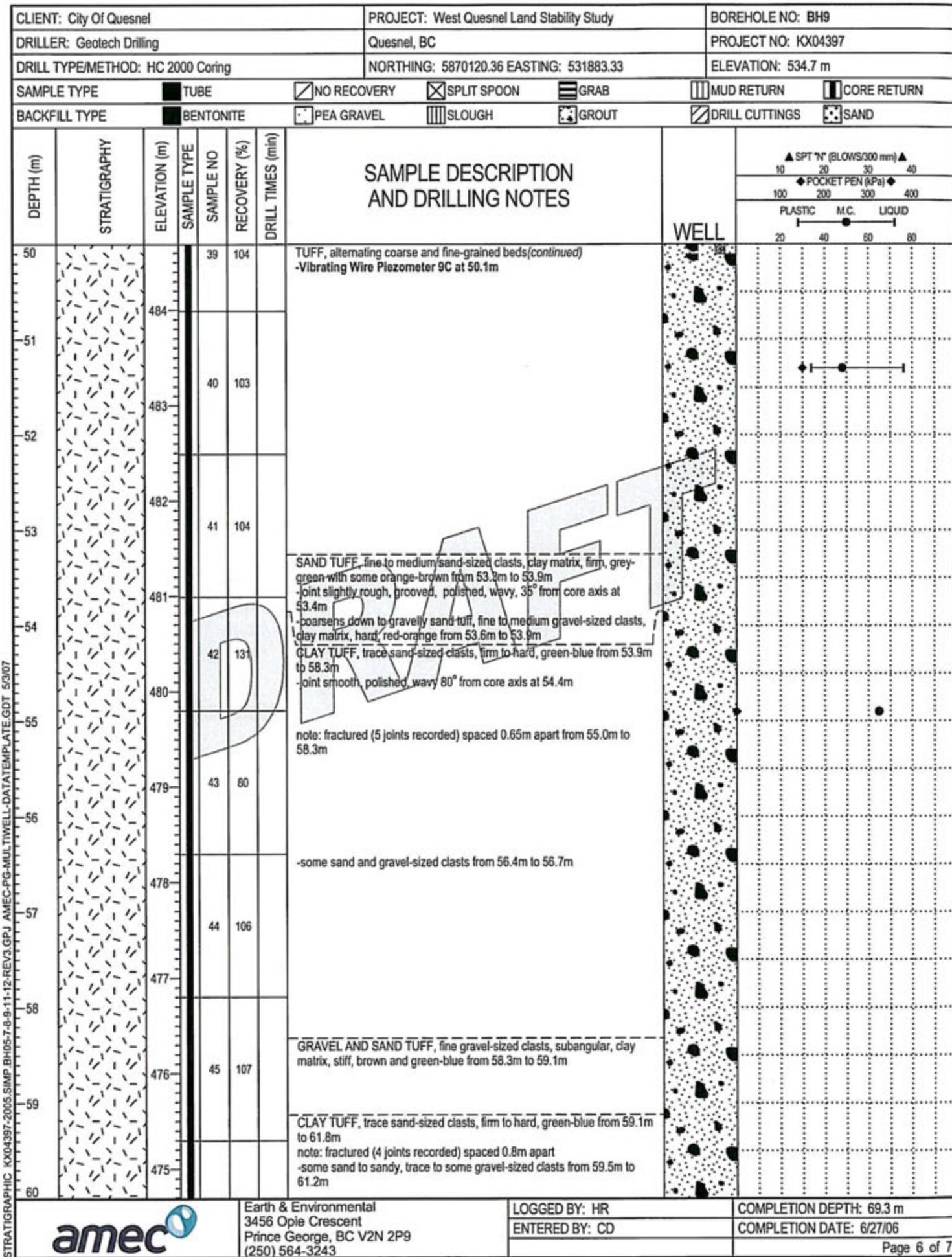


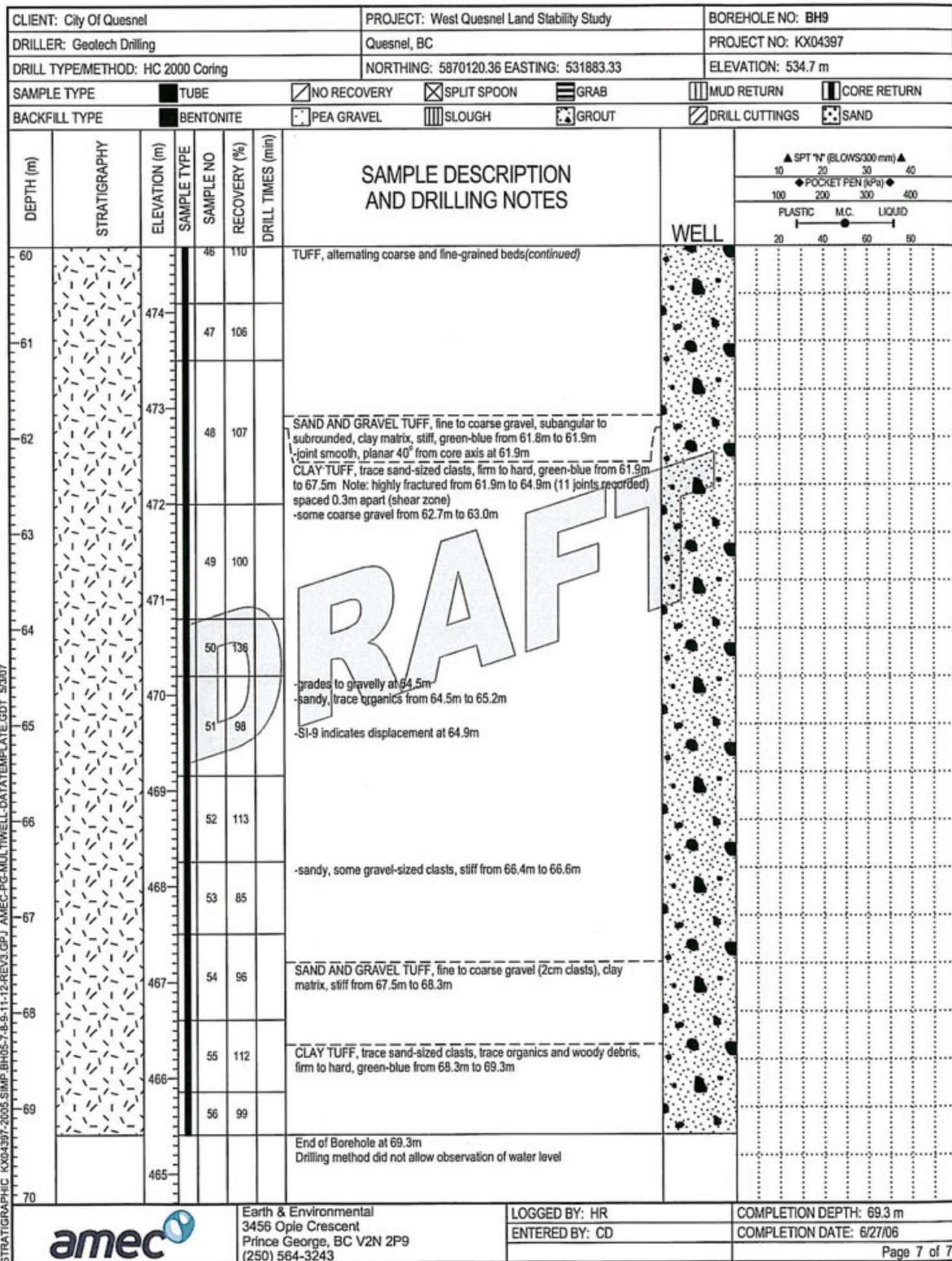


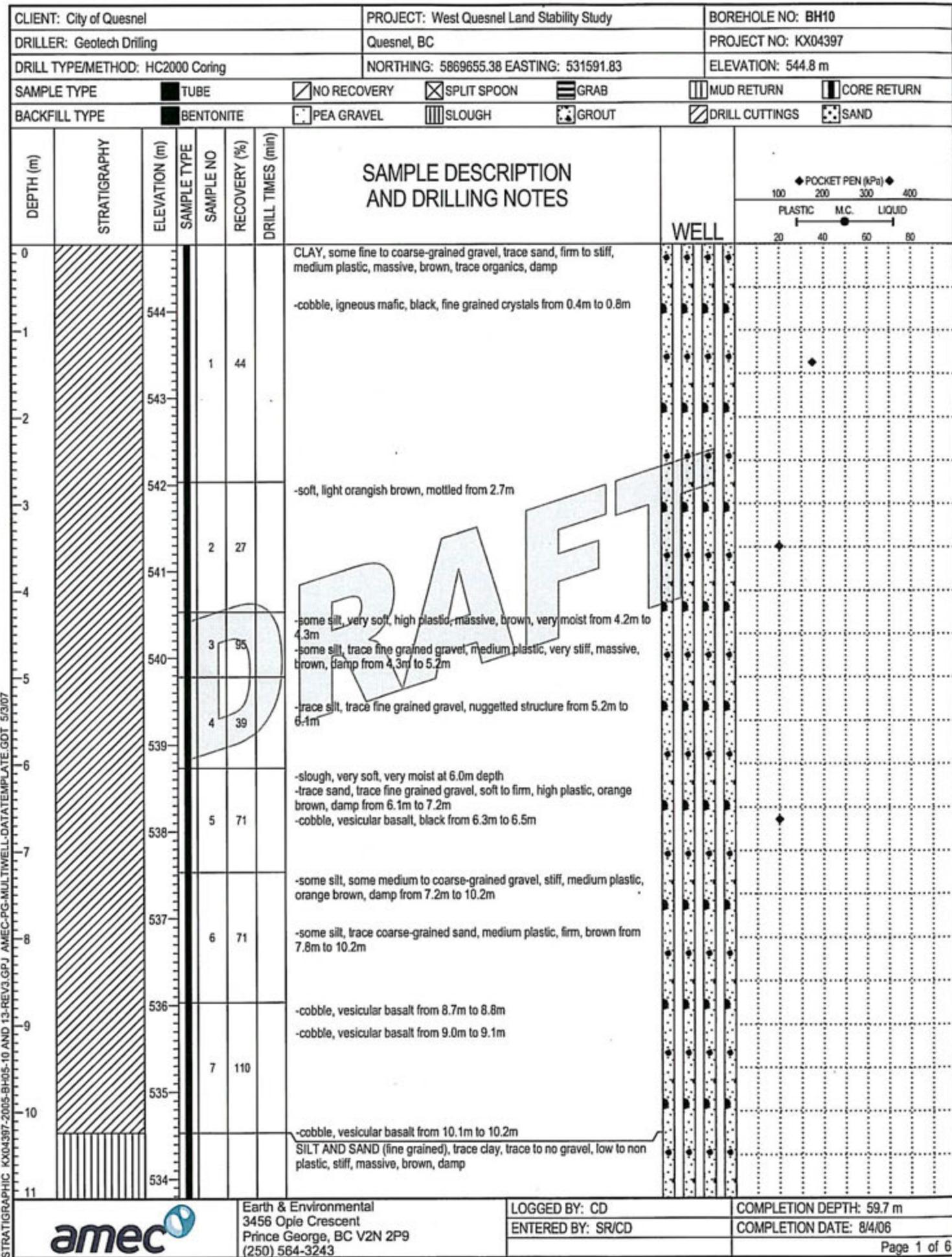


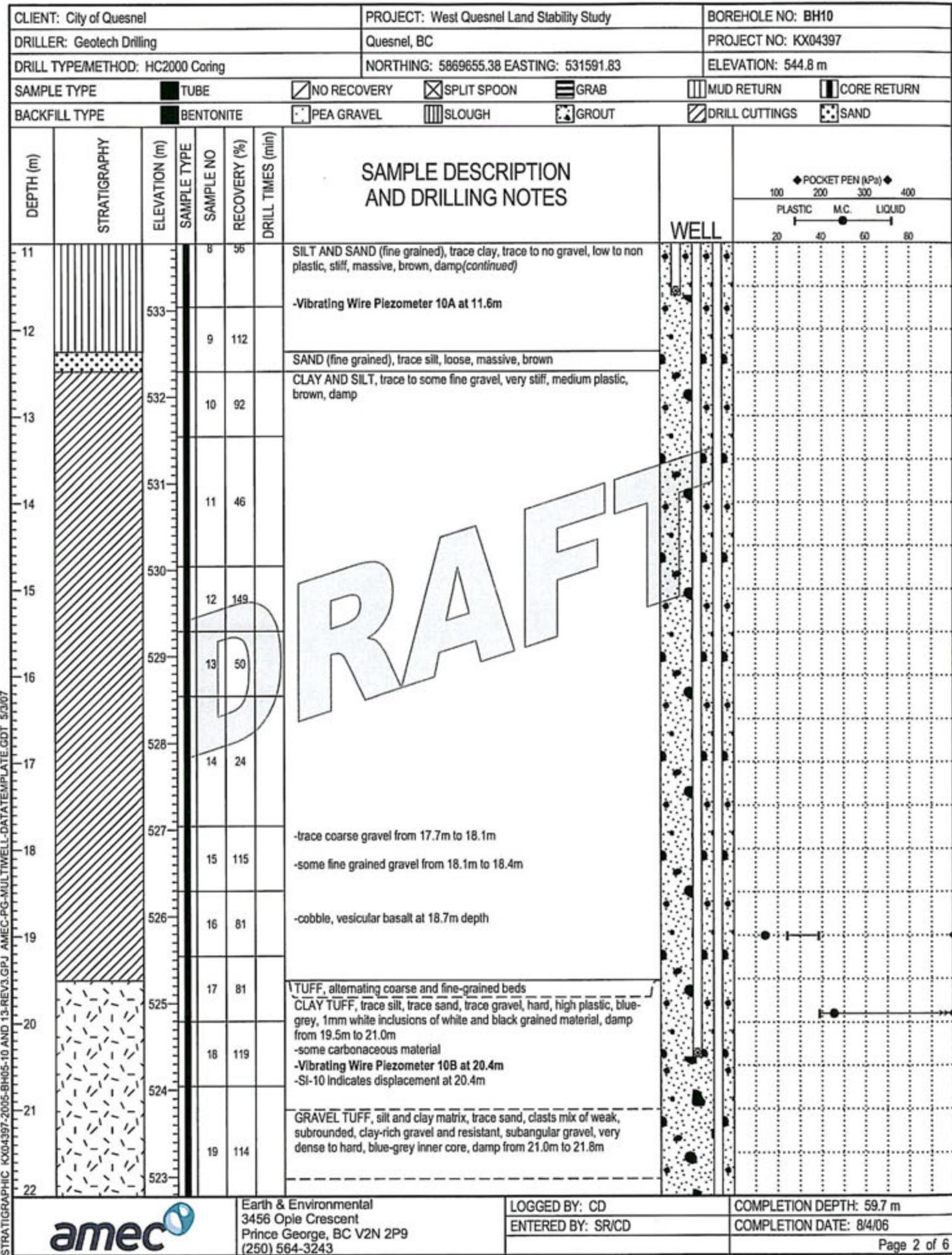




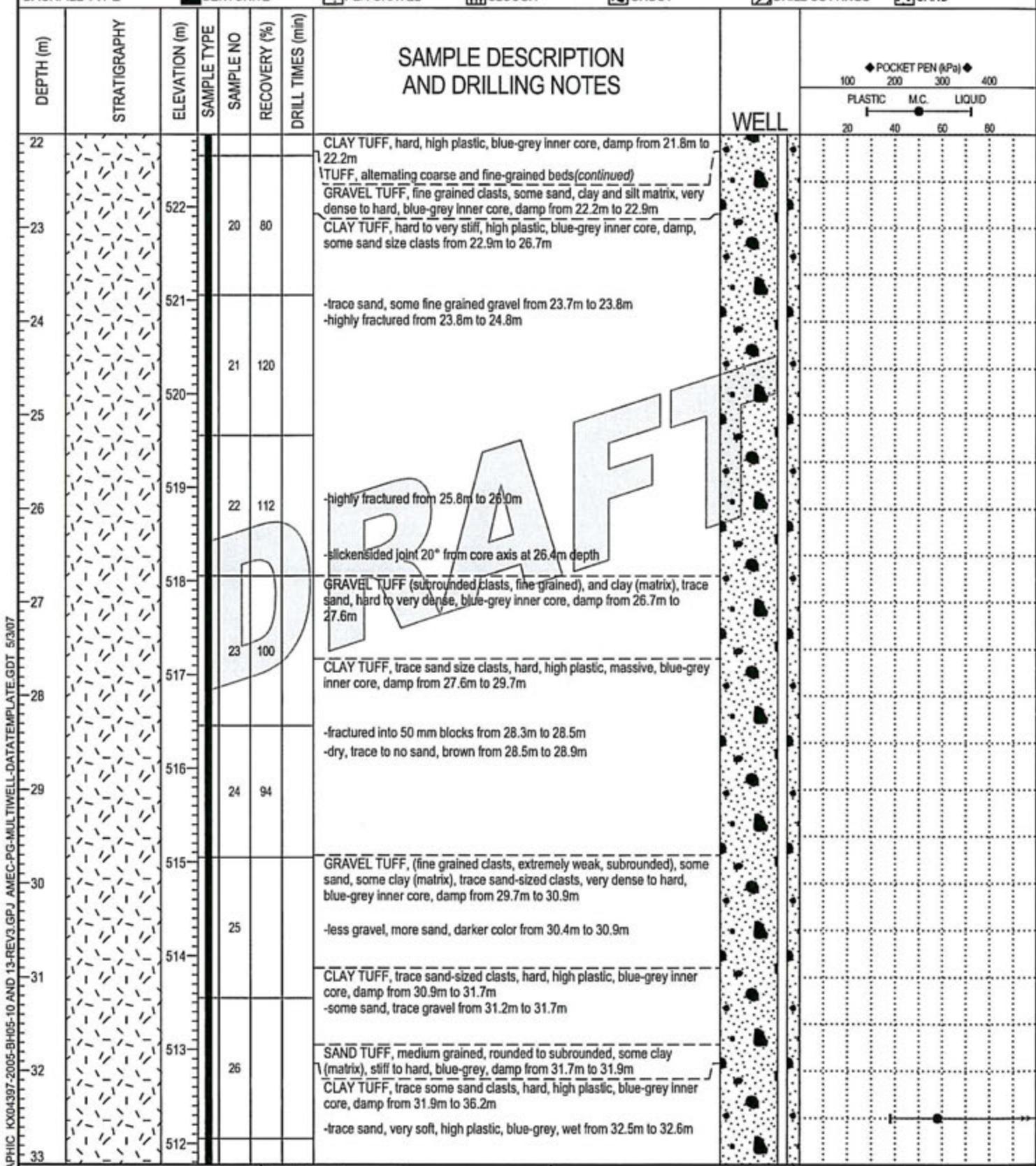




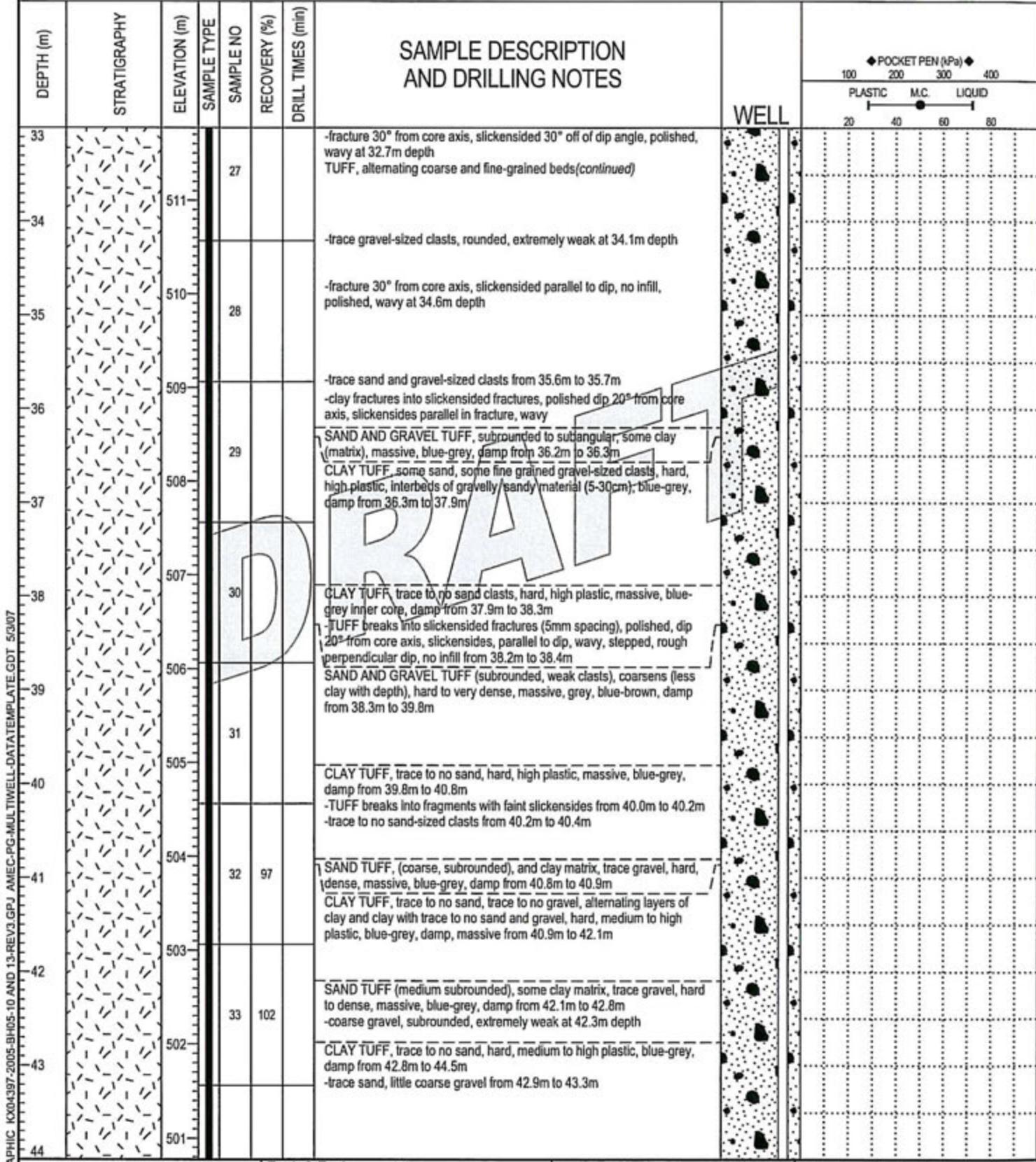




CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH10
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000 Coring	NORTHING: 5869655.38 EASTING: 531591.83	ELEVATION: 544.8 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH10
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000 Coring	NORTHING: 5869655.38 EASTING: 531591.83	ELEVATION: 544.8 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



STRATIGRAPHIC KX04397-2005-BH05-10 AND 13-REV3.GPJ AMEC PG MULTIWELL.DATATEMPLATE.GDT 5/2/2007



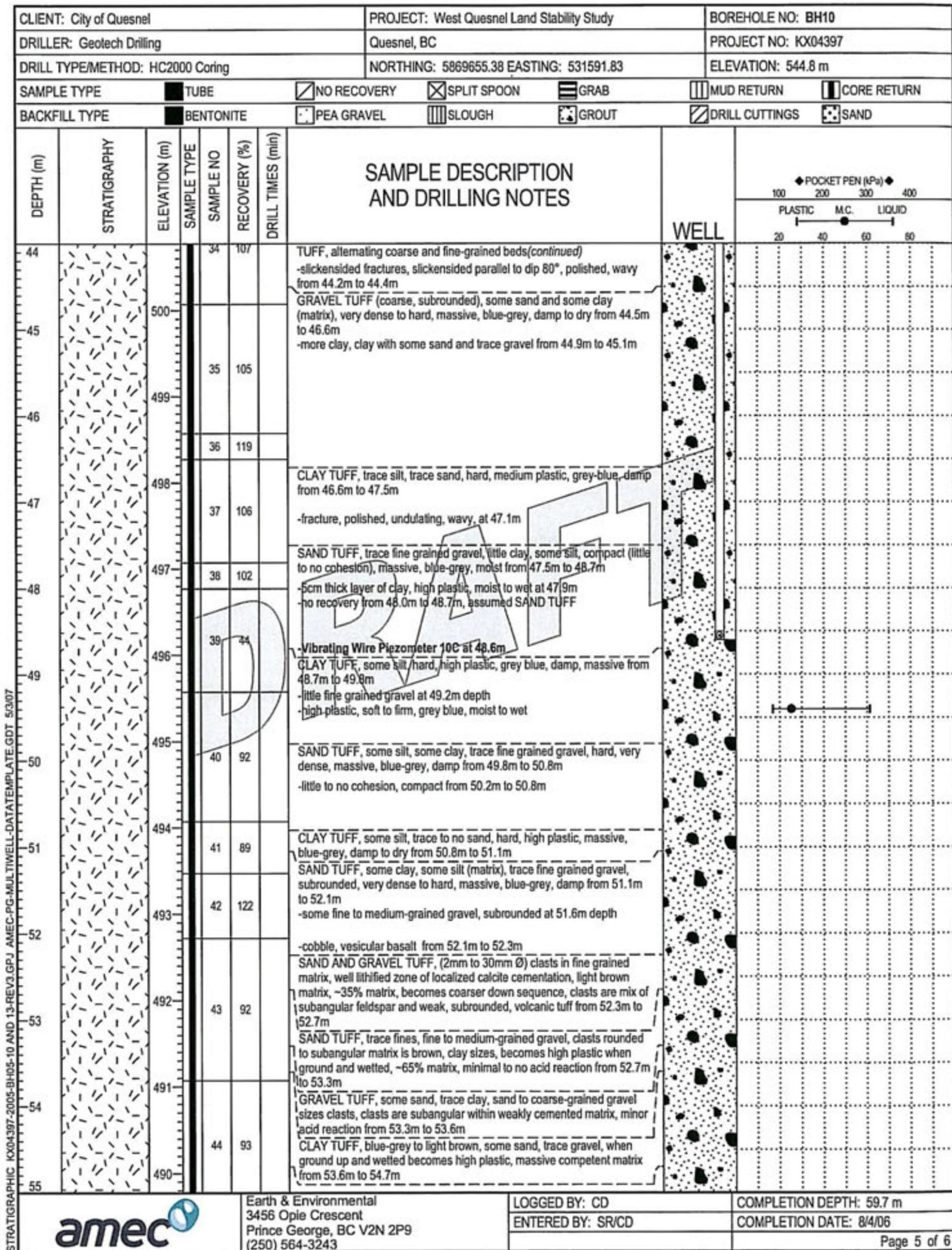
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

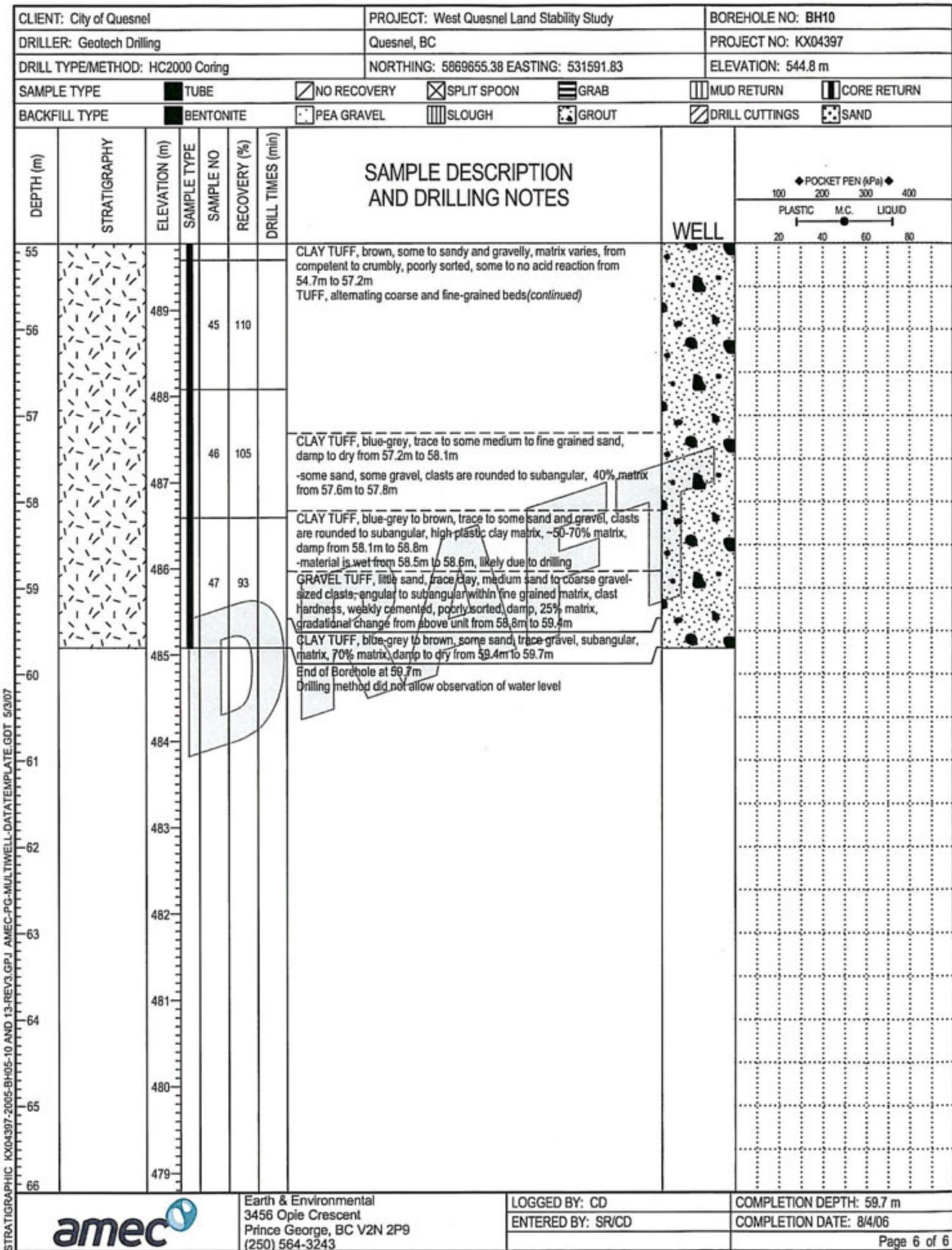
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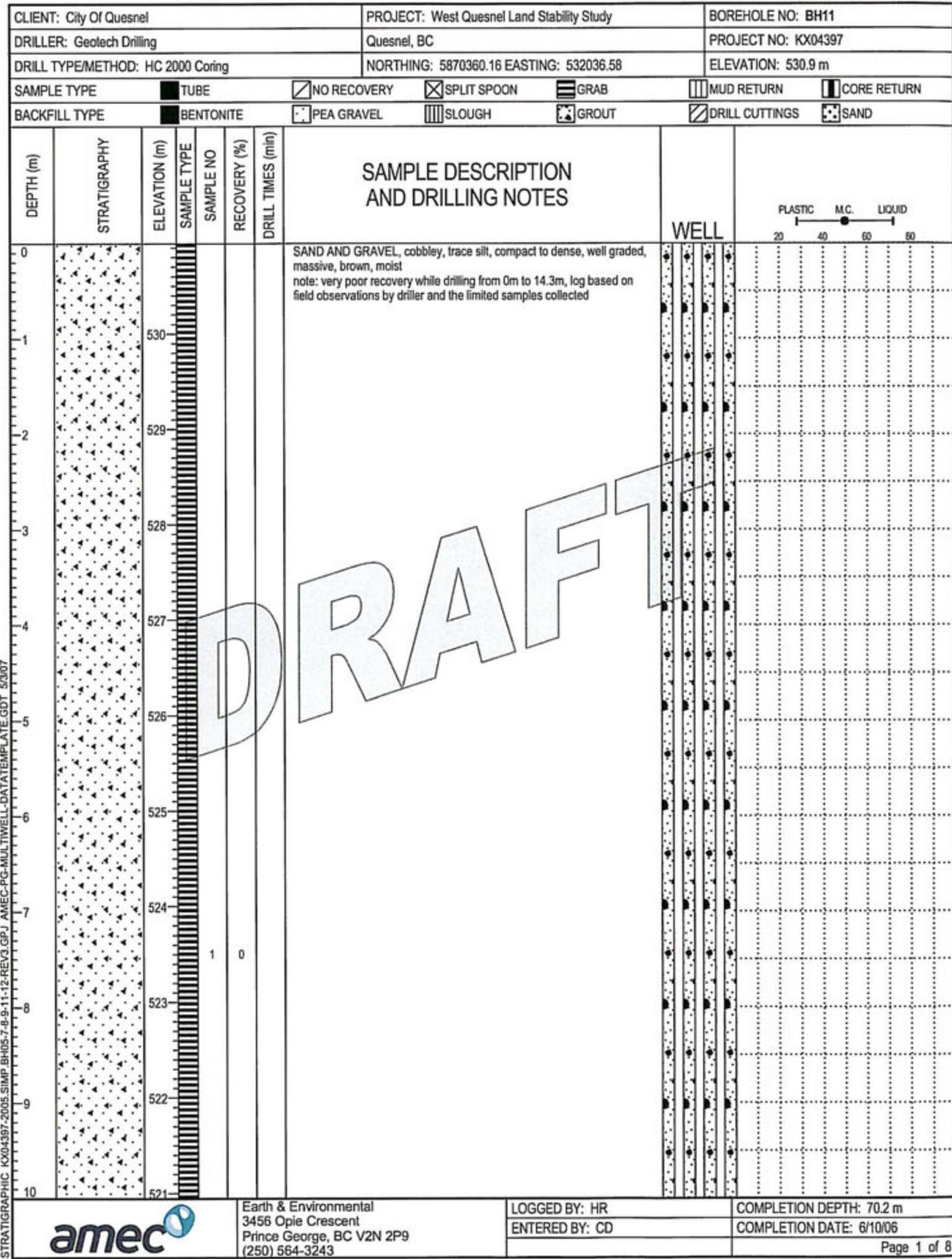
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COMPLETION DEPTH: 59.7 m

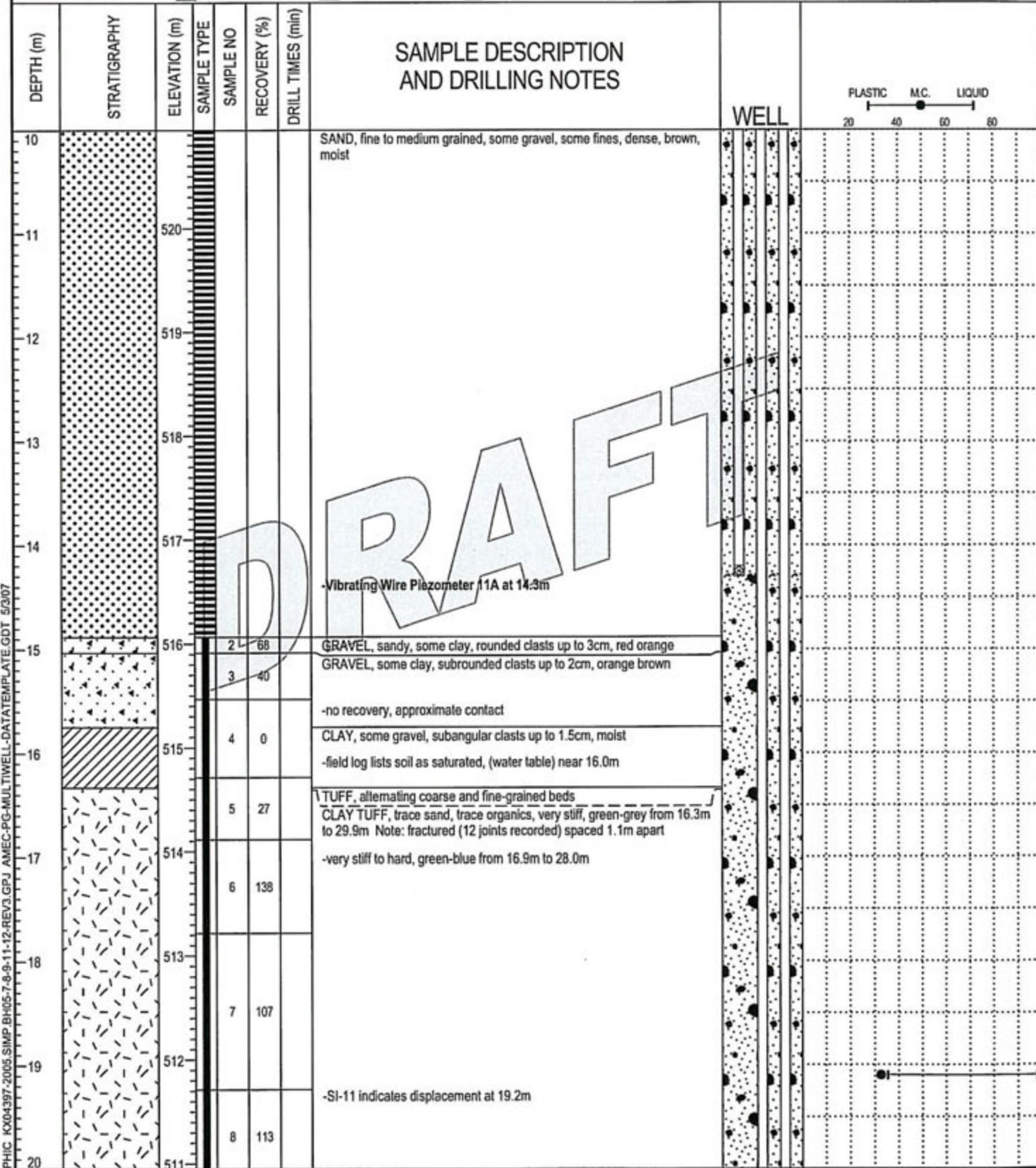
COMPLETION DATE: 8/4/06







CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



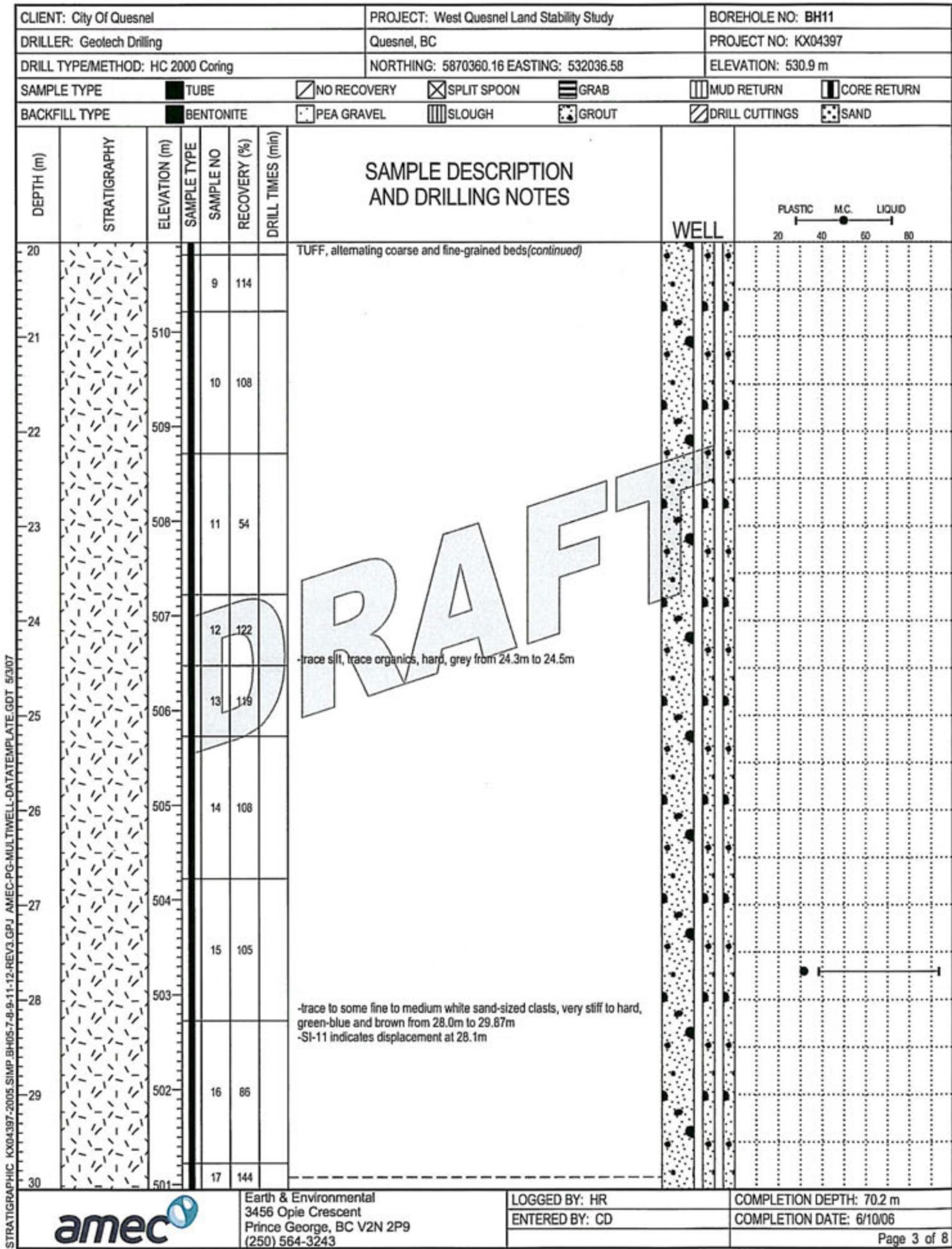
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: HR

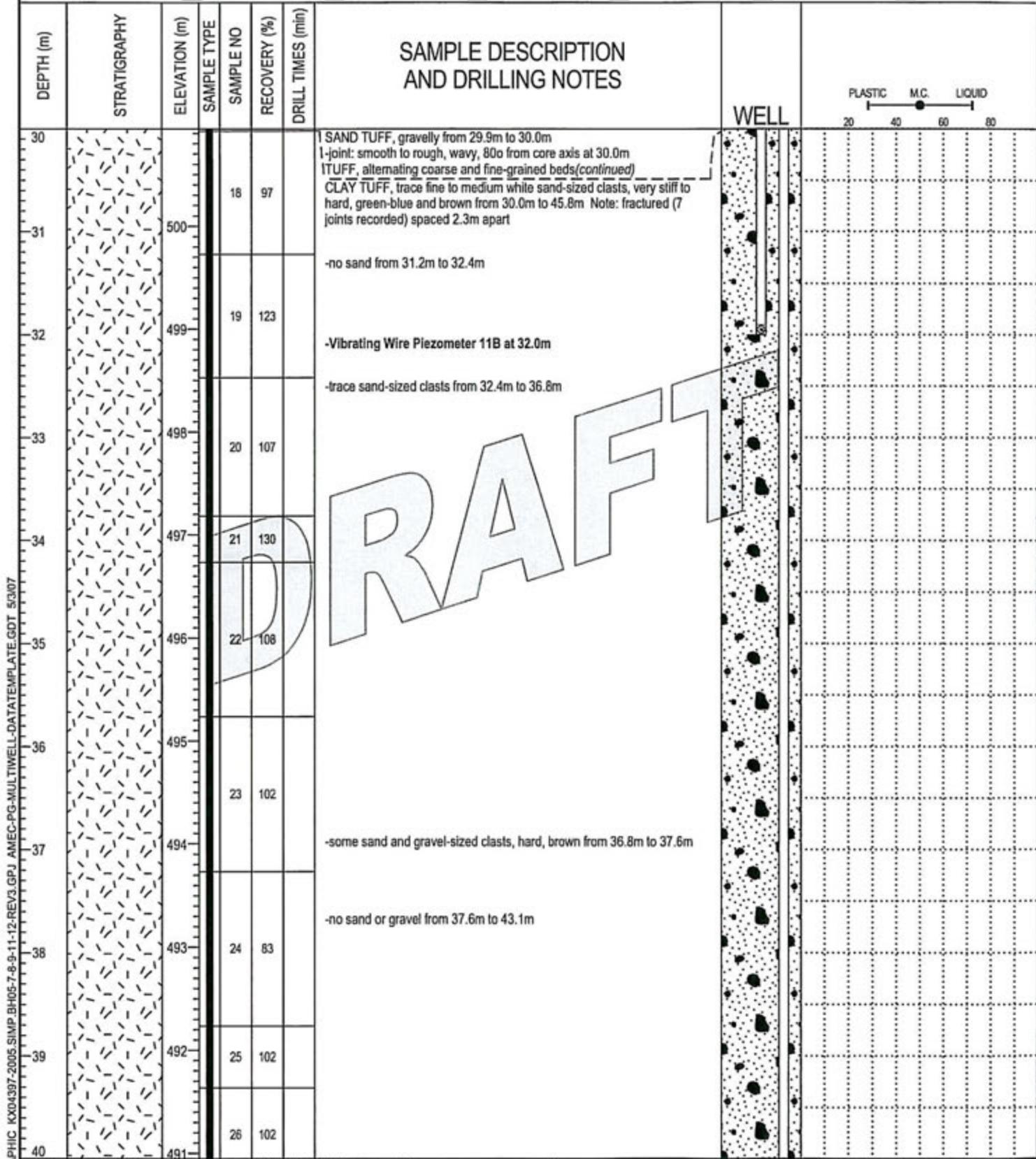
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COMPLETION DEPTH: 70.2 m

COMPLETION DATE: 6/10/06



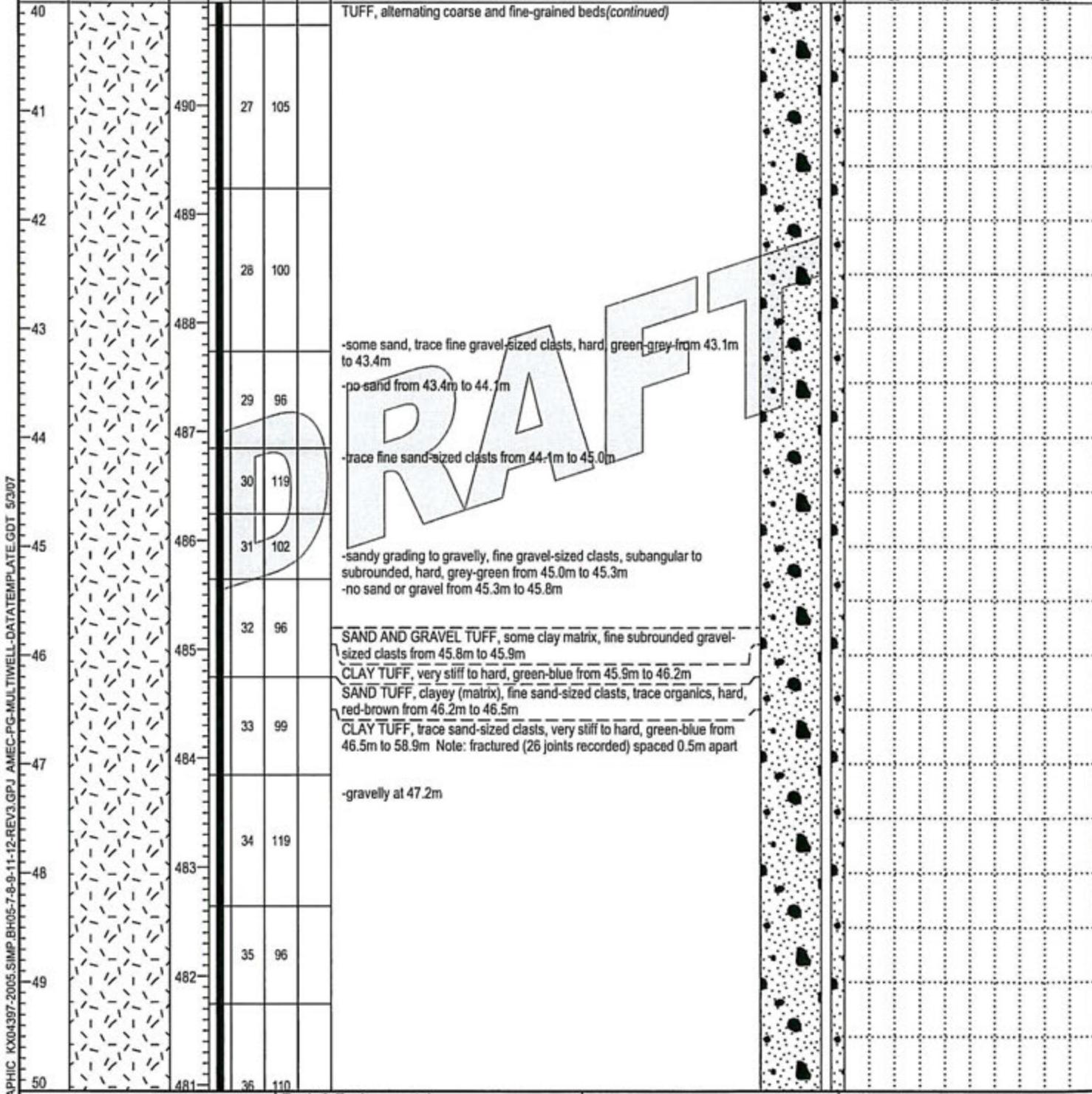
CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



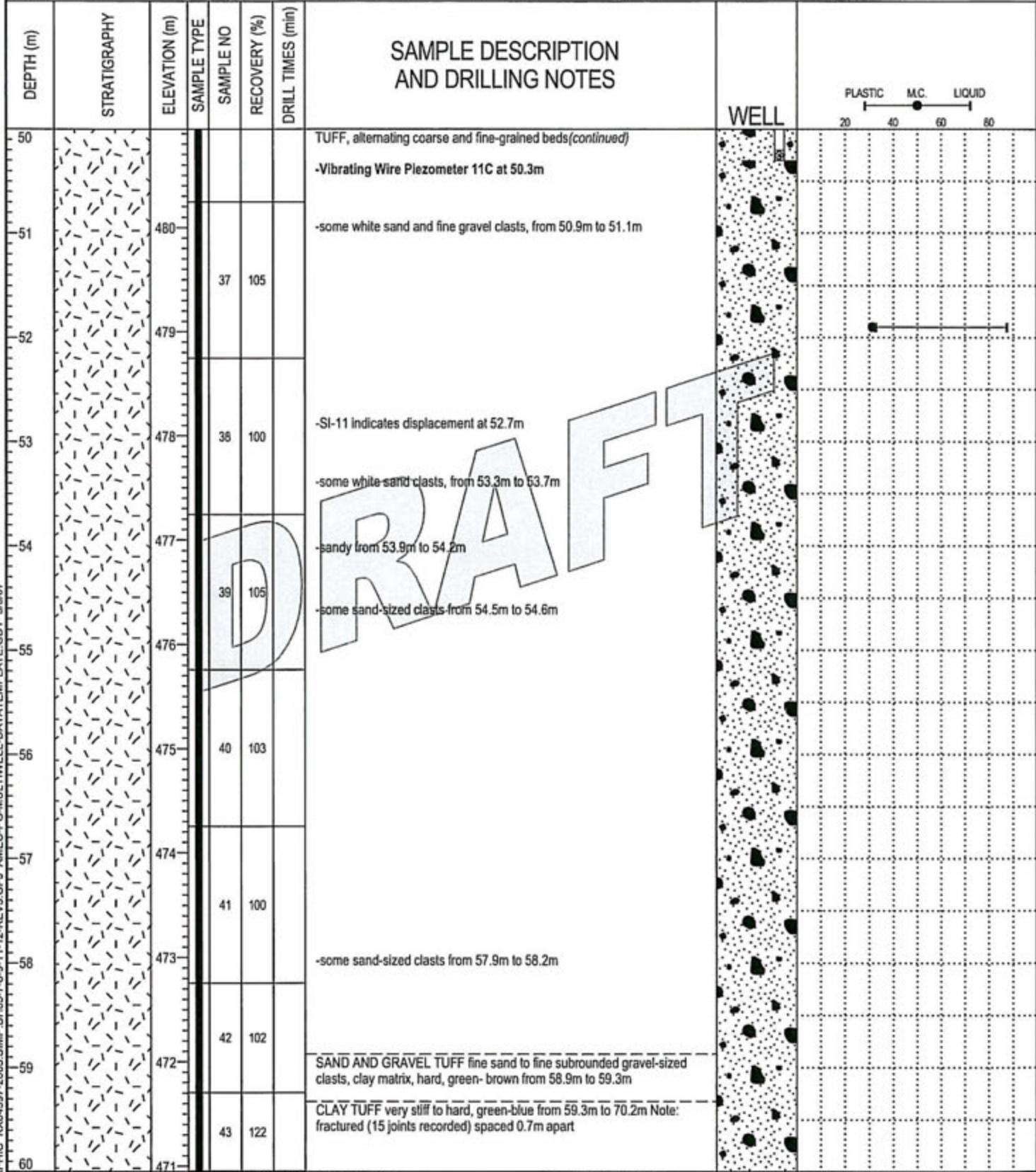
CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		

SAMPLE DESCRIPTION AND DRILLING NOTES

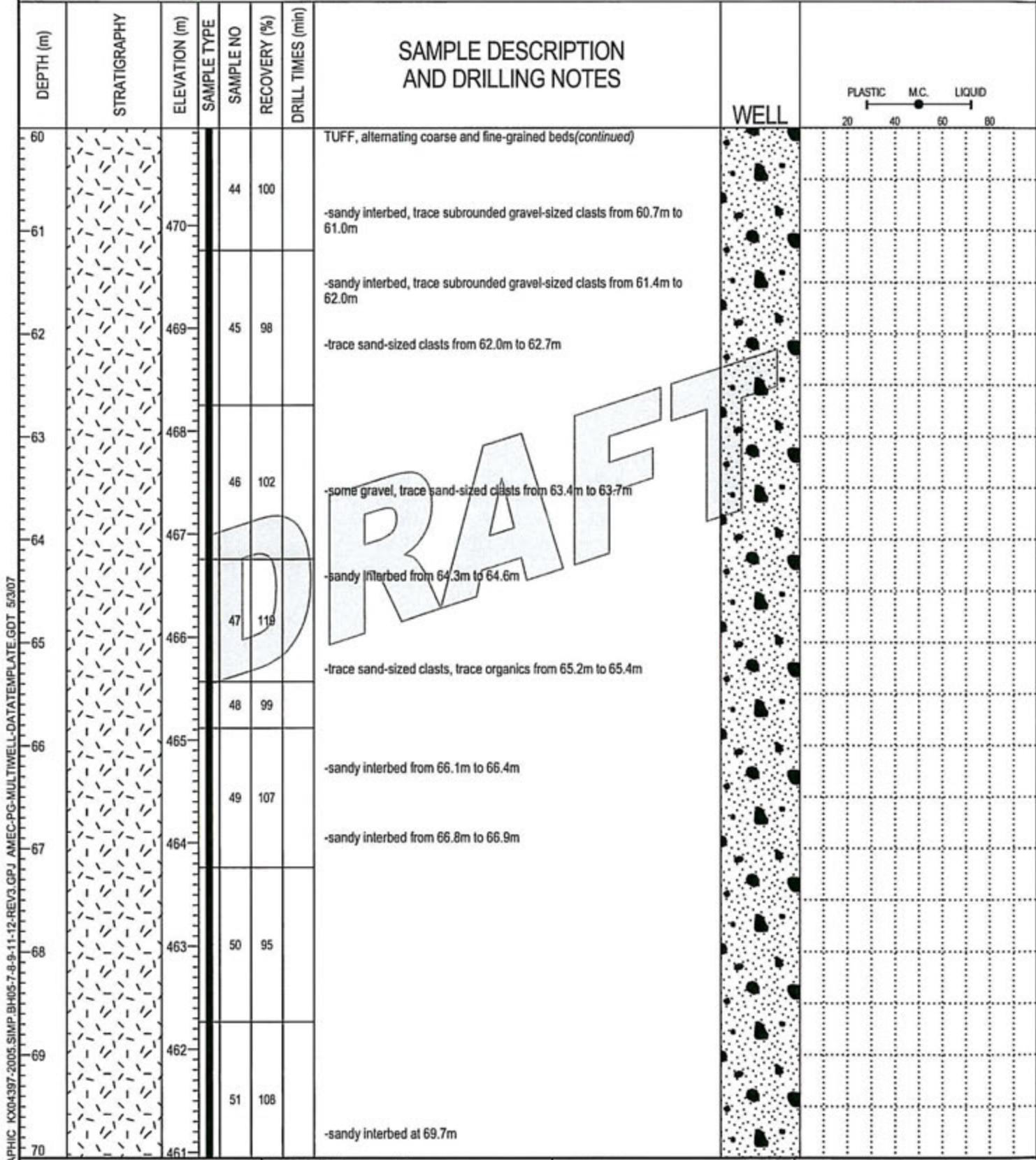
PLASTIC M.C. LIQUID
20 40 60 80



CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input checked="" type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH11
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870360.16 EASTING: 532036.58	ELEVATION: 530.9 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB		<input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT		<input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND

STRATIGRAPHY

ELEVATION (m)

SAMPLE TYPE

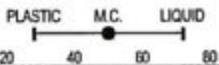
SAMPLE NO

RECOVERY (%)

DRILL TIMES (min)

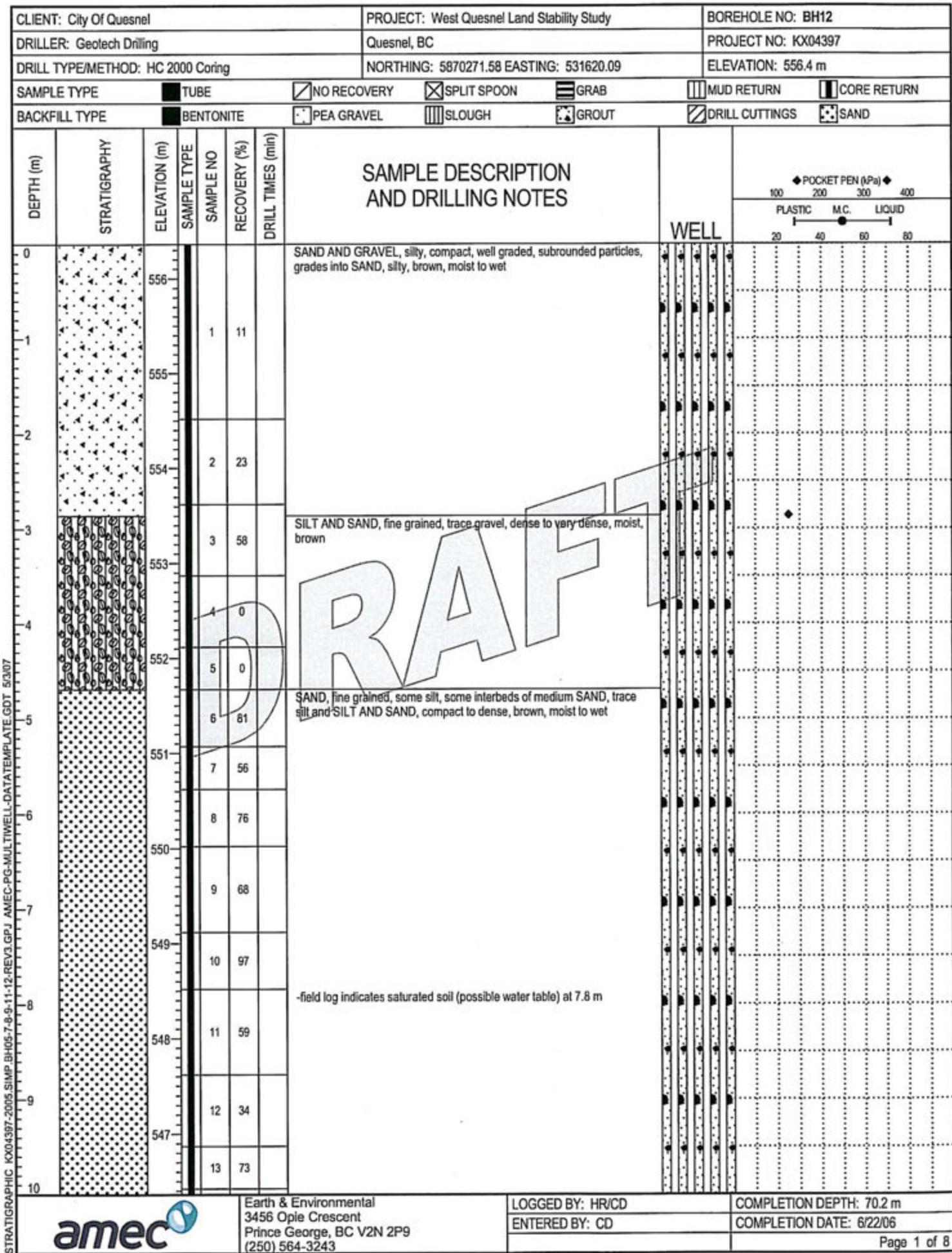
SAMPLE DESCRIPTION AND DRILLING NOTES

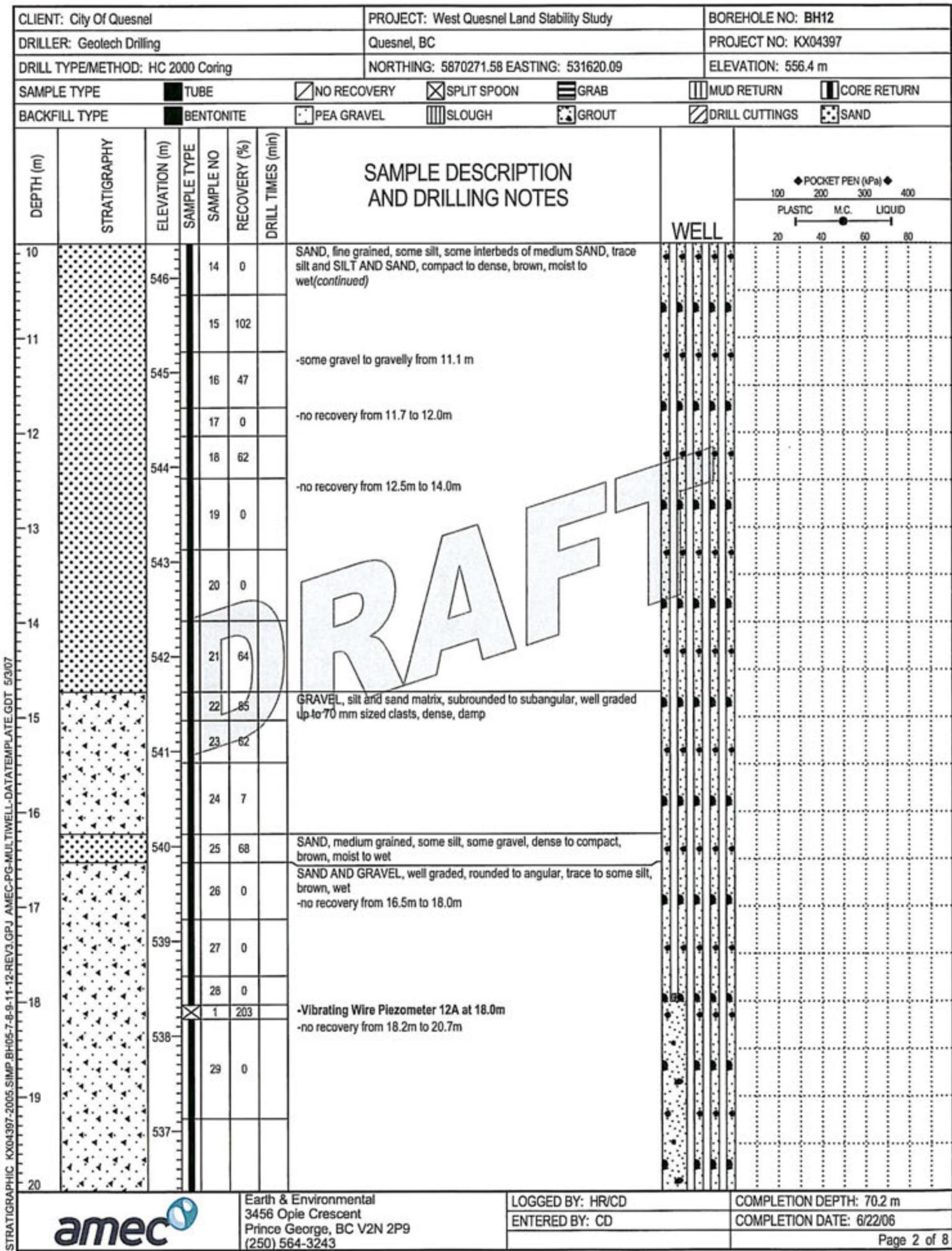
WELL



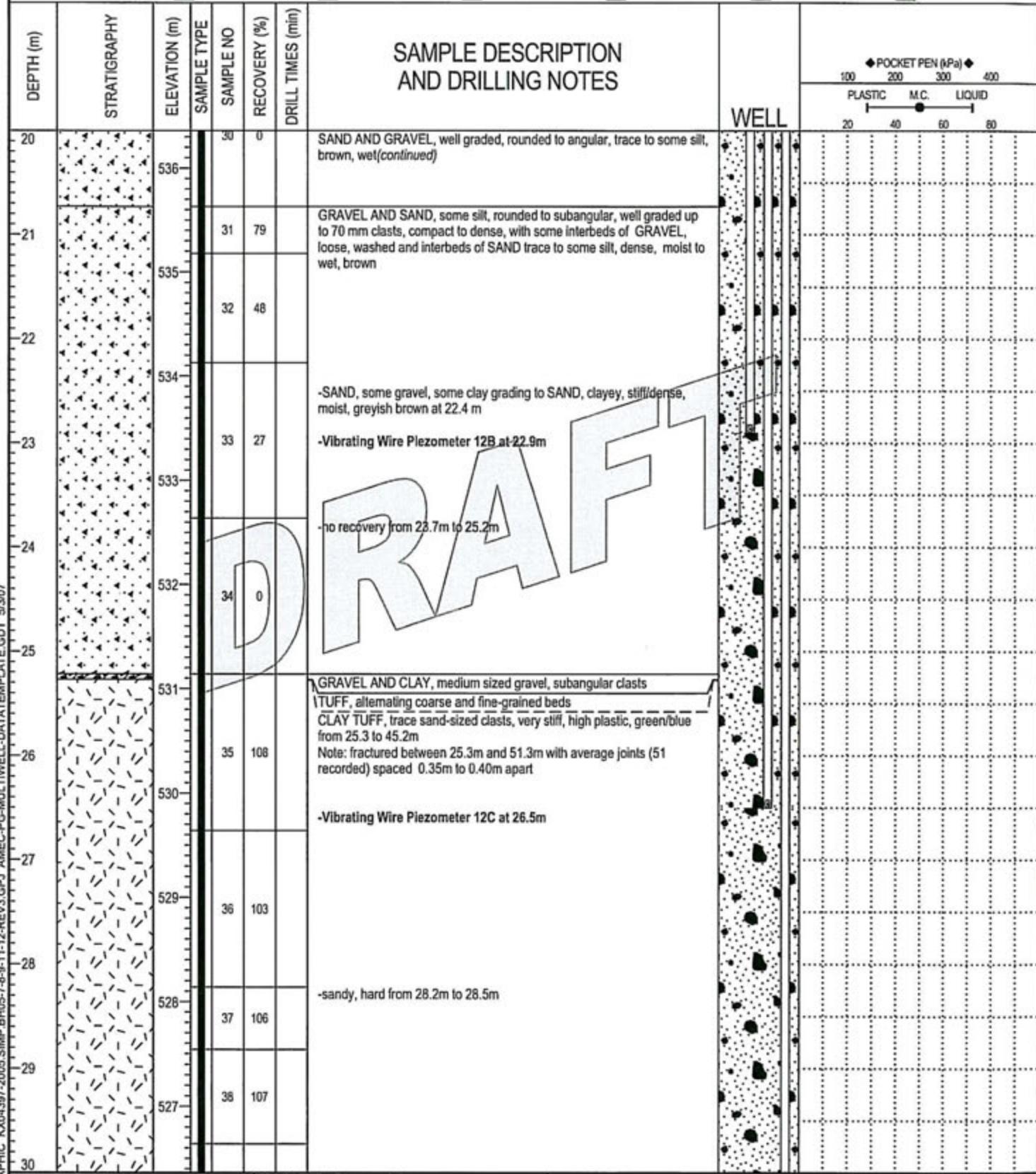
70	✓ ✓ ✓ ✓	■	TUFF, alternating coarse and fine-grained beds(continued) End of Borehole at 70.2m Drilling method did not allow observation of water level				
71		460					
72		459					
73		458					
74		457					
75		456					
76		455					
77		454					
78		453					
79		452					
80		451					

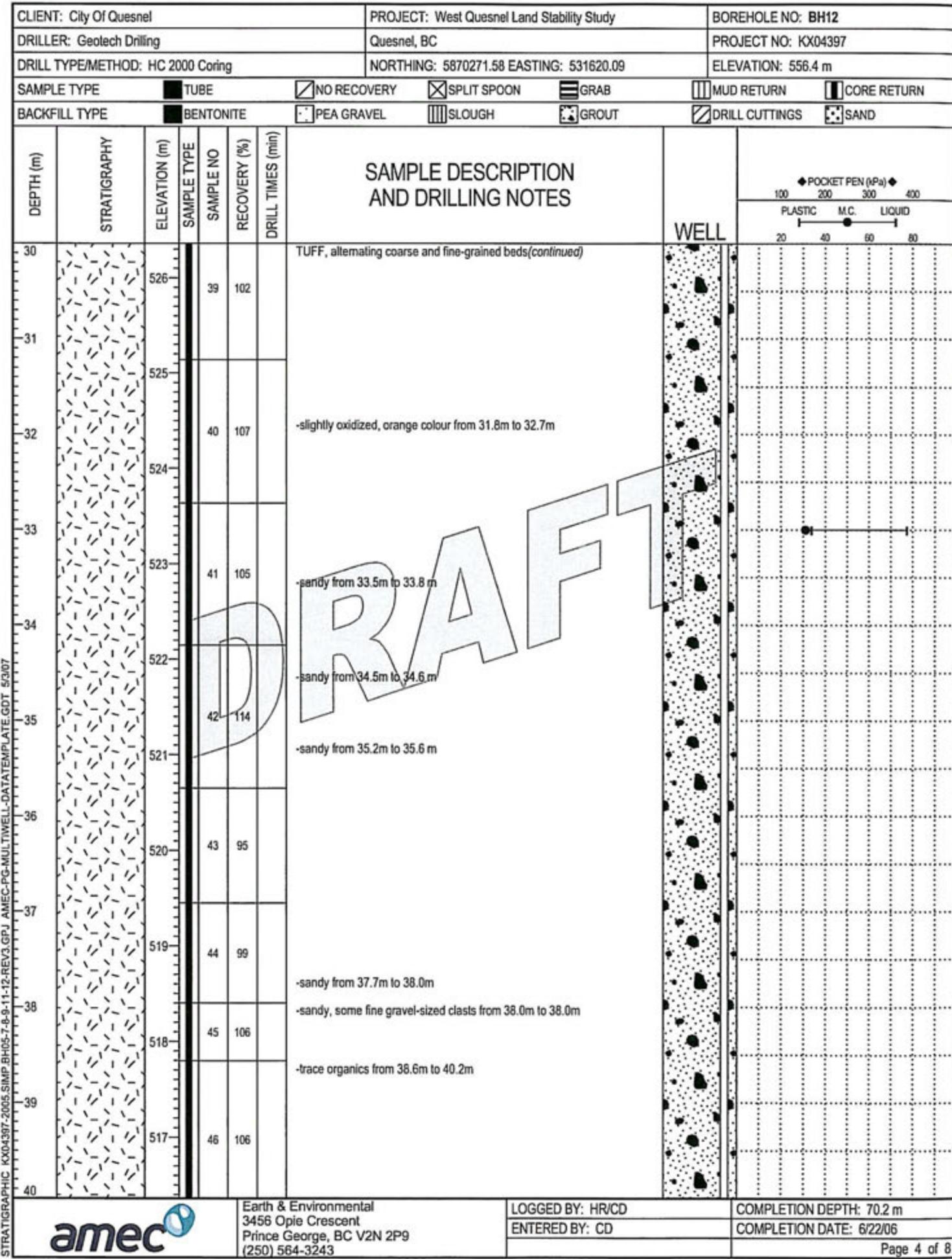
DRAFT

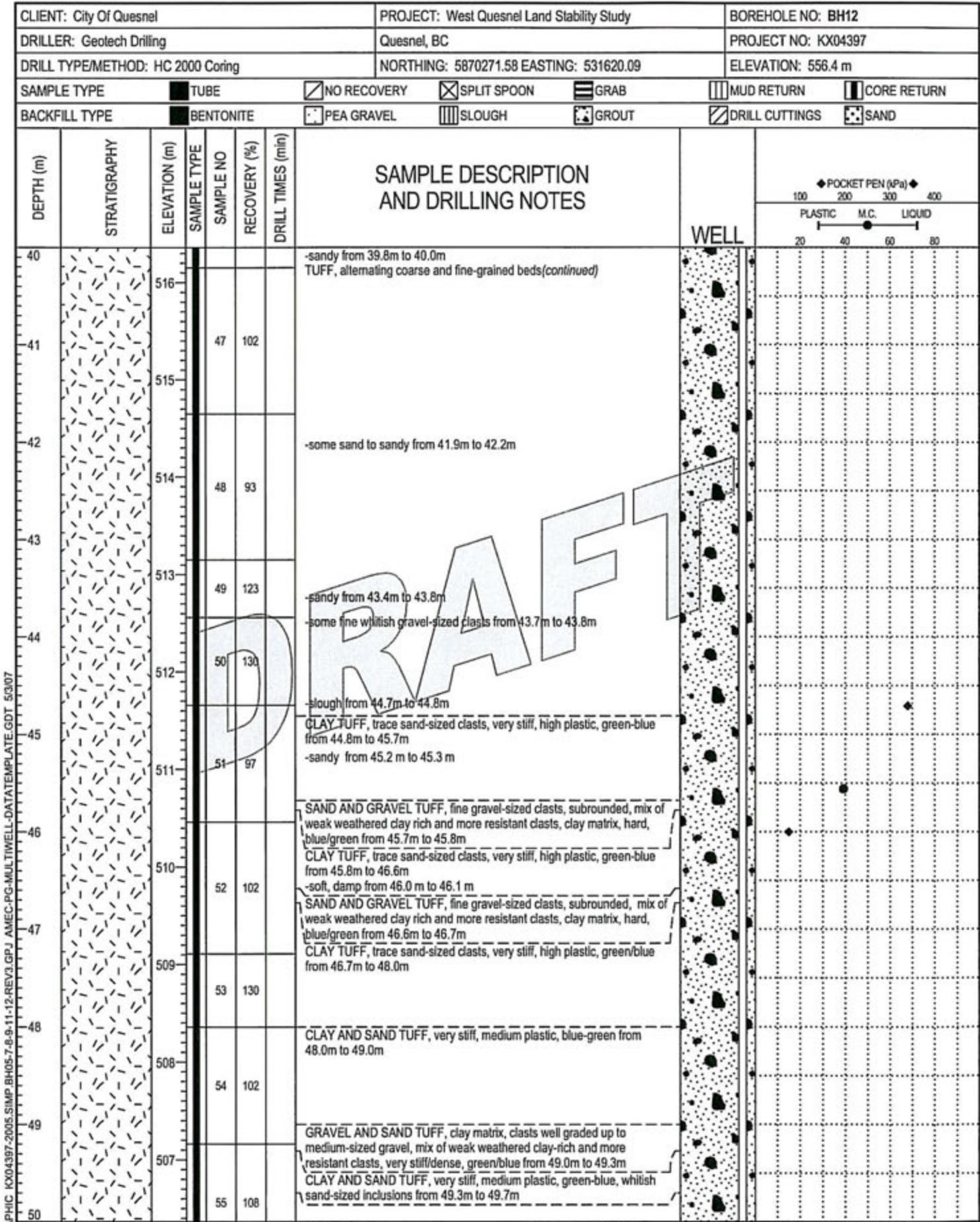




CLIENT: City Of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH12
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC 2000 Coring	NORTHING: 5870271.58 EASTING: 531620.09	ELEVATION: 556.4 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		







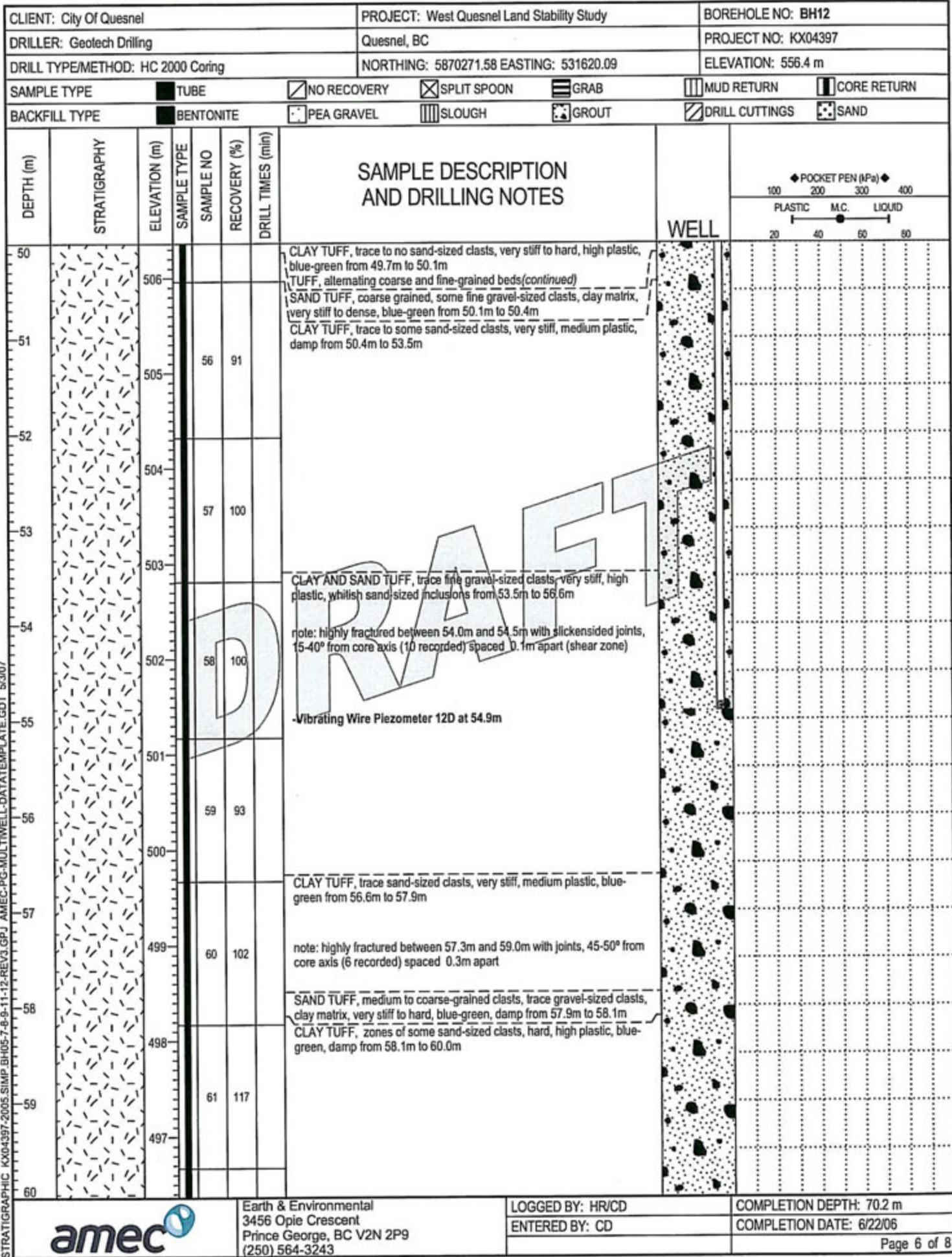
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

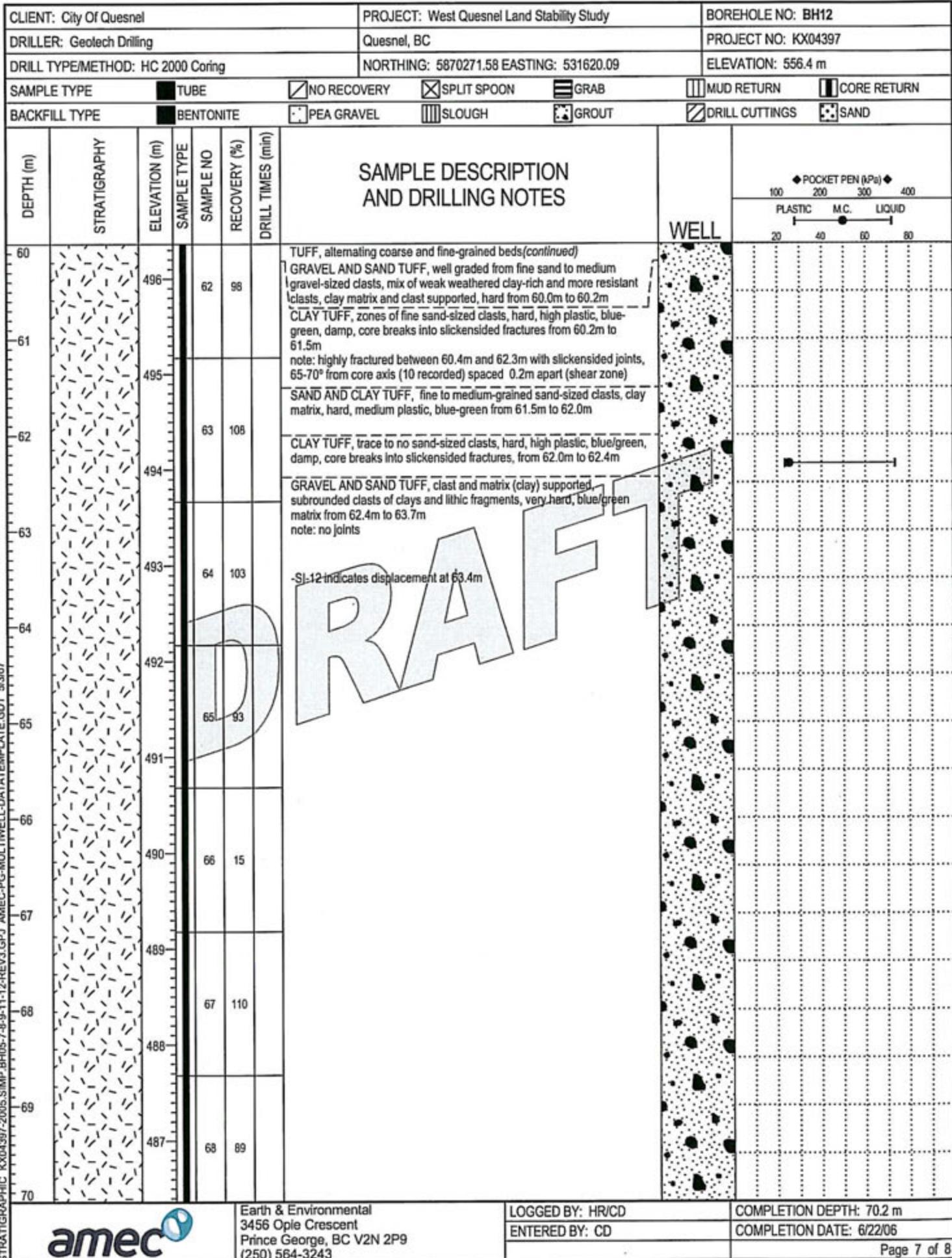
LOGGED BY: HR/CD

ENTERED BY: CD

COMPLETION DEPTH: 70.2 m

COMPLETION DATE: 6/22/06





STRATIGRAPHIC KX04397-2005.SIMP.BH057-8-9-11-12-REV3.GPJ AMEC-PCG-MULTIWELL-DATA TEMPLATE.GDT 5/3/07



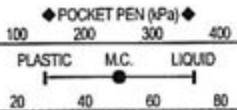
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: HR/CD
ENTERED BY: CD

COMPLETION DEPTH: 70.2 m
COMPLETION DATE: 6/22/06

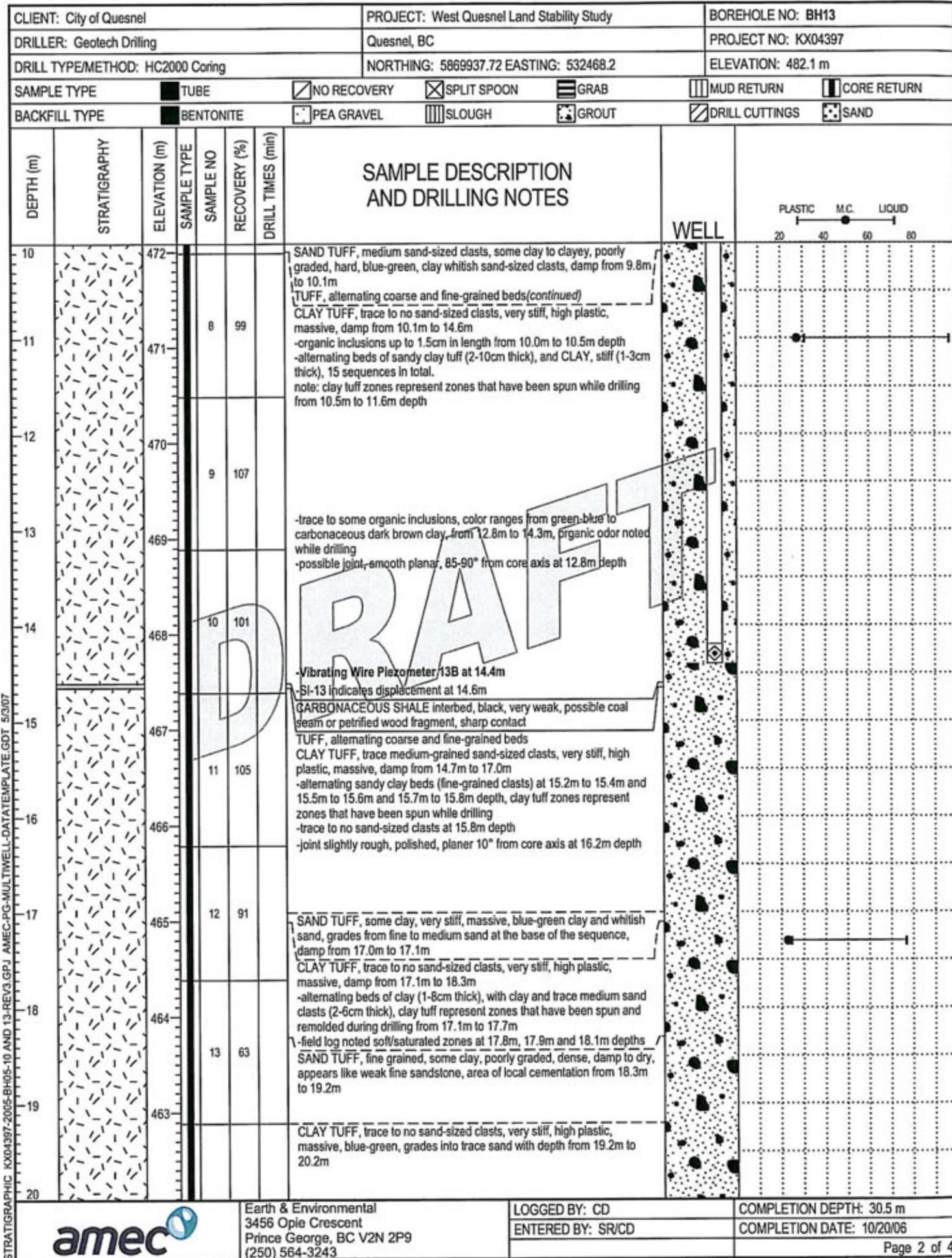
CLIENT: City Of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH12			
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397			
DRILL TYPE/METHOD: HC 2000 Coring				NORTHING: 5870271.58 EASTING: 531620.09				ELEVATION: 556.4 m			
SAMPLE TYPE	TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN					
BACKFILL TYPE	BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND					
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES				
70	486					TUFF, alternating coarse and fine-grained beds(continued) SAND TUFF interbed, fine grained, some clay, trace to some gravel-sized clasts, supported by clay and sand matrix, blue-green from 63.7m to 70.2m note: no joints -coarse sand and fine gravel-sized clasts from 64.8 m to 65.5 m -coarse sand and fine gravel-sized clasts from 66.3 m to 66.4 m -coarse sand and fine gravel-sized clasts from 67.0 m to 67.2 m -coarse sand and fine gravel-sized clasts from 68.7 m to 68.9 m End of Borehole at 70.2m Drilling method did not allow observation of water level			WELL	
71		485									
72		484									
73		483									
74		482									
75		481									
76		480									
77		479									
78		478									
79		477									
80											
STRATIGRAPHIC KX04397-2005-SIMP-BH05-7-8-9-11-12-REV3.GPJ AMEC-PC-MULTIWELL-DATAWELL-TEMPLATE.GDT 5/30/07											
 amec			Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: HR/CD	COMPLETION DEPTH: 70.2 m			
			ENTERED BY: CD					COMPLETION DATE: 6/22/06			
								Page 8 of 8			

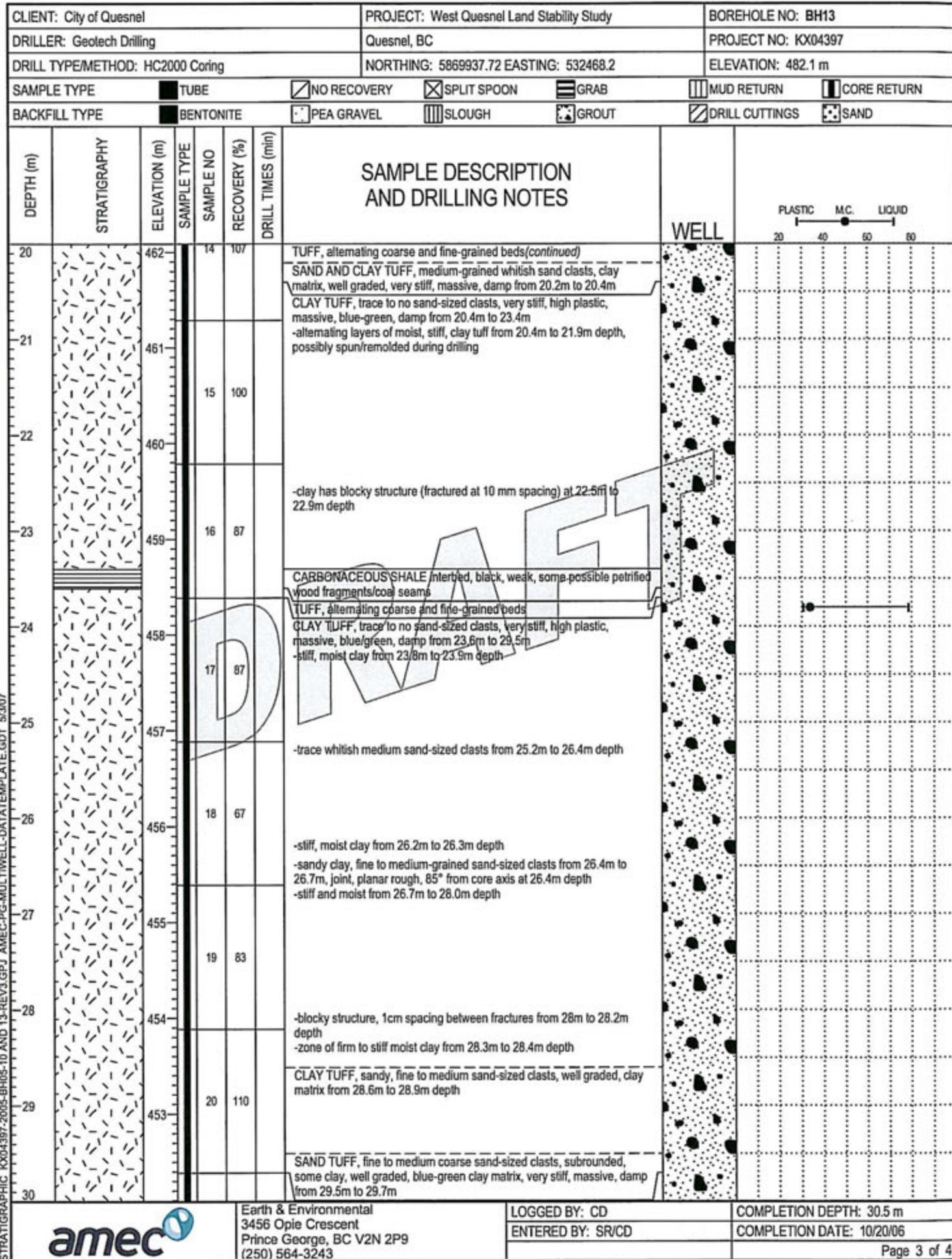
SAMPLE DESCRIPTION AND DRILLING NOTES



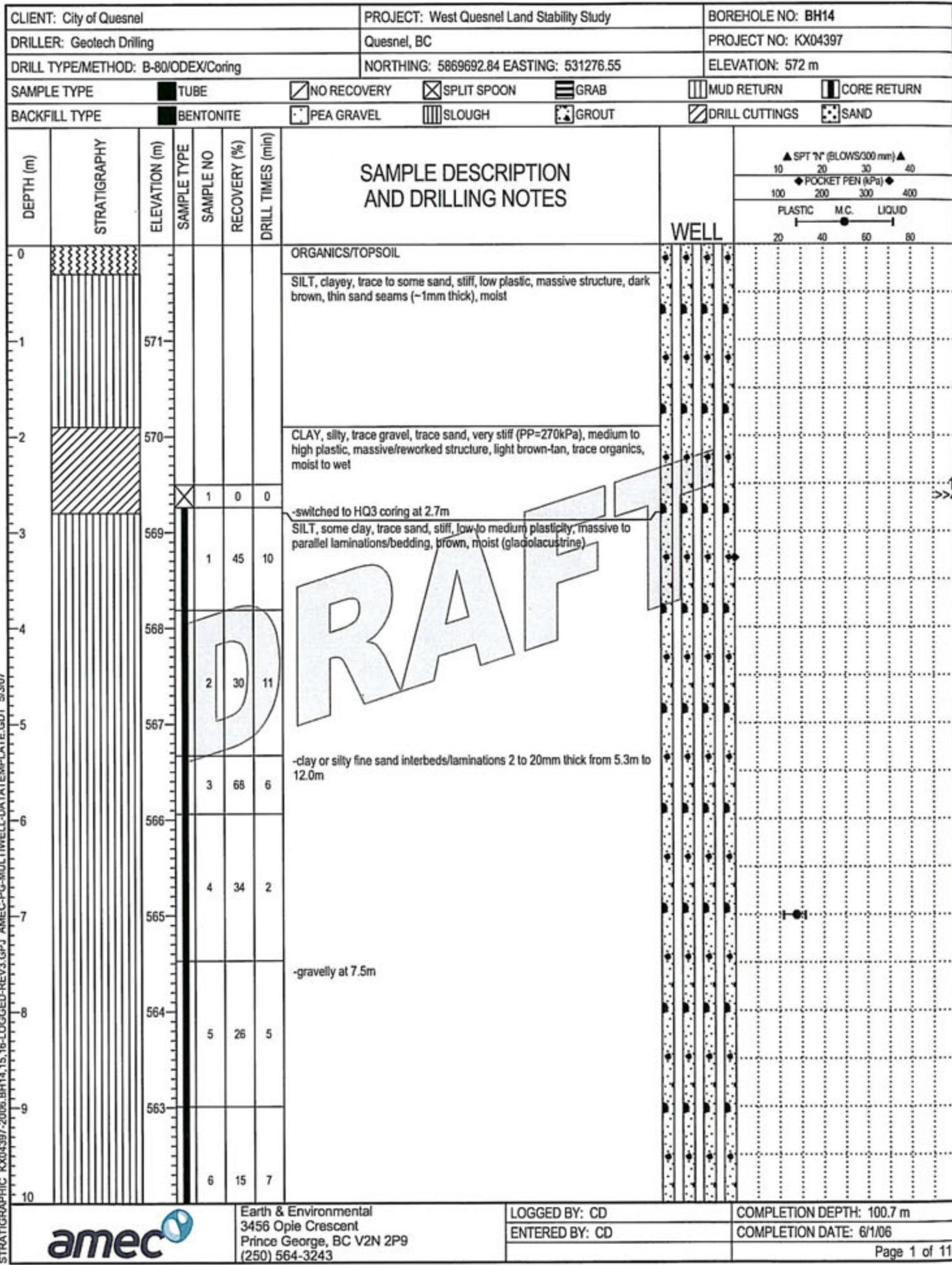
DRAFT

CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH13				
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397				
DRILL TYPE/METHOD: HC2000 Coring				NORTHING: 5869937.72 EASTING: 532468.2				ELEVATION: 482.1 m				
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN						
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND						
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES					
0		482					COBBLES, some silt, some clay, trace sand, loose, cobbles are volcanic andesite					
1		481		1	17							
2		480		2	7		COBBLES AND CLAY, firm to stiff, high plastic					
3		479		3	100		TUFF, alternating coarse and fine-grained beds CLAY TUFF, very stiff, high plastic, massive, greyish brown, damp from 2.8m to 6.4m -grey-brown clay grades to dark brown clay from 3.5m to 3.8m -stiff, very high plastic from 4.3m to 4.7m, trace organics -blue-green colour from 4.2m					
4		478		4	103		-trace to some sized clasts from 5.1m to 5.3m -Vibrating Wire Plezometer 13A at 5.1m					
5		477		5	107		SAND TUFF, fine to coarse sand-sized clasts, some clay, very stiff, green-blue, damp from 6.4m to 6.53m CLAY TUFF, very stiff, high plastic, massive, greenish blue, damp, trace organics from 6.5m to 9.9m					
6		476		6	98		-alternating beds (3cm thick) of clay and clay with trace sand from 7.4m to 7.6m -alternating beds (5cm thick) of clay tuff and sandy clay tuff with white medium-grained sand-sized clasts, sandy zones become finer, less sand with depth from 7.6m to 8.3m note: clay tuff zones represent areas which have been spun while drilling					
7		475		7	87							
8		474										
9		473										
10												
WELL												
PLASTIC M.C. LIQUID												
20 40 60 80												





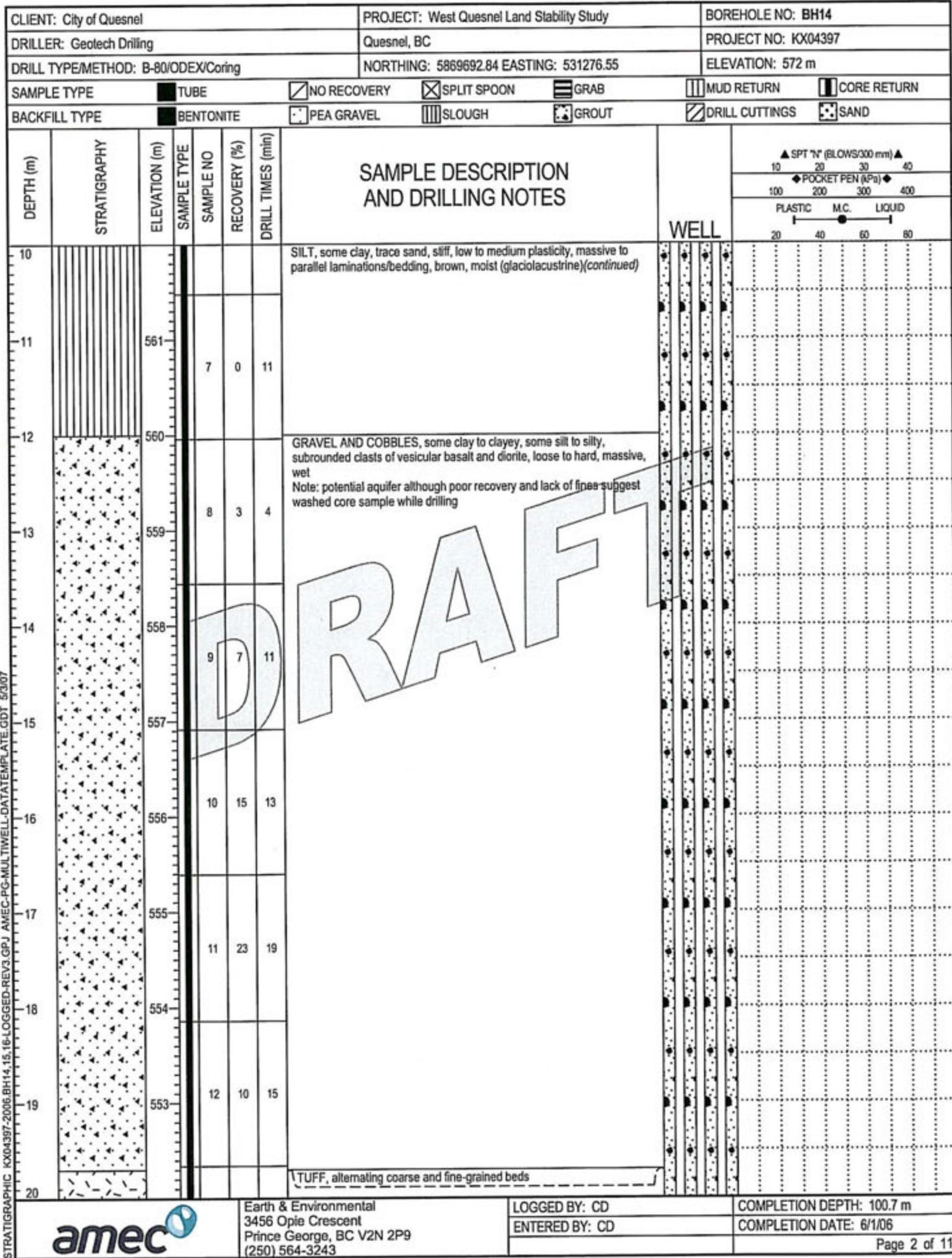
CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH13	
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397	
DRILL TYPE/METHOD: HC2000 Coring				NORTHING: 5869937.72 EASTING: 532468.2				ELEVATION: 482.1 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN			
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND			
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES		
30		452	<input checked="" type="checkbox"/>	21	109		CLAY TUFF, high plastic, stiff to firm, massive, blue-green, moist from 29.7m to 30.5m TUFF, alternating coarse and fine-grained beds(continued) End of Borehole at 30.5m Drilling method did not allow observation of water level		
31		451							
32		450							
33		449							
34		448							
35		447							
36		446							
37		445							
38		444							
39		443							
40									
DRAFT									
STRATIGRAPHIC KX04397-2005-BH05-10 AND 13-REV3.GPJ AMEC-PG-MULTIWELL-DATA TEMPLATE.GOT 5/30/07				Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: CD	COMPLETION DEPTH: 30.5 m
				ENTERED BY: SR/CD				COMPLETION DATE: 10/20/06	
								Page 4 of 4	

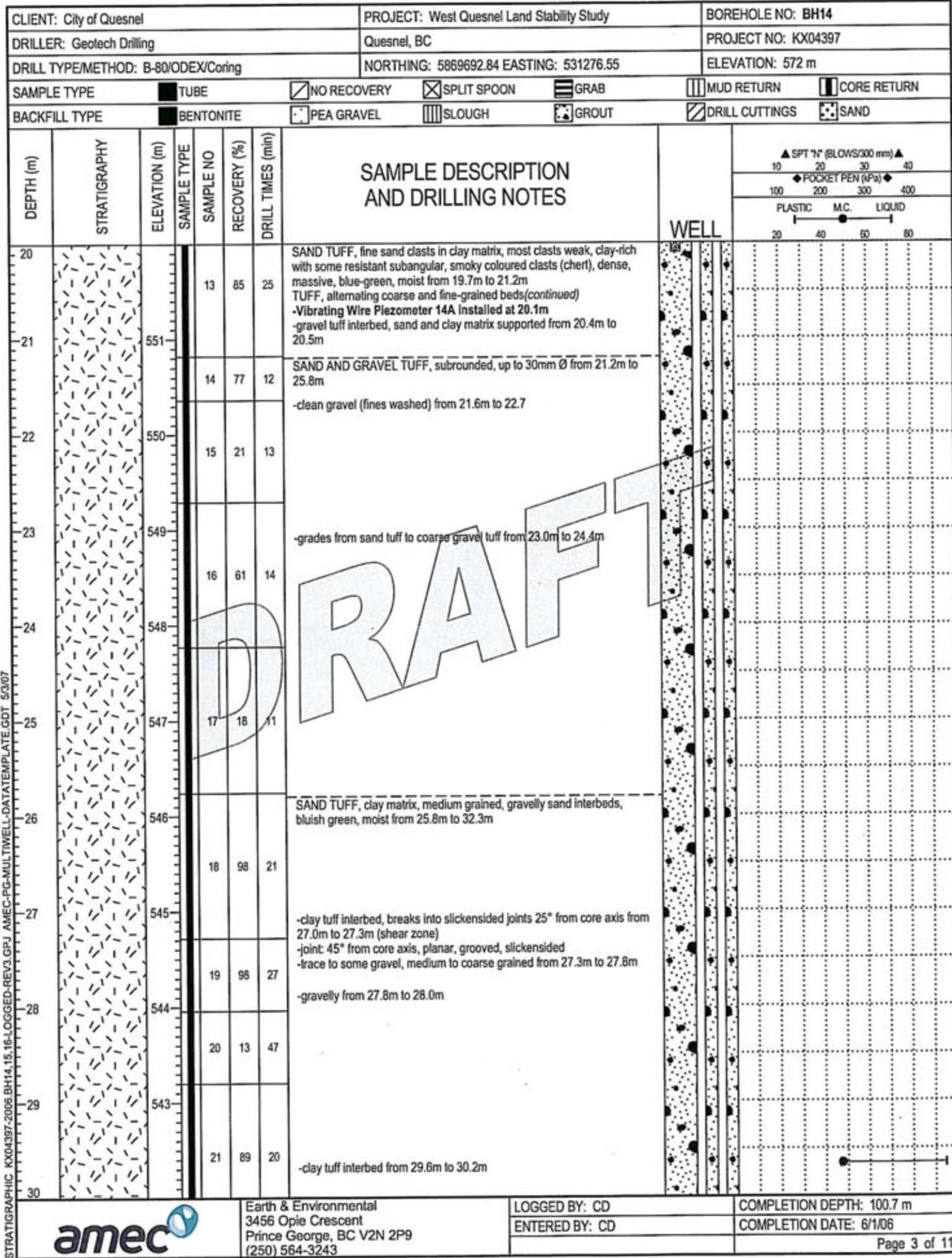


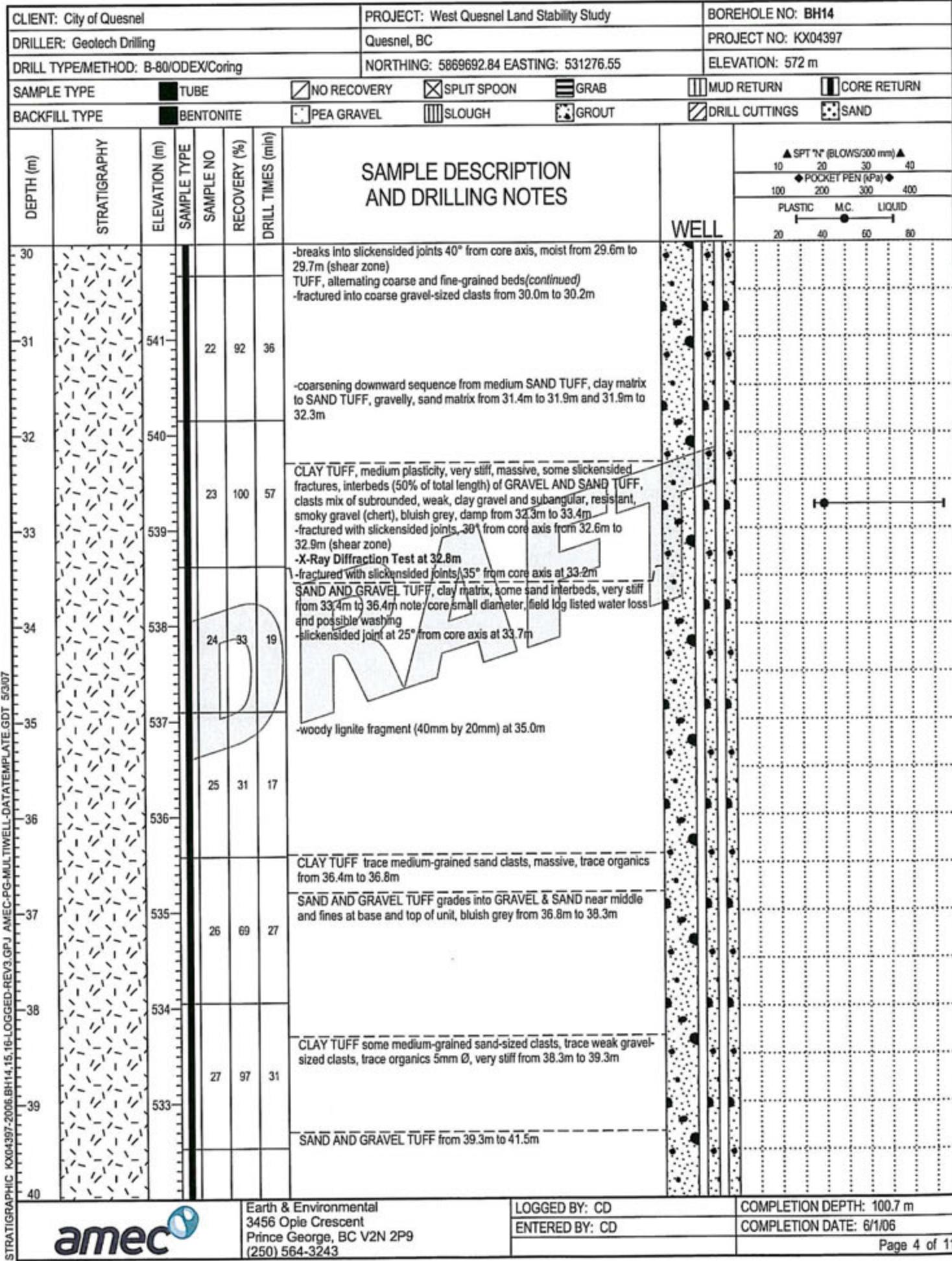
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

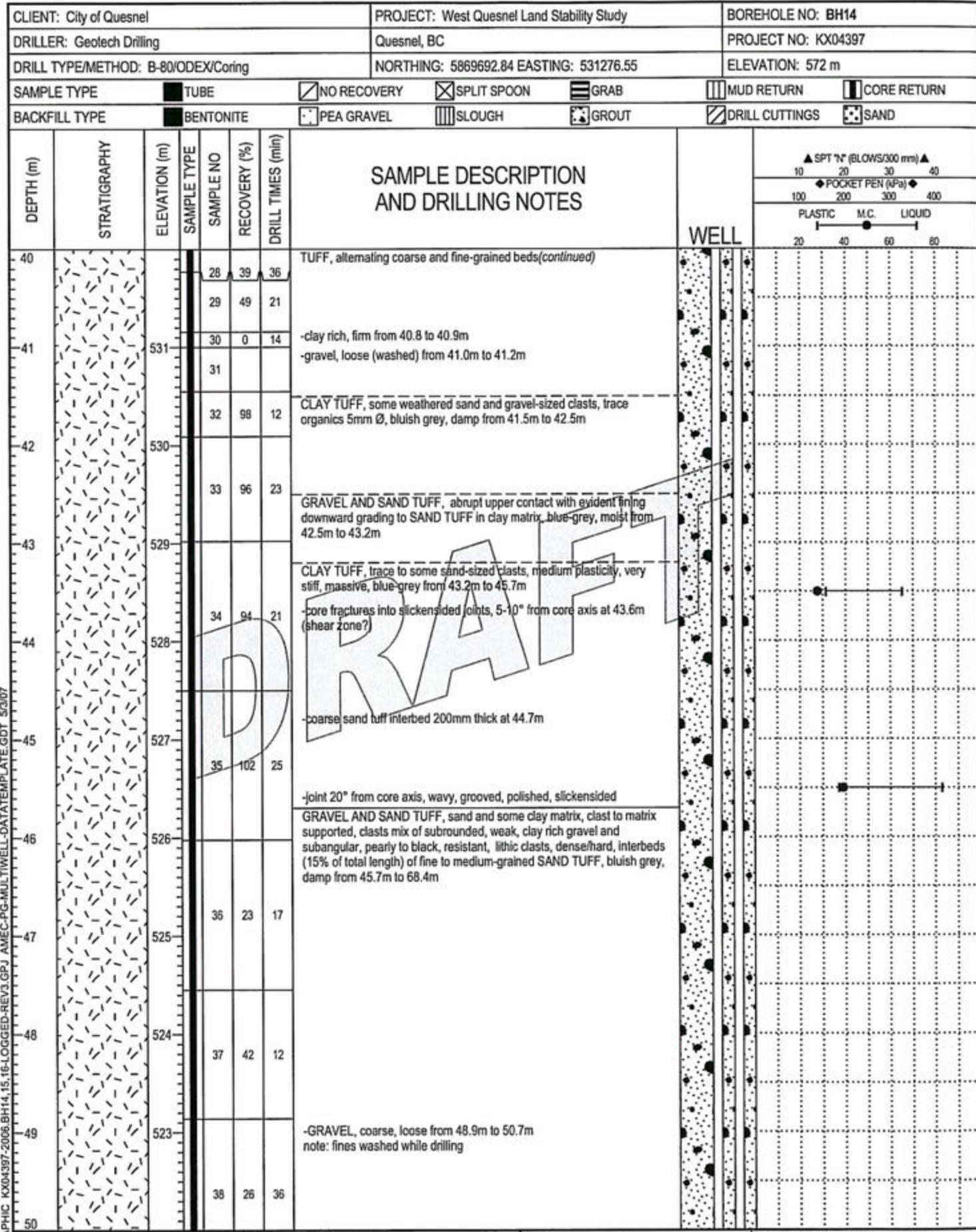
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COMPLETION DEPTH: 100.7 m
COMPLETION DATE: 6/1/06









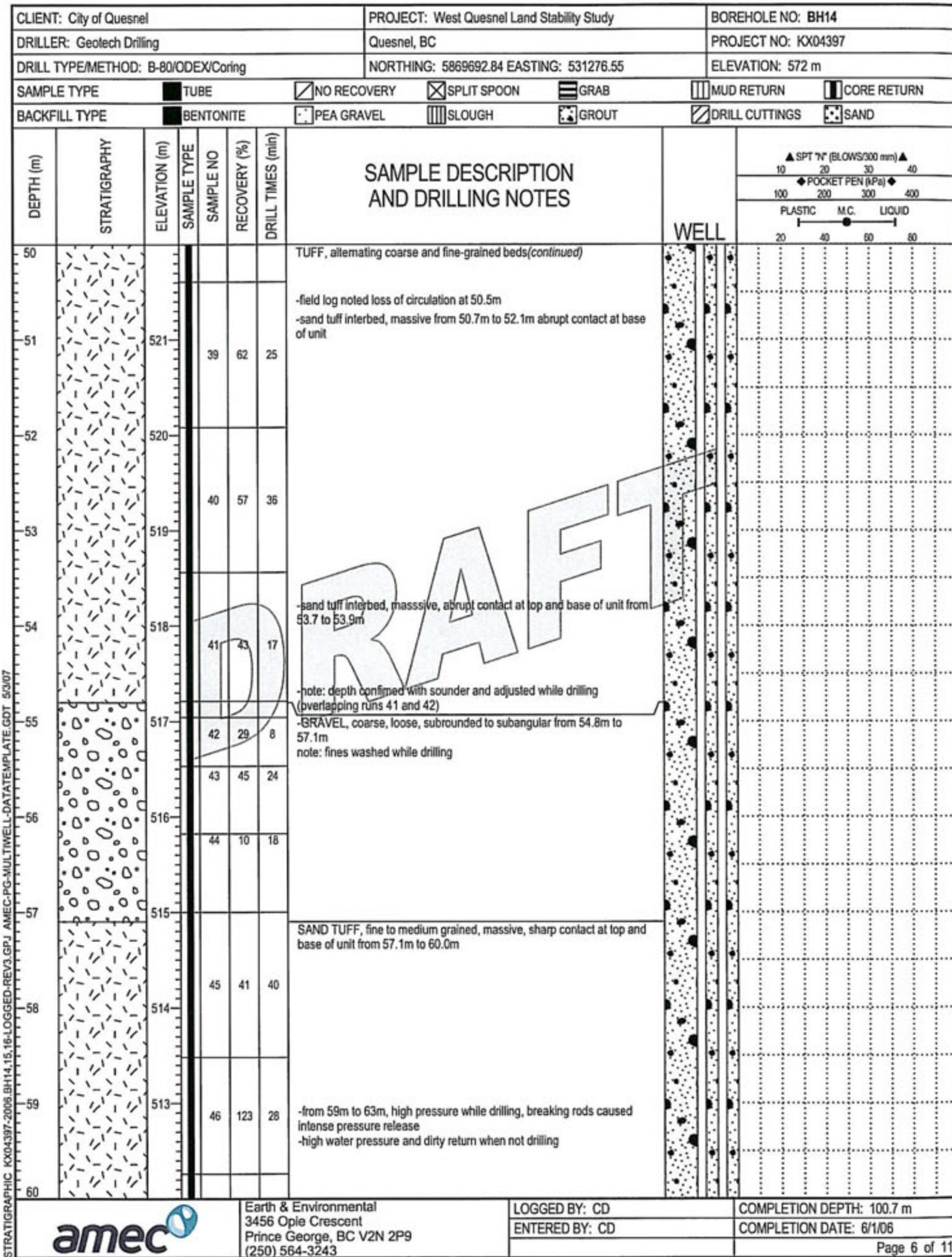
STRATIGRAPHIC LOGGED-REV3.GPJ AMEC/PG-MULTIWELL-DATA TEMPLATE GDT 5/2007

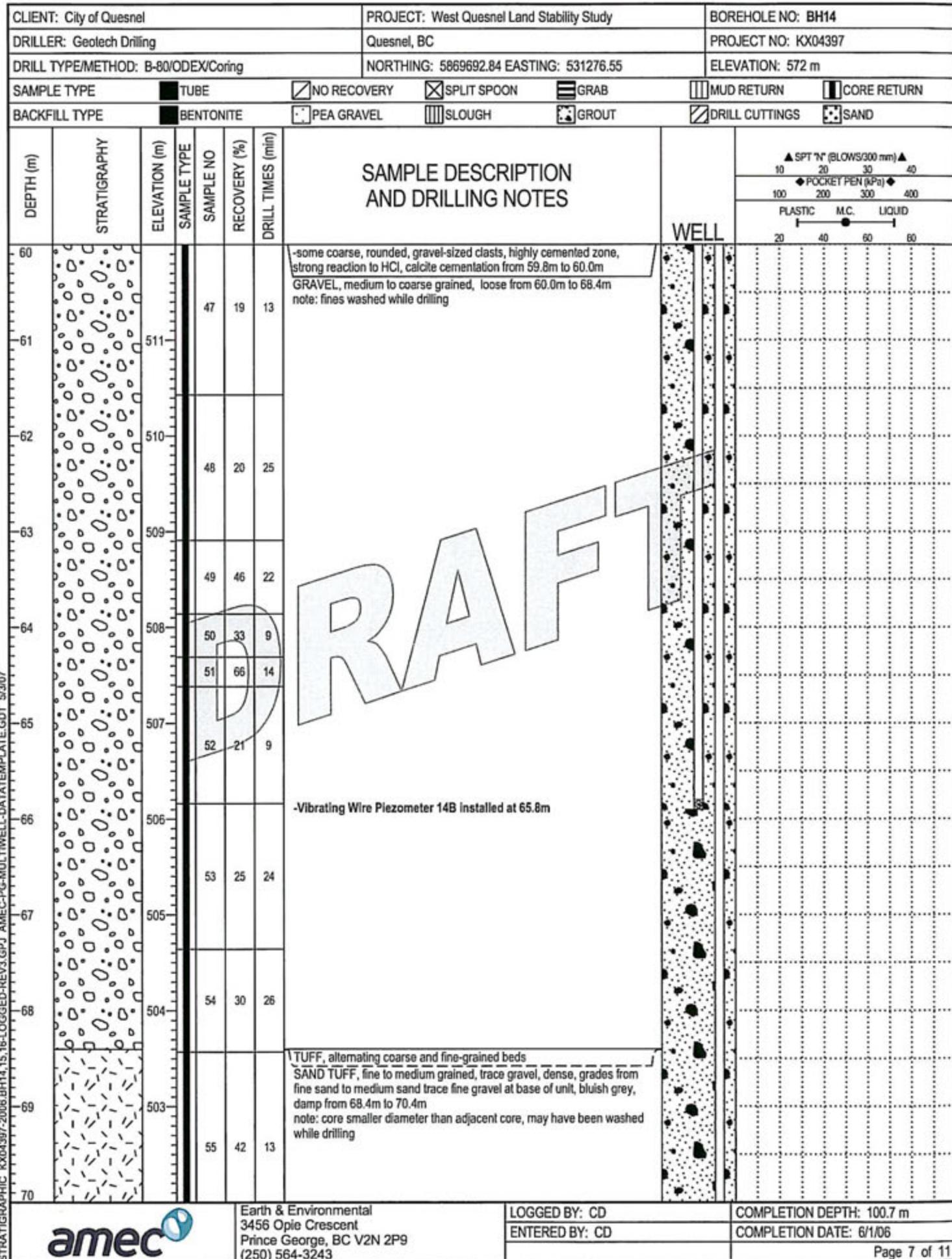


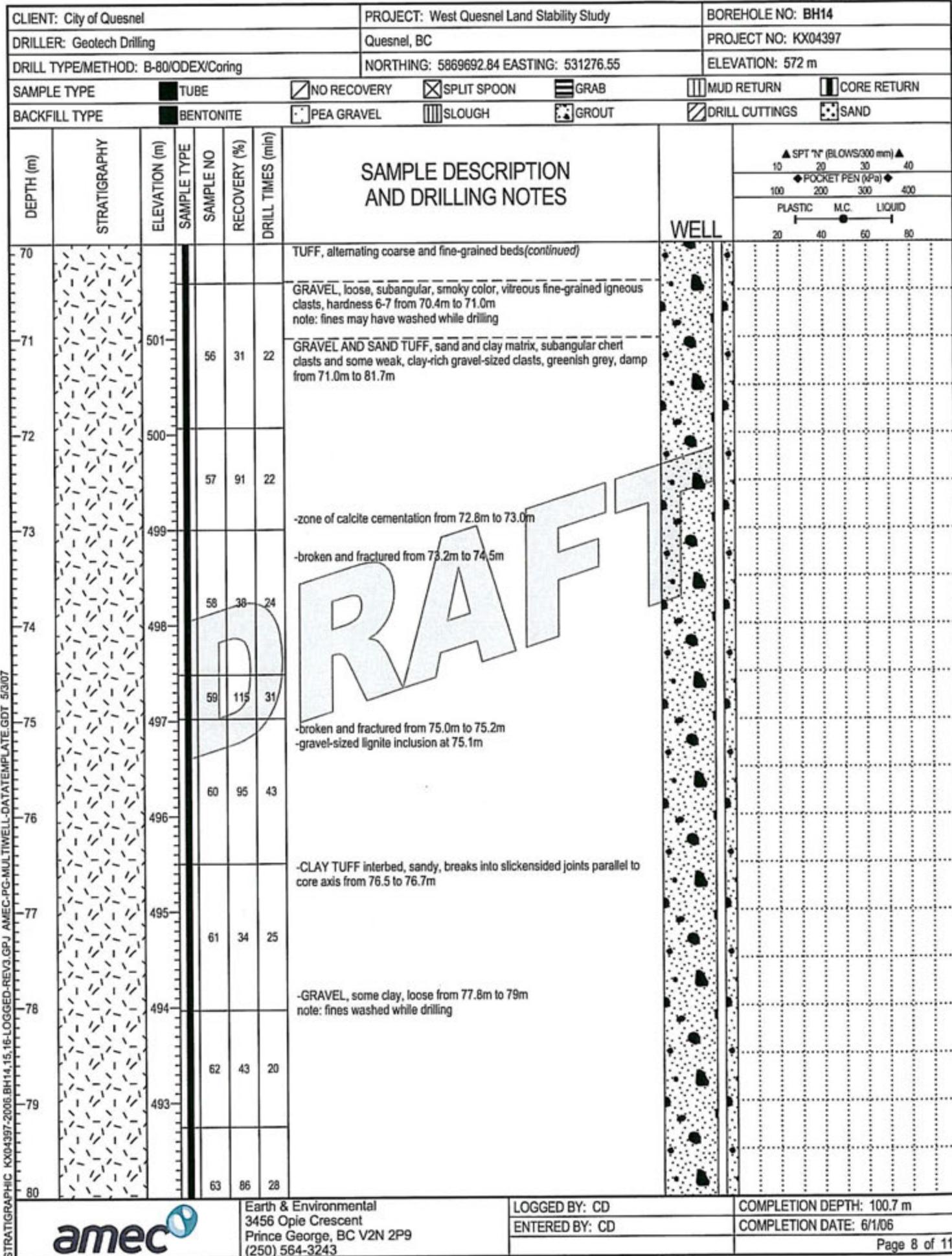
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

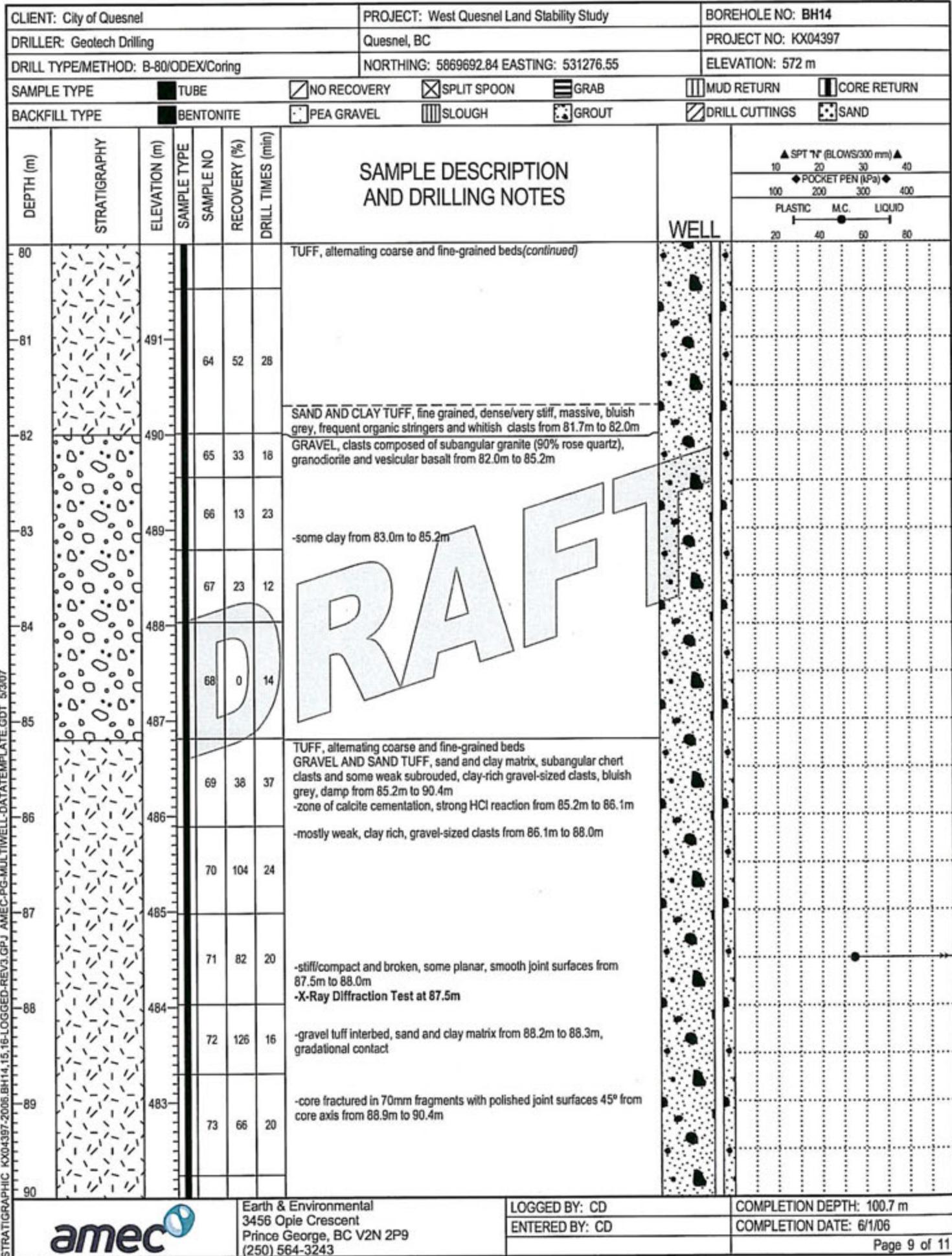
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COMPLETION DEPTH: 100.7 m
COMPLETION DATE: 6/106





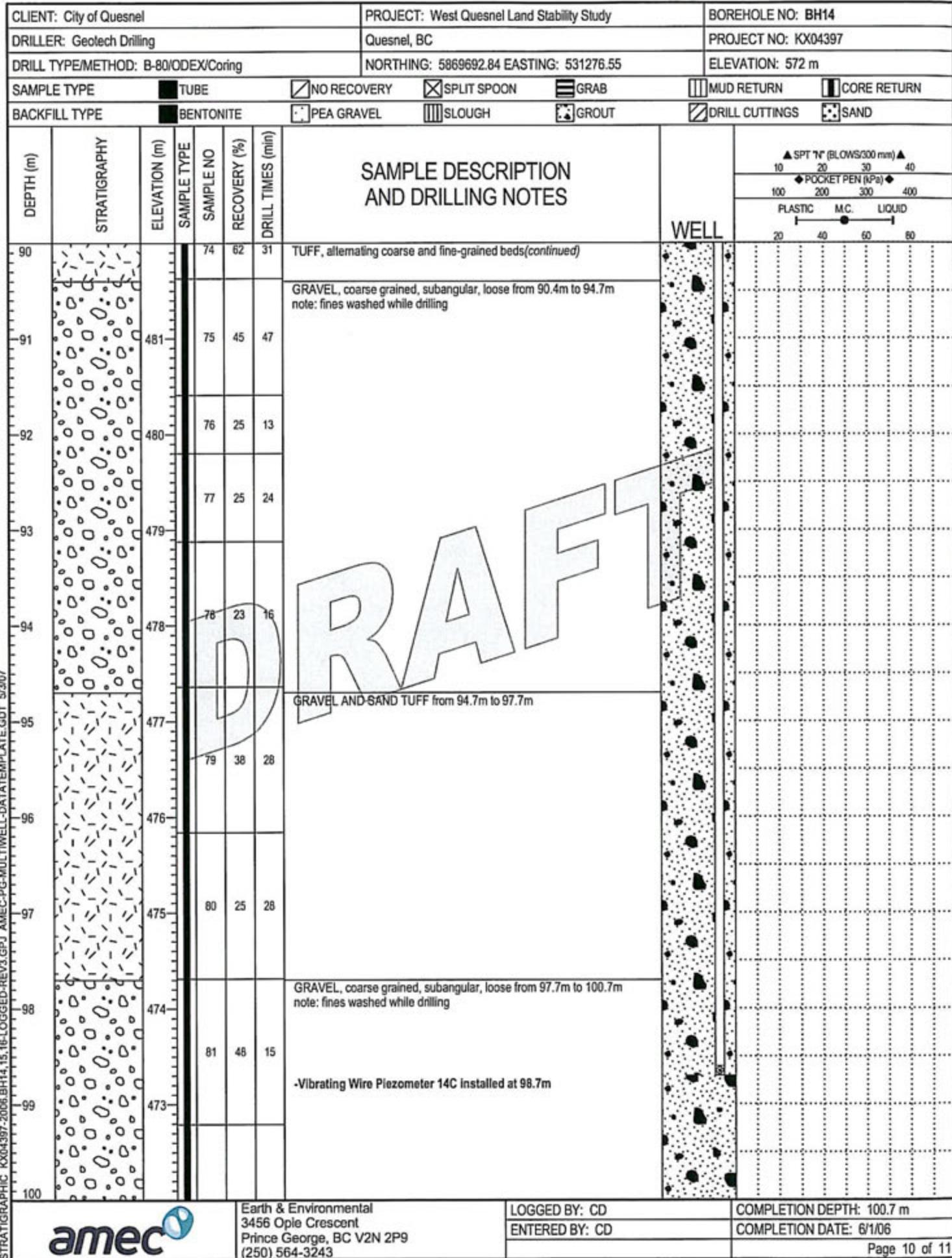


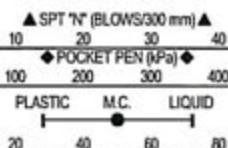


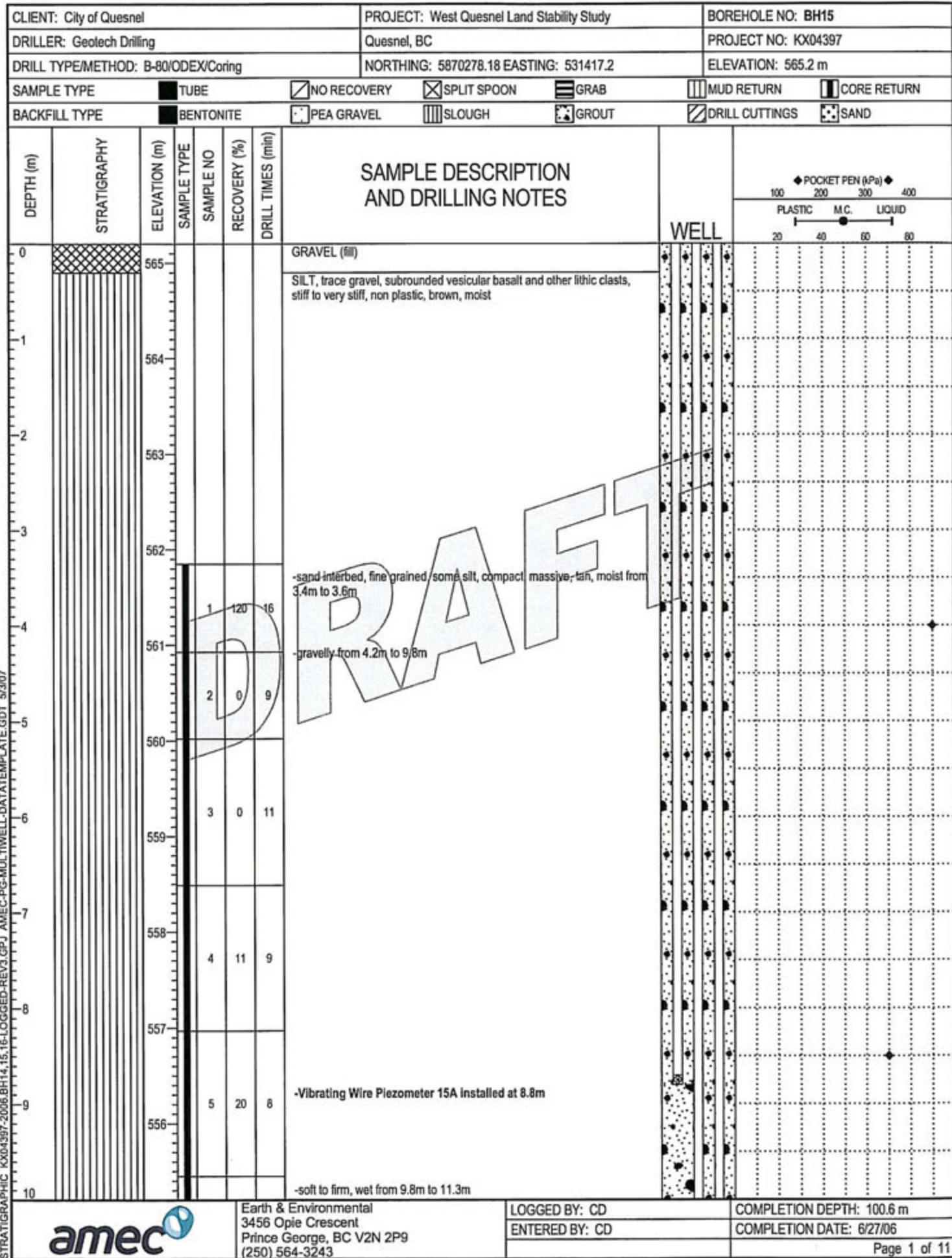
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: CD
ENTERED BY: CD

COMPLETION DEPTH: 100.7 m
COMPLETION DATE: 6/1/06



CLIENT: City of Quesnel				PROJECT: West Quesnel Land Stability Study				BOREHOLE NO: BH14				
DRILLER: Geotech Drilling				Quesnel, BC				PROJECT NO: KX04397				
DRILL TYPE/METHOD: B-80/ODEX/Coring				NORTHING: 5869692.84 EASTING: 531276.55				ELEVATION: 572 m				
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN						
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input checked="" type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND						
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES					
100				82	16	45	GRAVEL, coarse grained, subangular, loose from 97.7m to 100.7m note: fines washed while drilling(continued)					
101		471					End of Borehole at 100.7m Drilling method did not allow observation of water level					
102		470										
103		469										
104		468										
105		467										
106		466										
107		465										
108		464										
109		463										
110												
DRAFT												
WELL												
												
STRATIGRAPHIC LOGGED-REV3.GPJ AMEC-PPG-MULTIWELL-DATATEMPLATE.GDT 5/2/2007												
amec			Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243				LOGGED BY: CD	COMPLETION DEPTH: 100.7 m				
							ENTERED BY: CD	COMPLETION DATE: 6/1/06				
								Page 11 of 11				

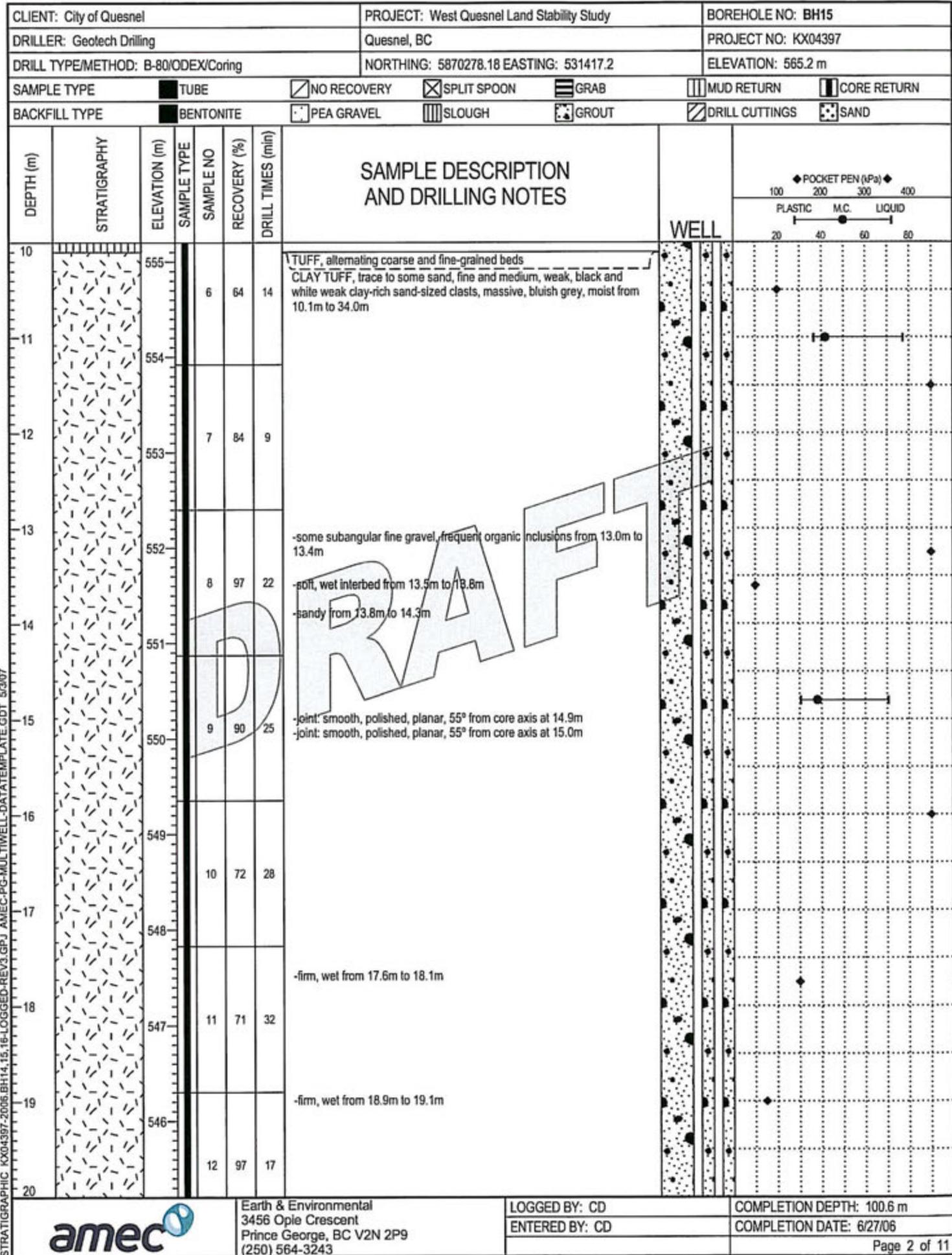


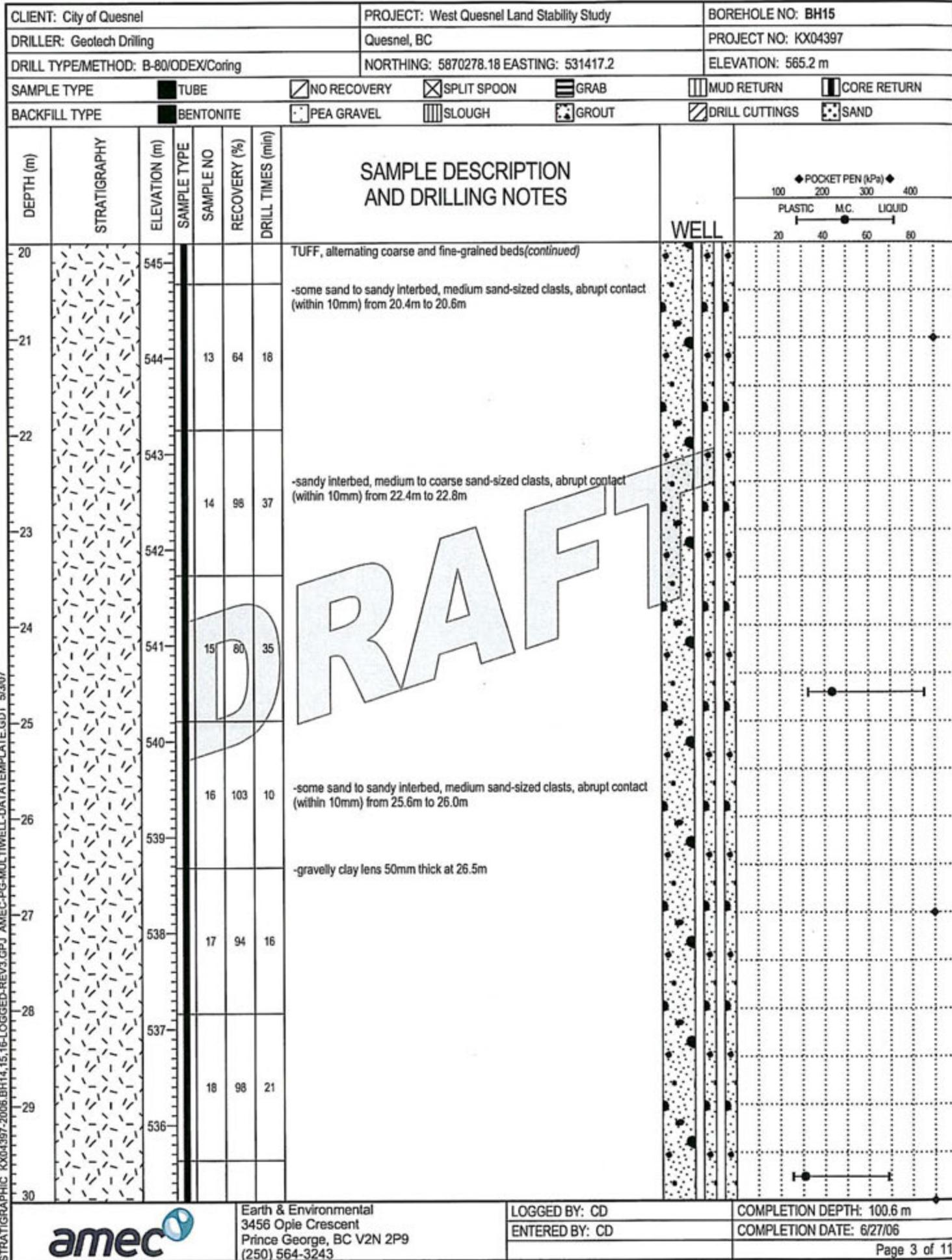
Earth & Environmental
3456 Opie Crescent
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(250) 564-3243

LOGGED BY: CD
ENTERED BY: CD

COMPLETION DEPTH: 100.6 m
COMPLETION DATE: 6/27/06

Page 1 of 11





STRATIGRAPHIC LOGGED-REV3 GPJ AMEC/PG-MULTIWELL-DATATEMPLATE.GDT 5/3/07

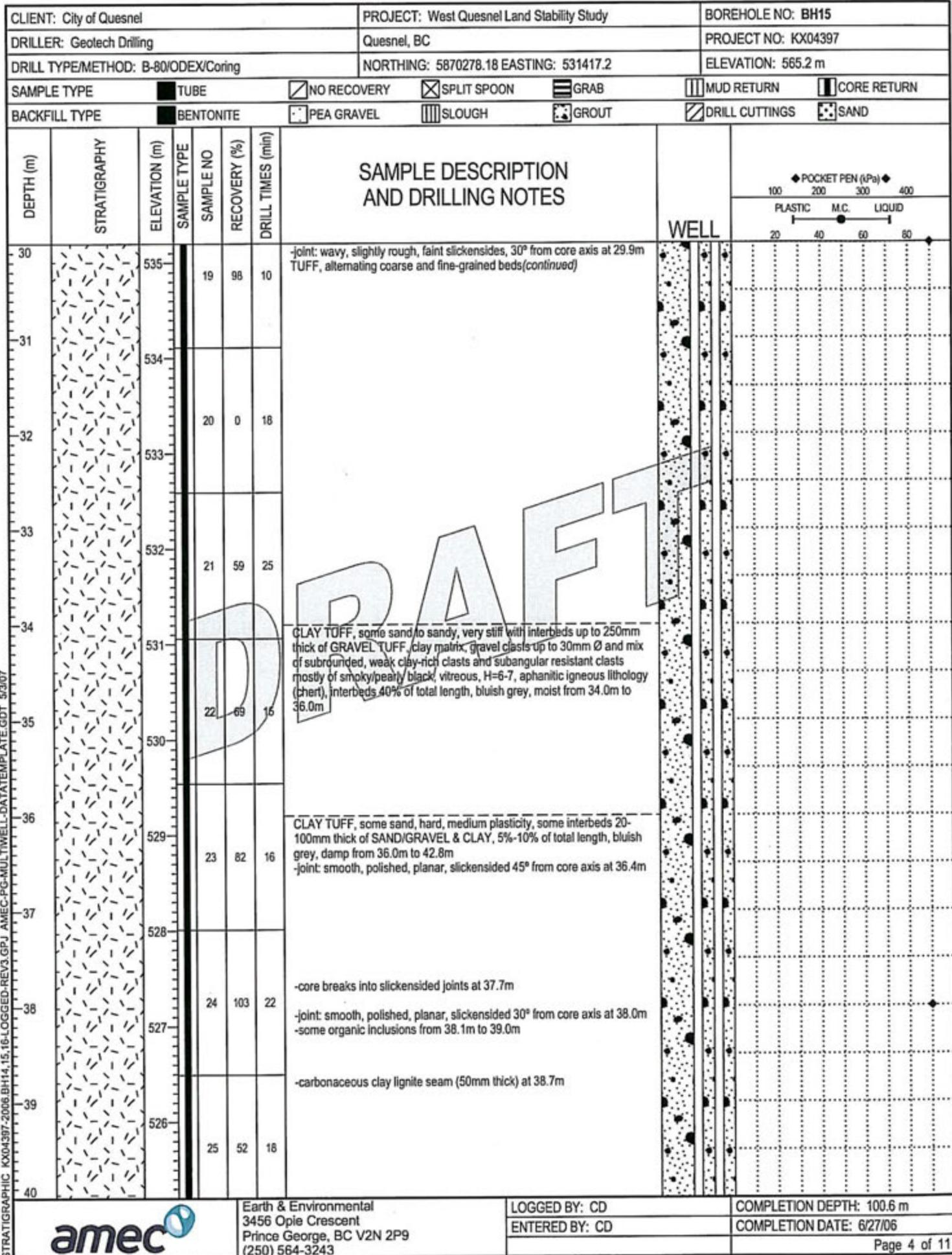


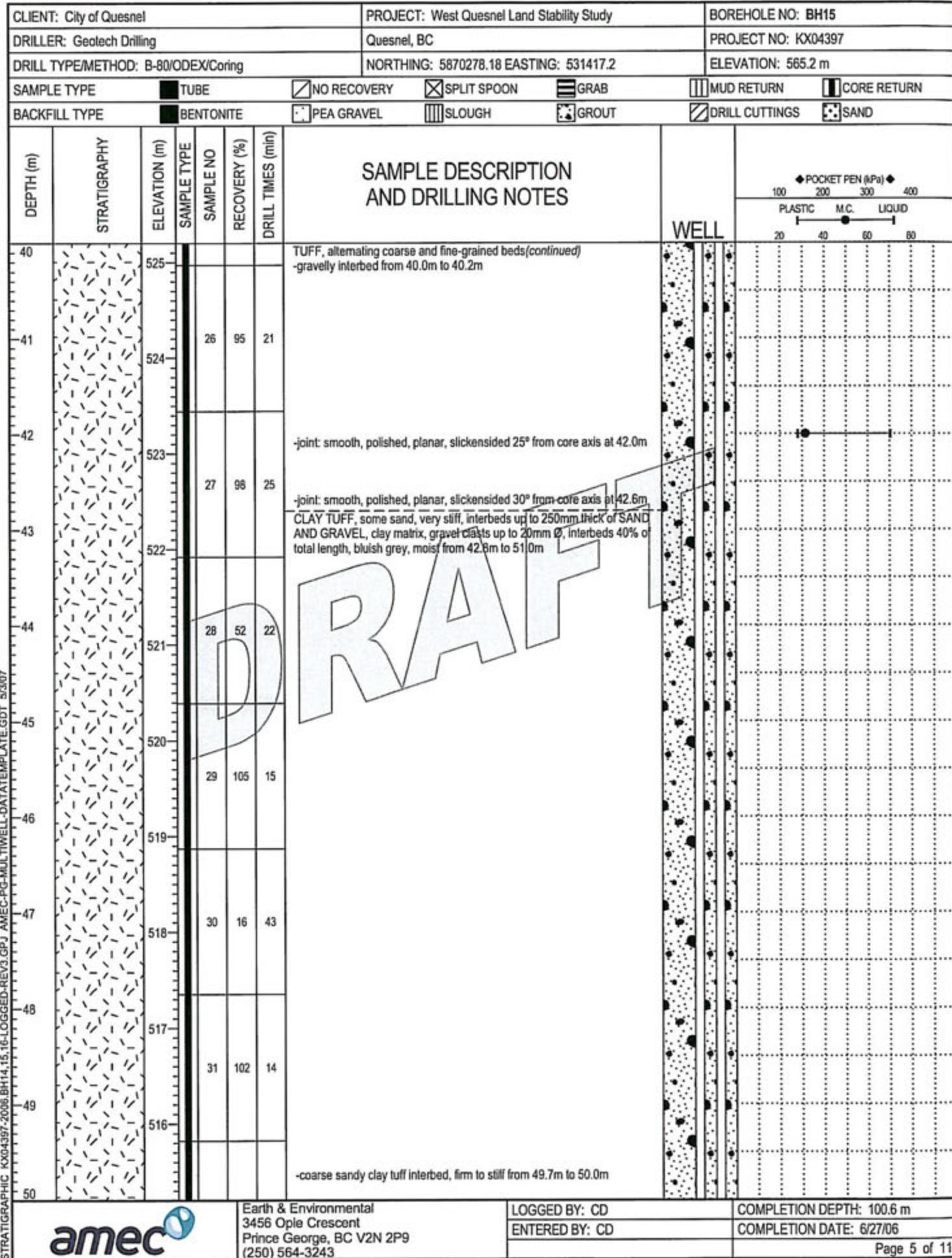
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

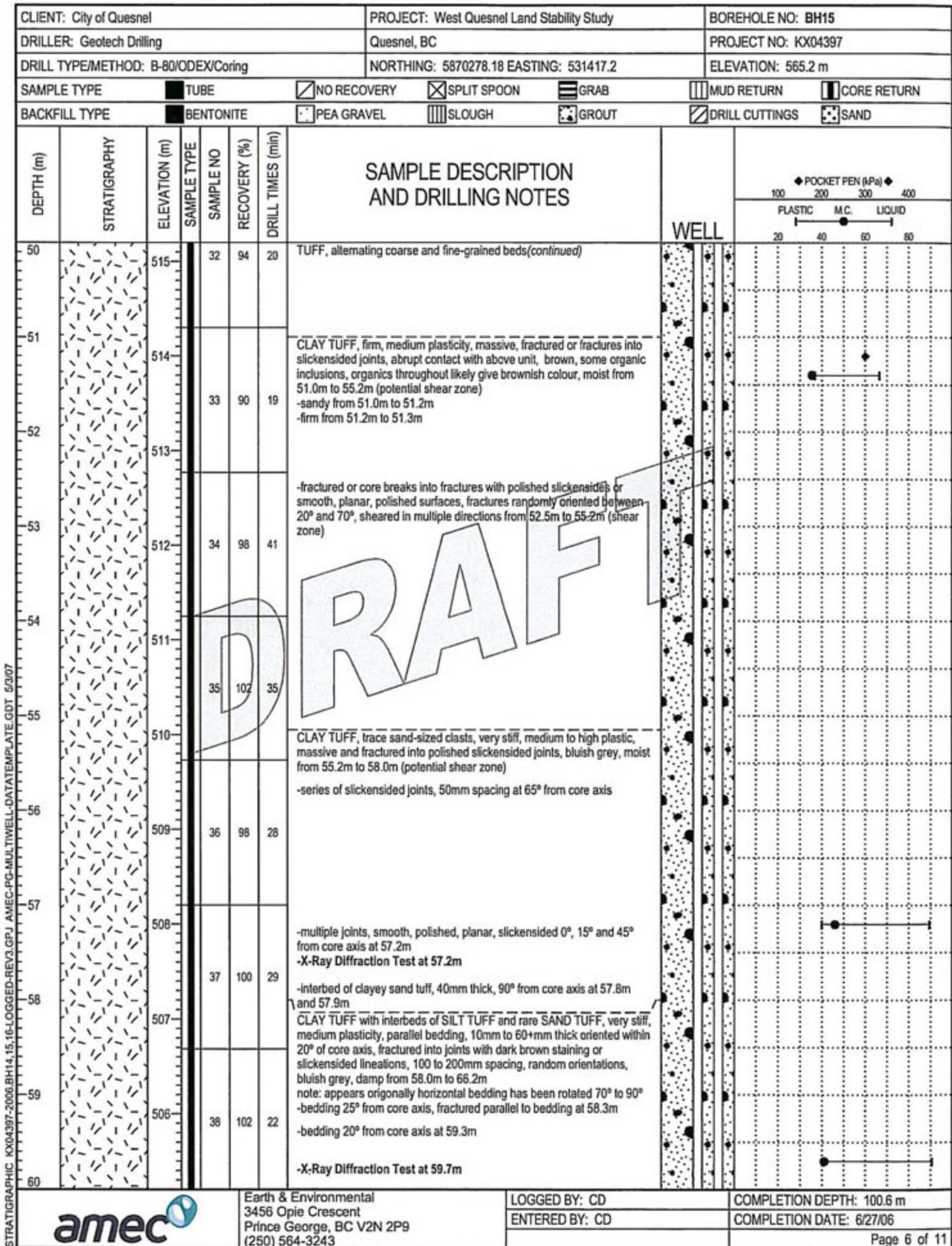
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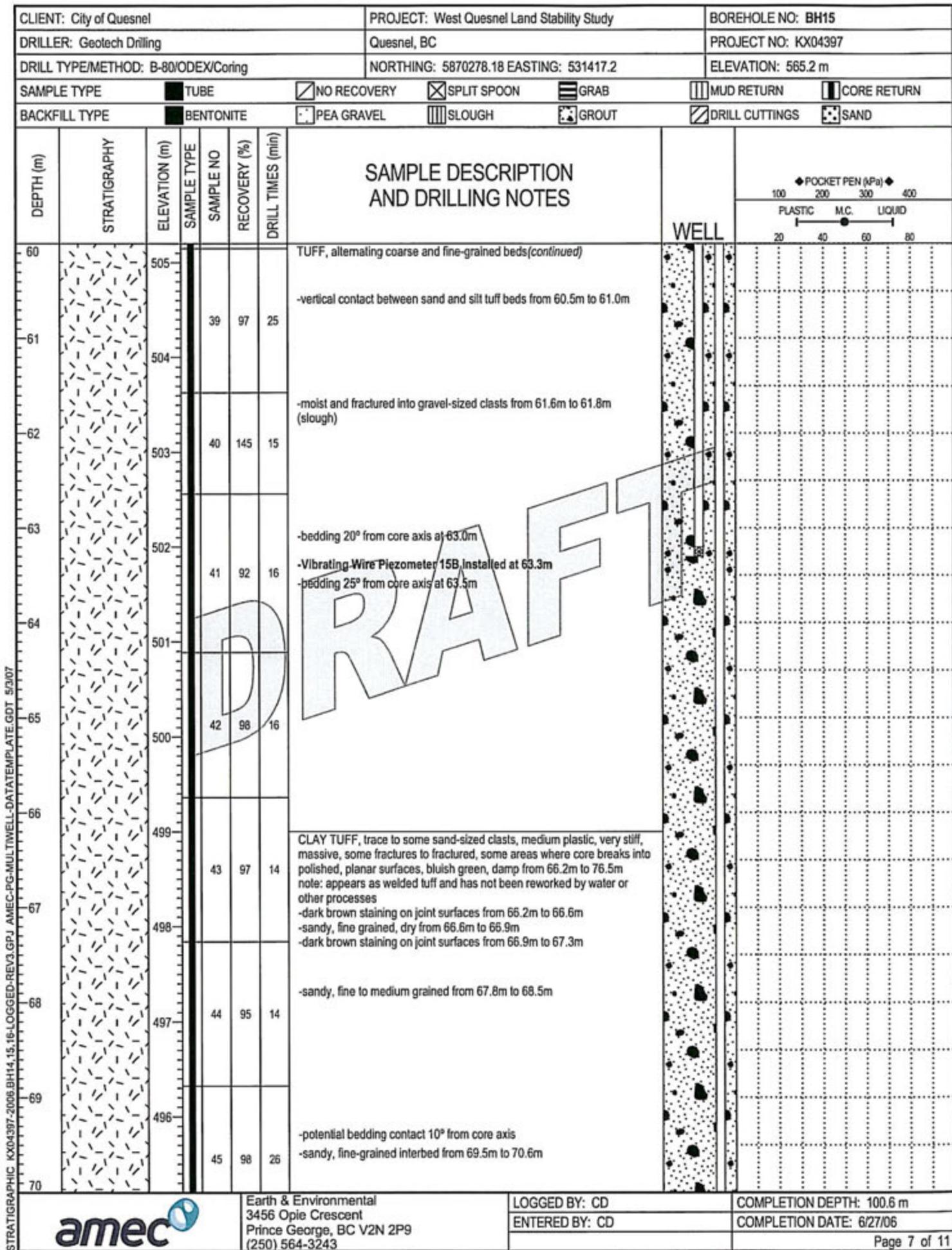
COMPLETION DEPTH: 100.6 m
COMPLETION DATE: 6/27/06

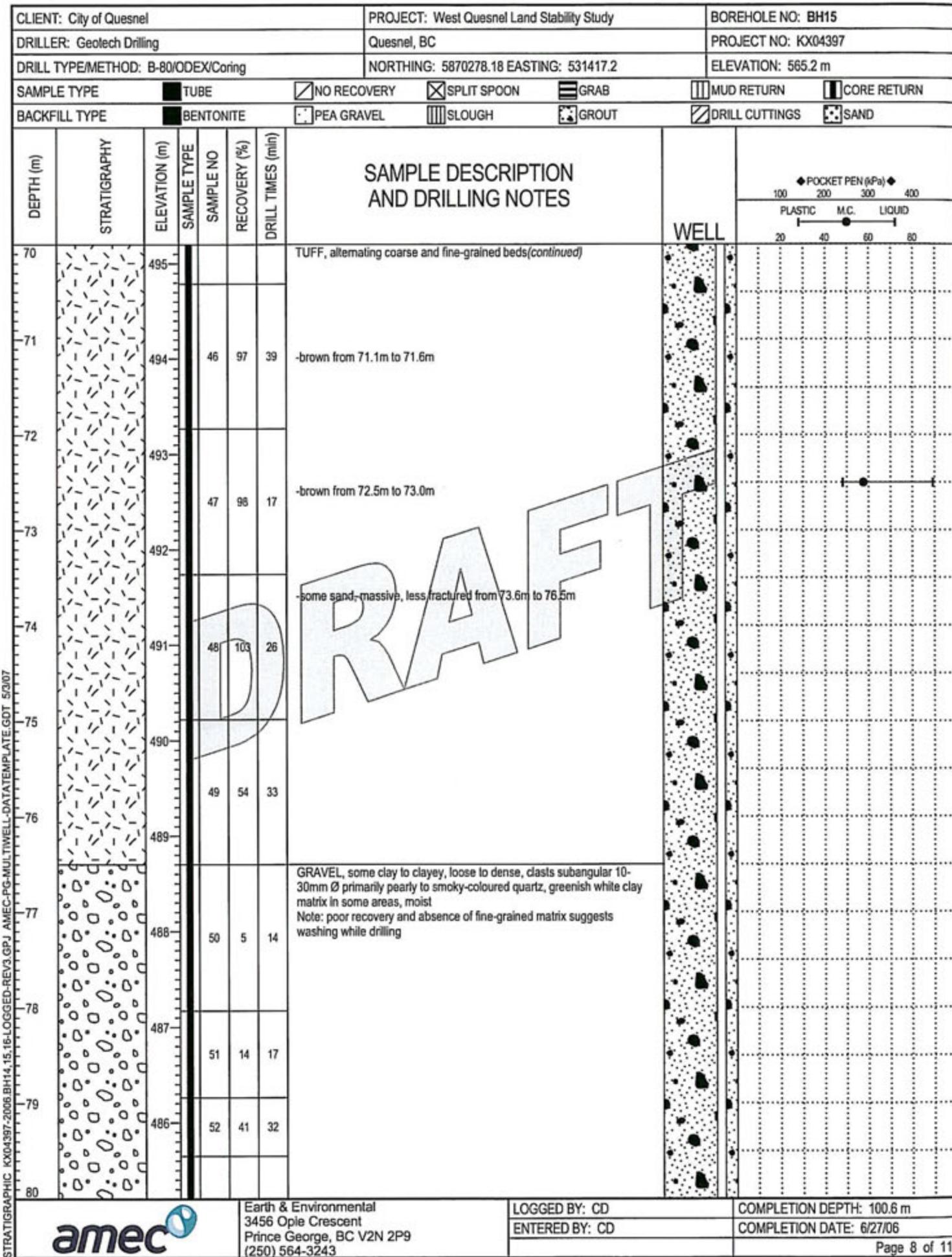
Page 3 of 11

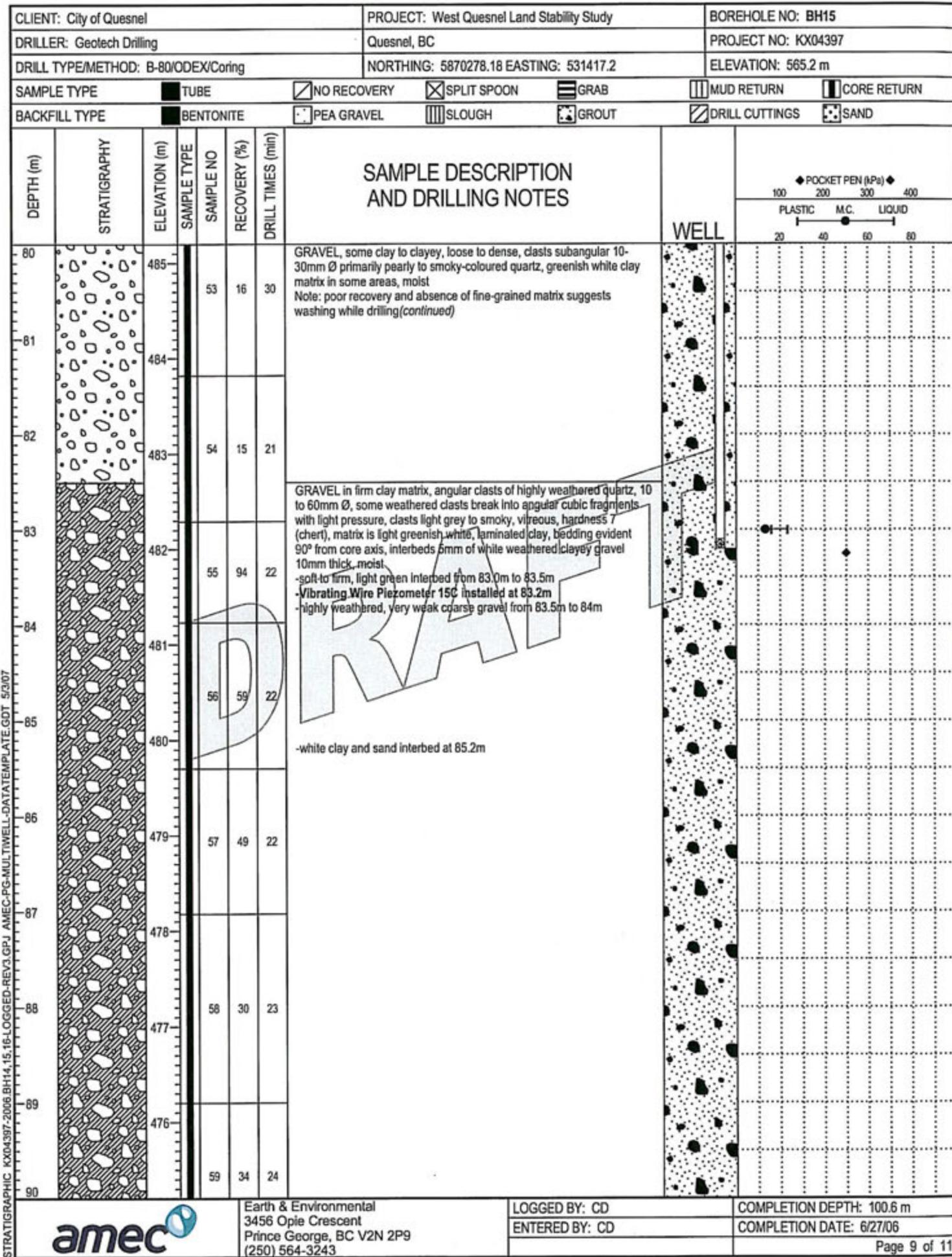


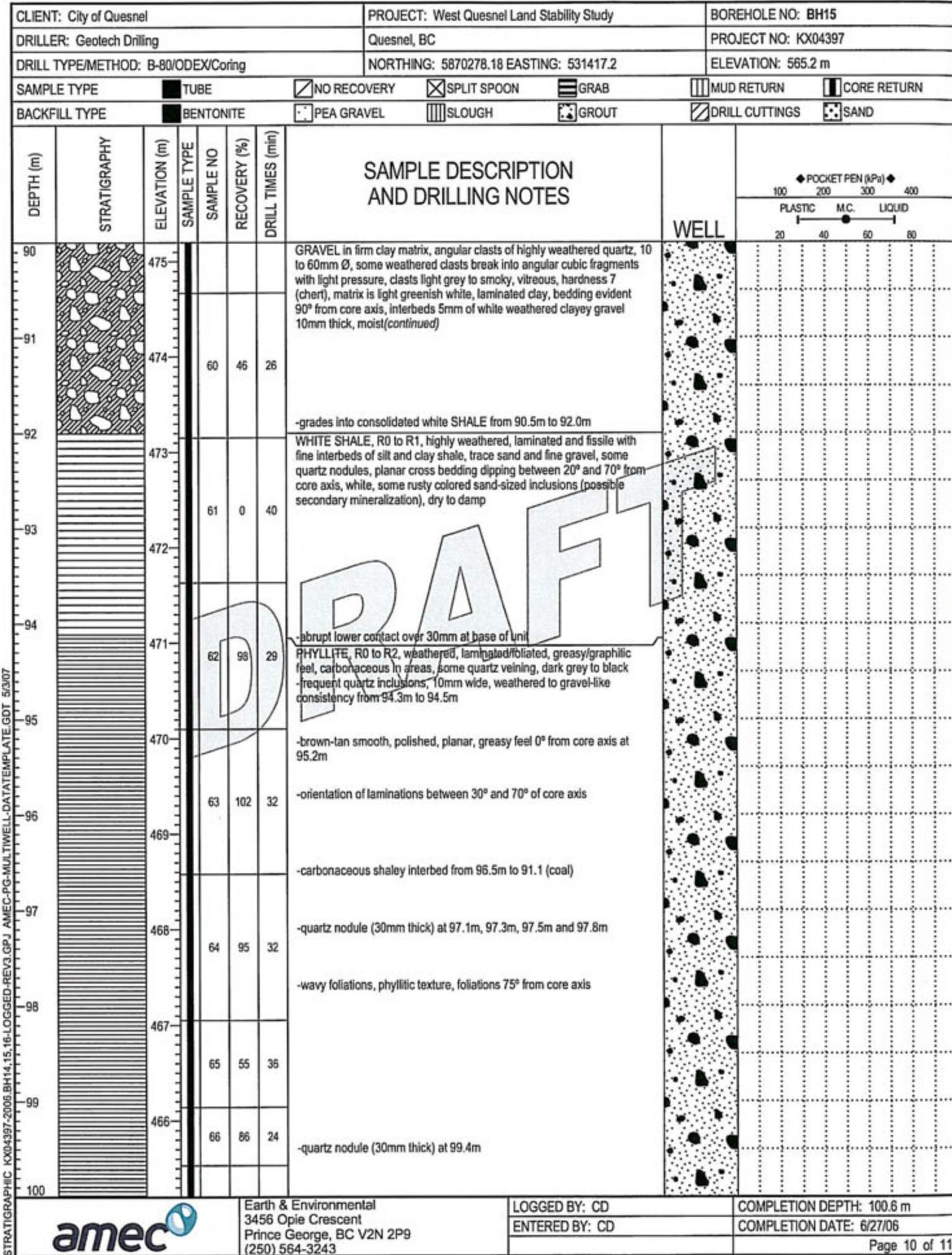




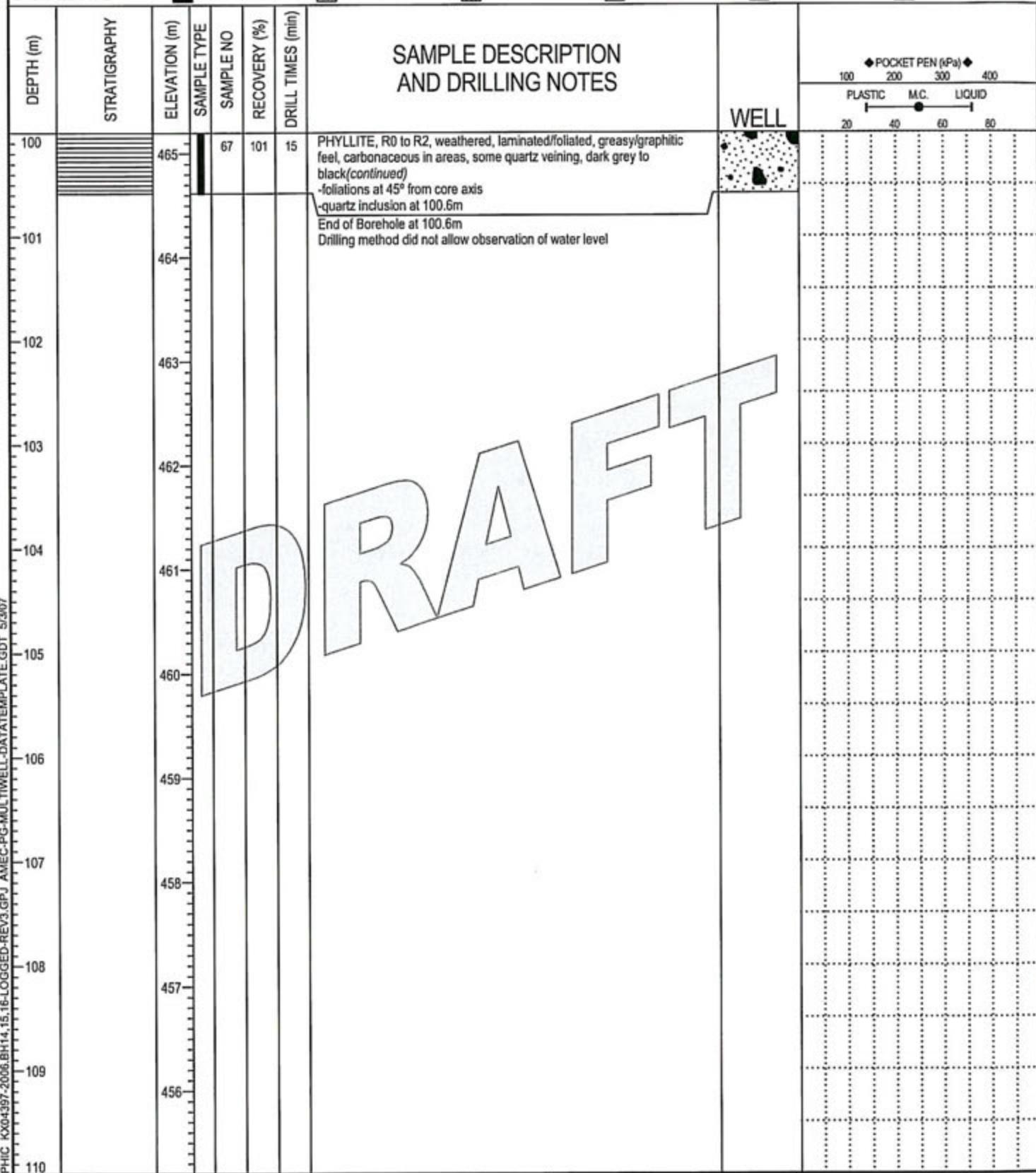








CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH15
DRILLER: Geotech Drilling	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: B-80/ODEX/Coring	NORTHING: 5870278.18 EASTING: 531417.2	ELEVATION: 565.2 m
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND	



SIBATIGRAPHIC KX04397-2006 BH14 15-16-000000-BEV3 SPY AWE.C.PG-MHL-TWEH-DATATEMPLE-GDT 53/07



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Prince George, BC V2N 2P9
(250) 564-3243**

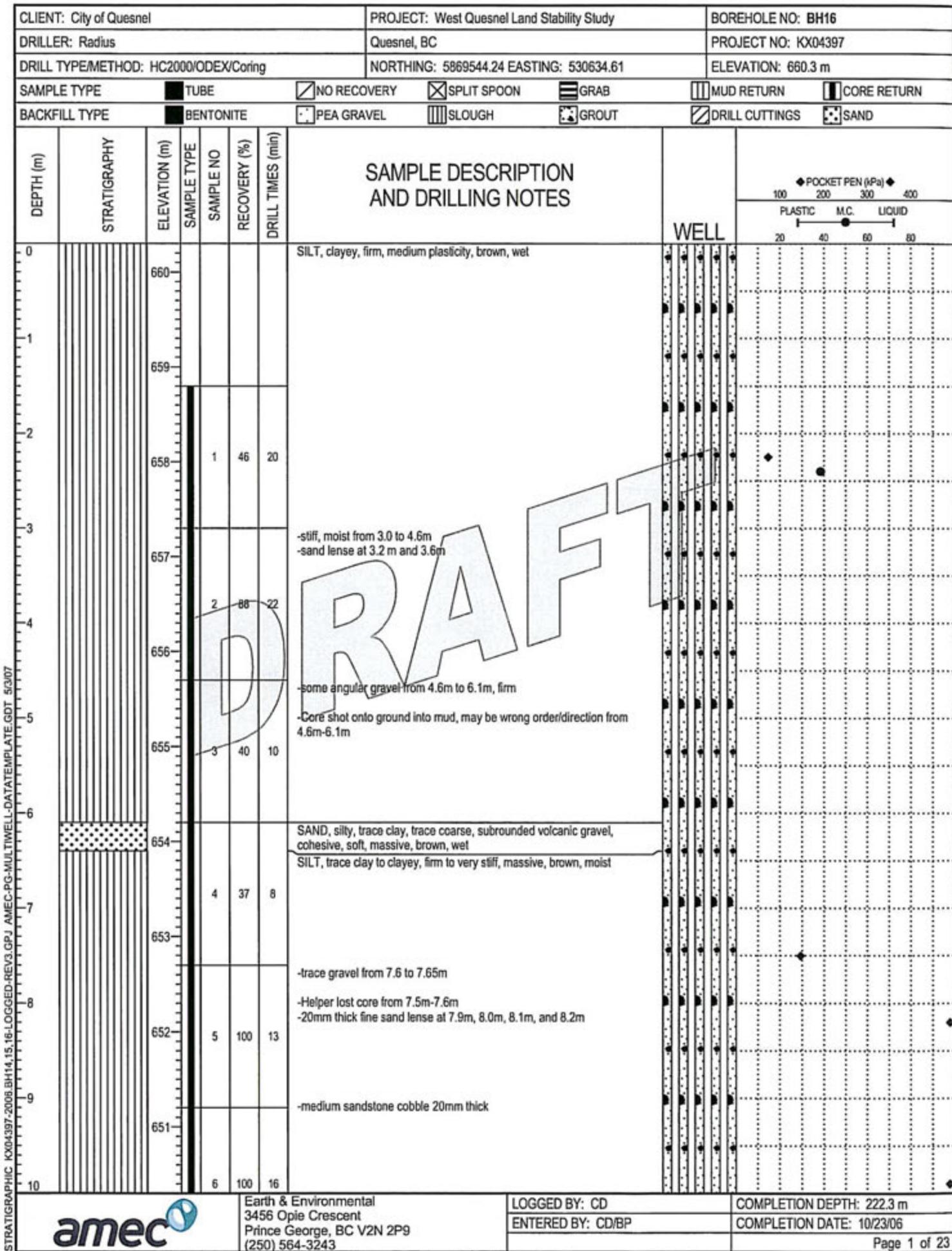
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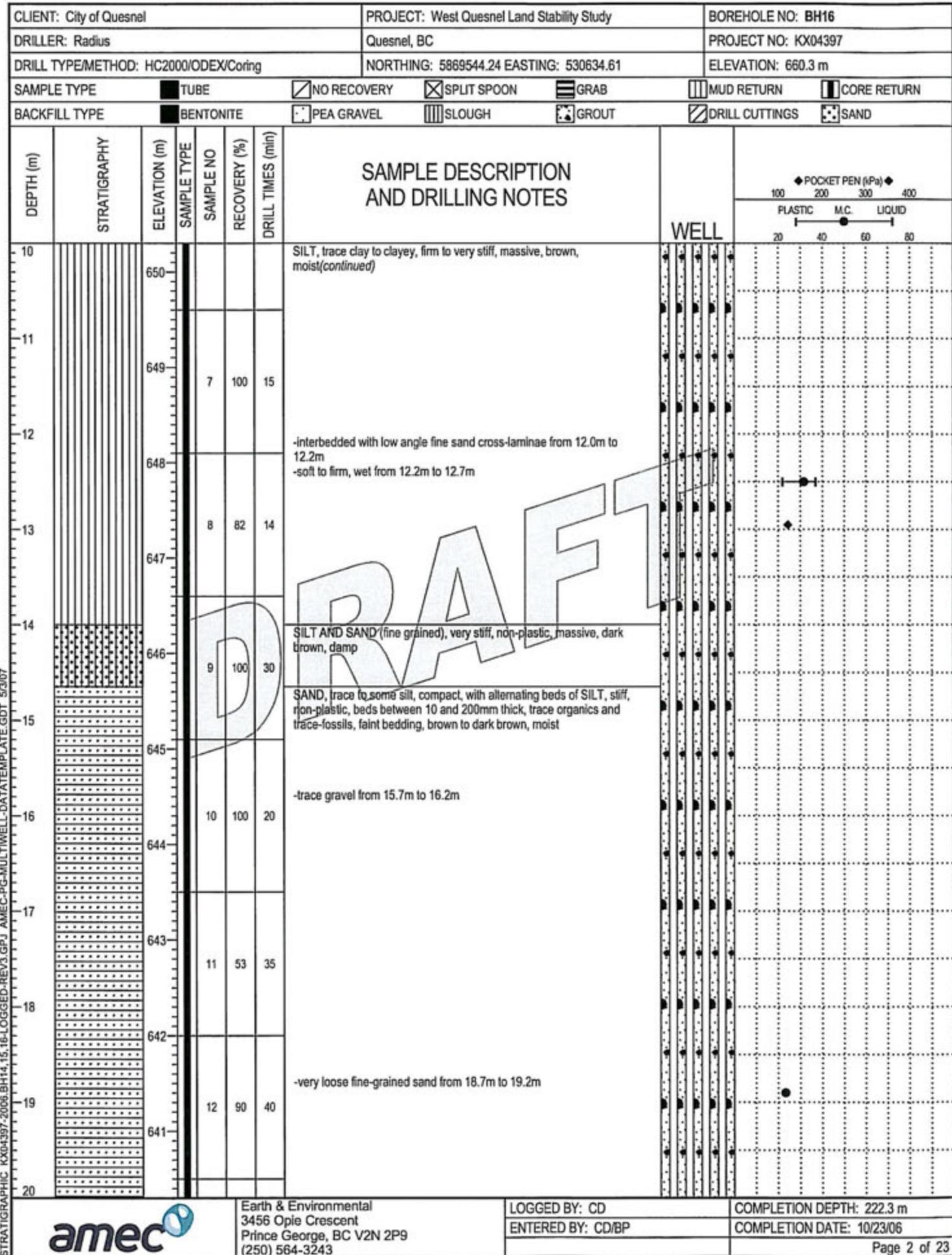
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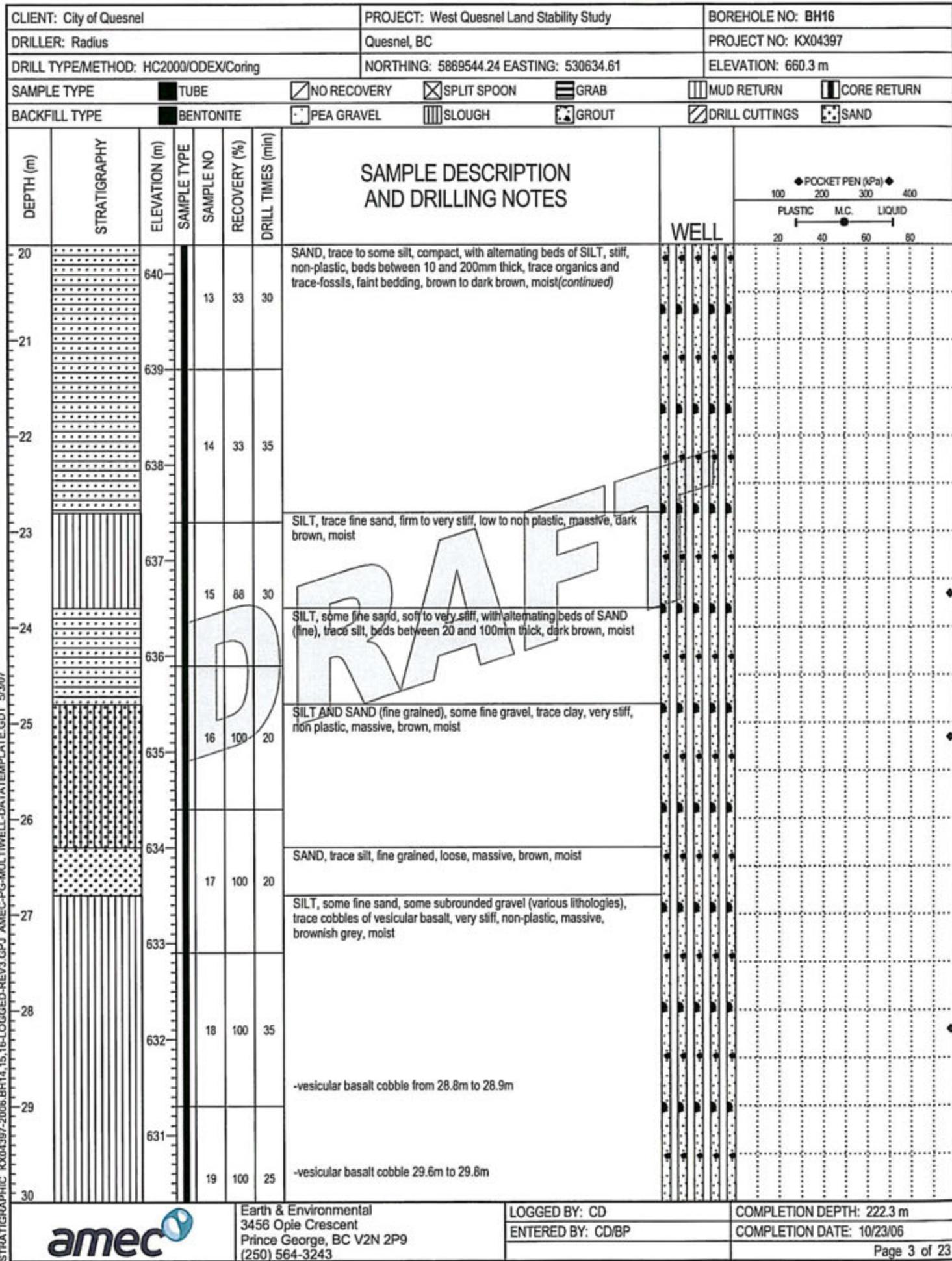
COMPLETION DEPTH: 100.6 m

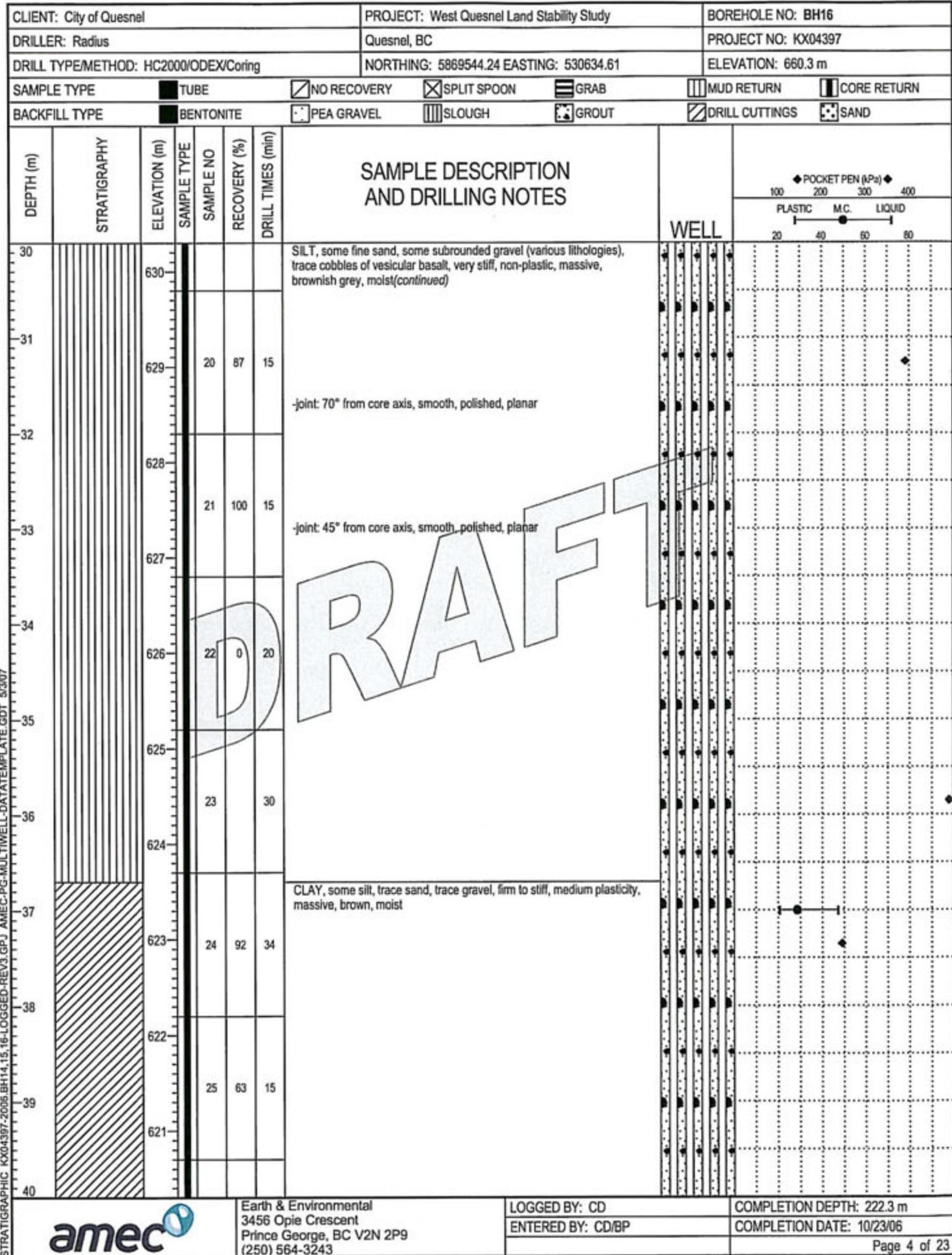
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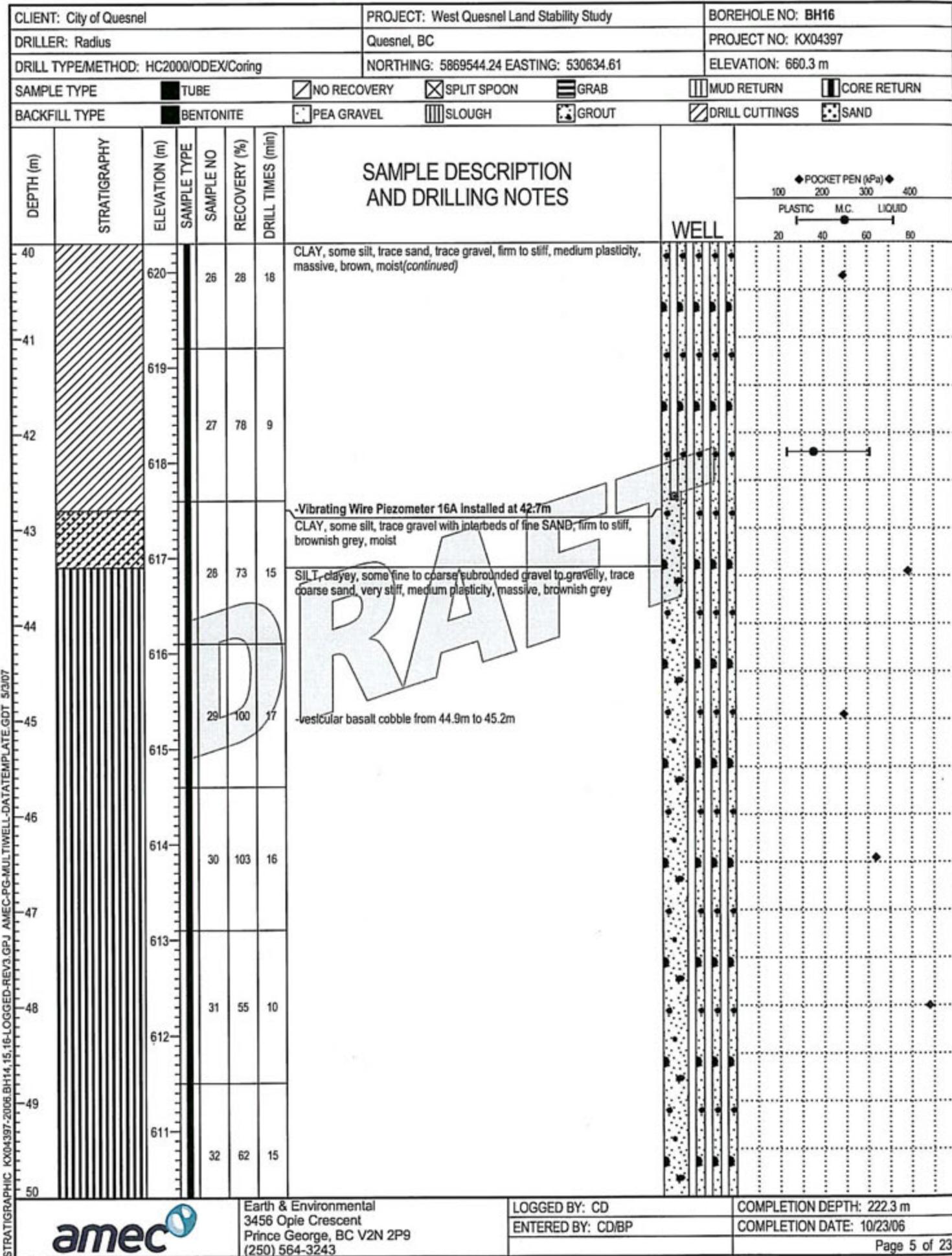
Page 11 of 11

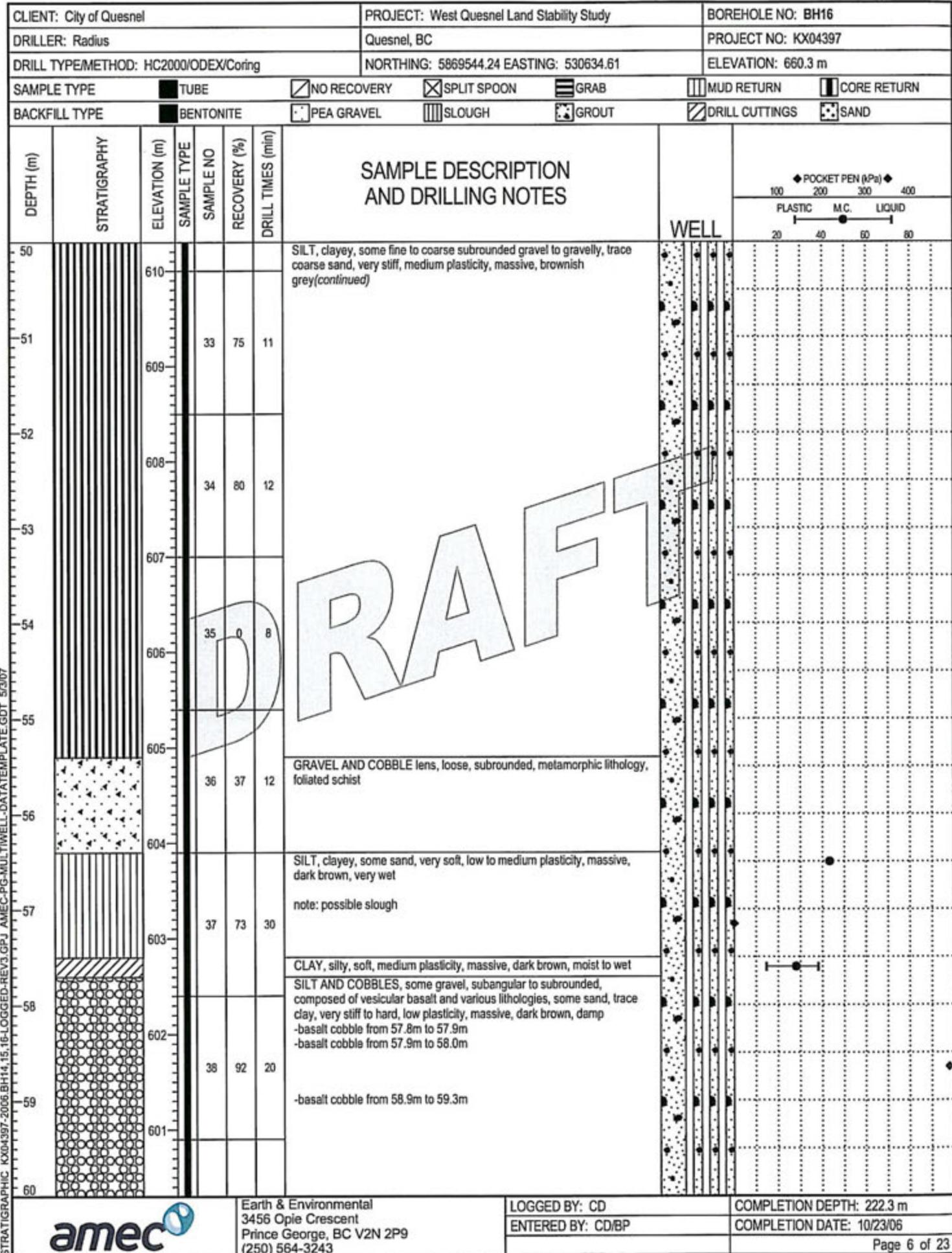


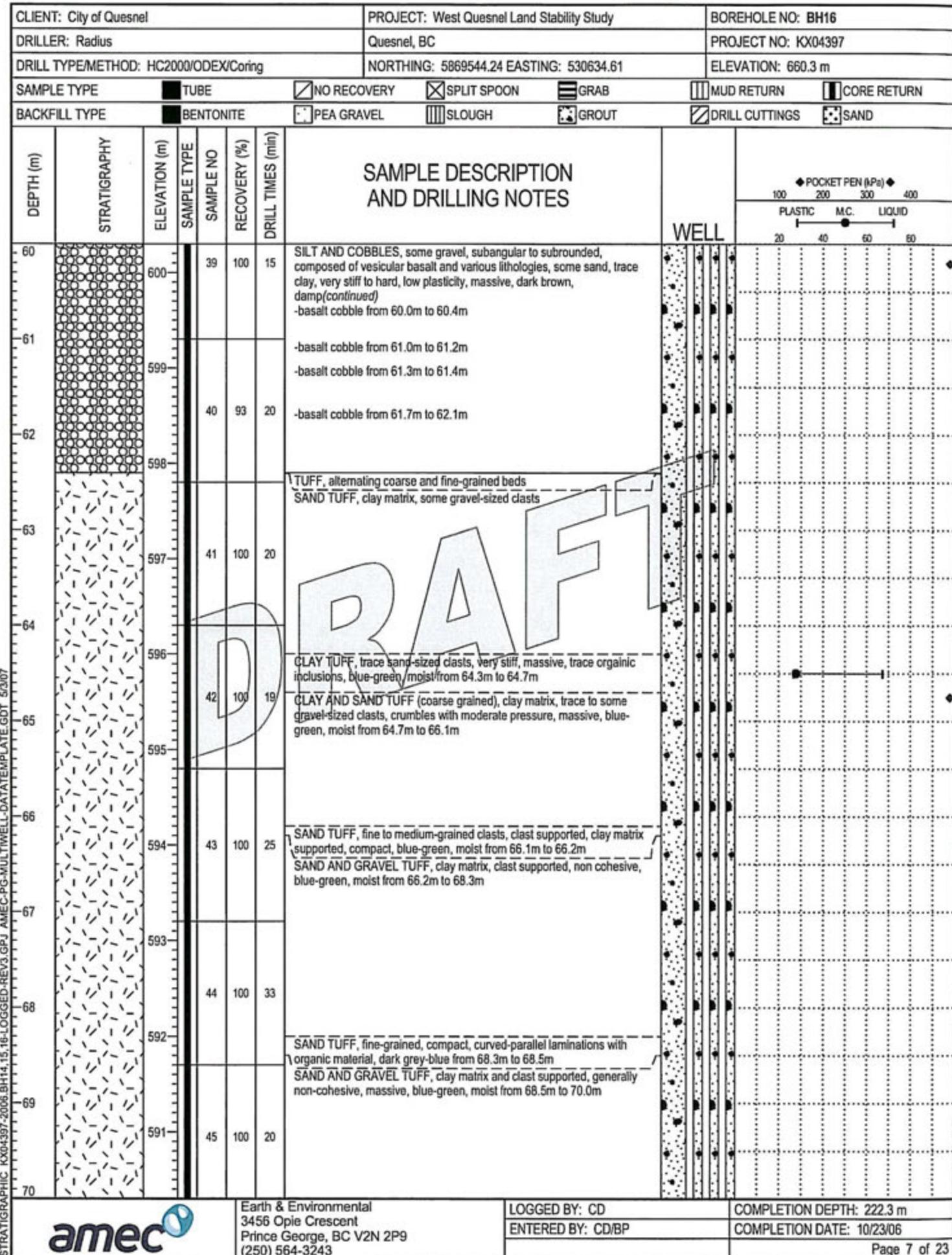


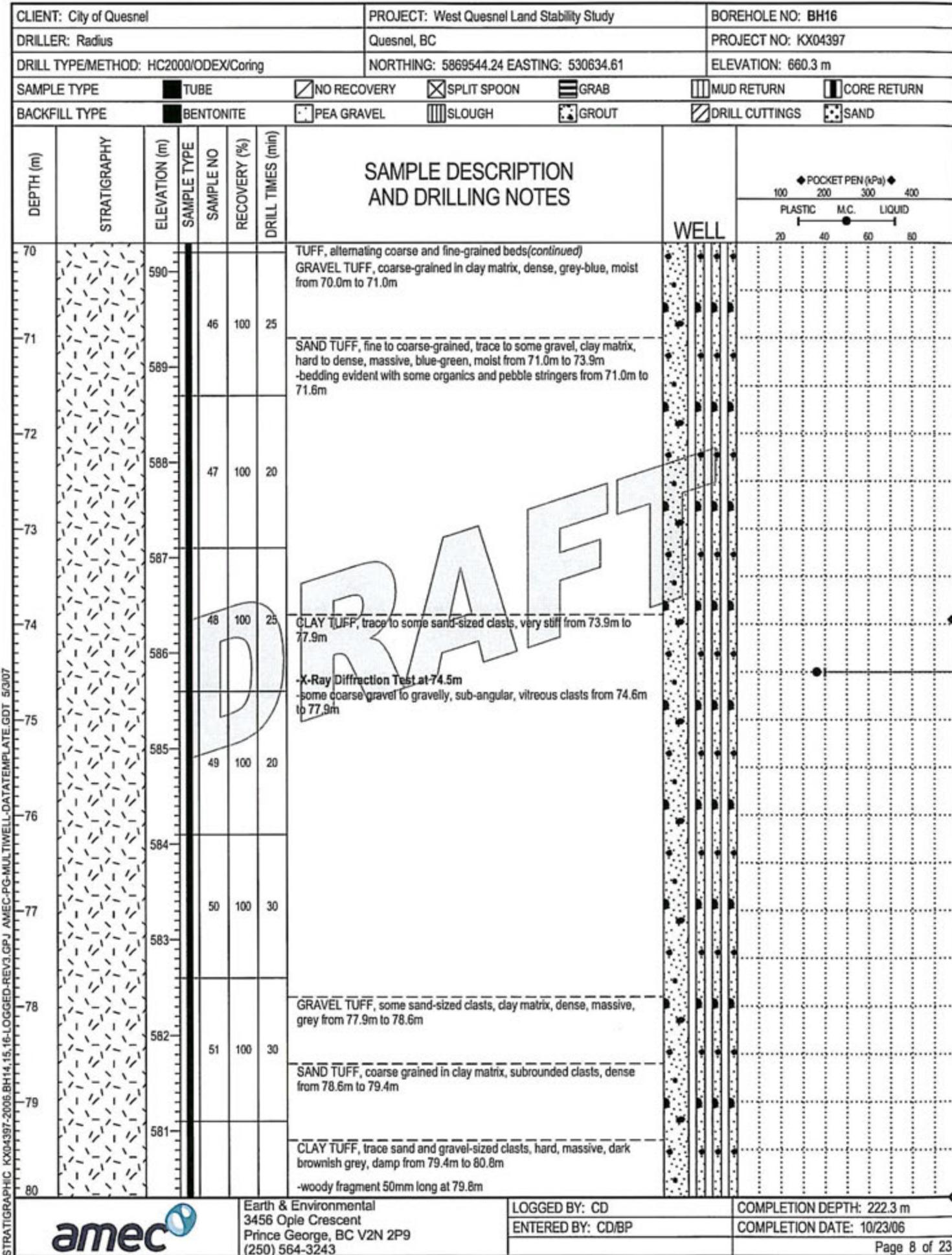


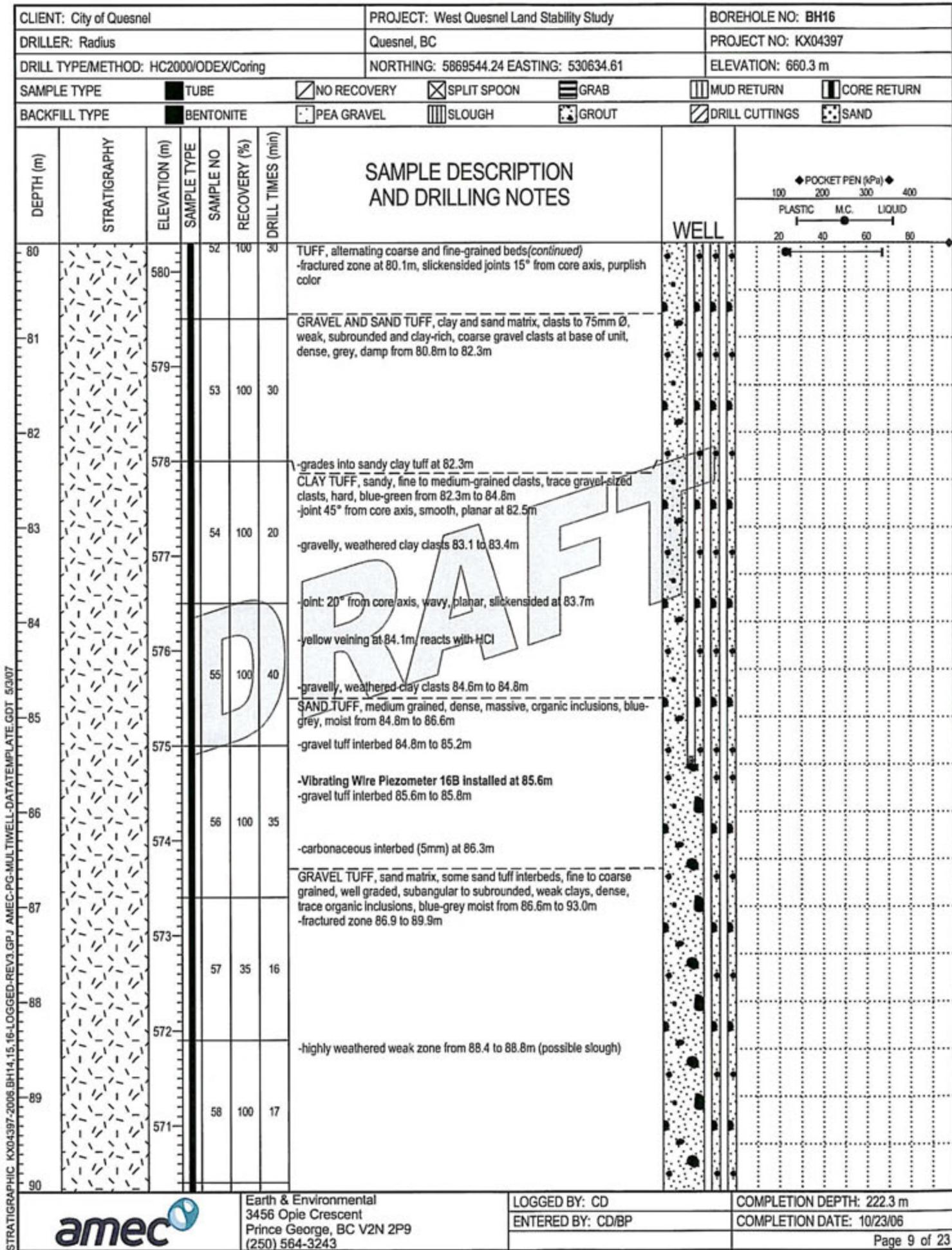


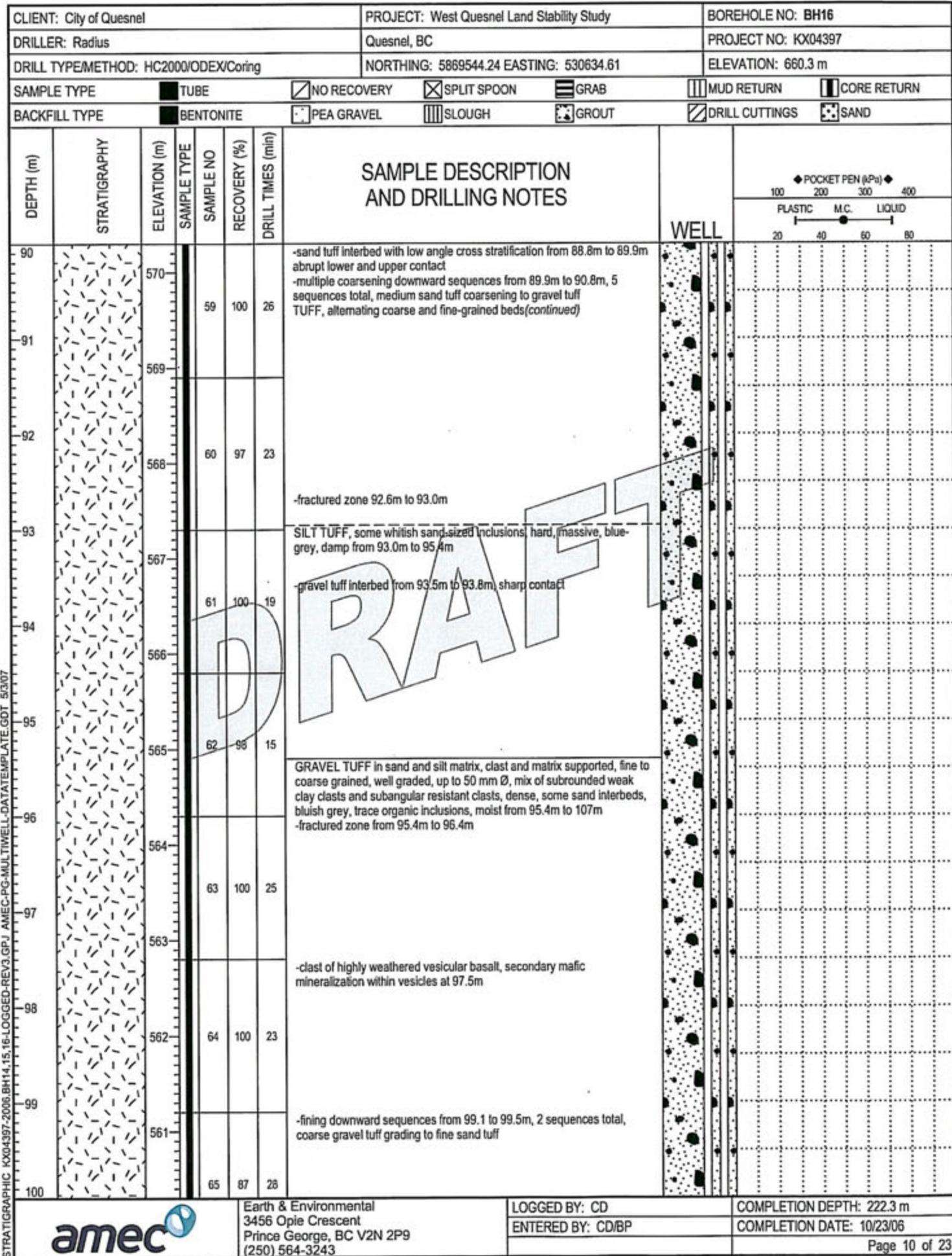


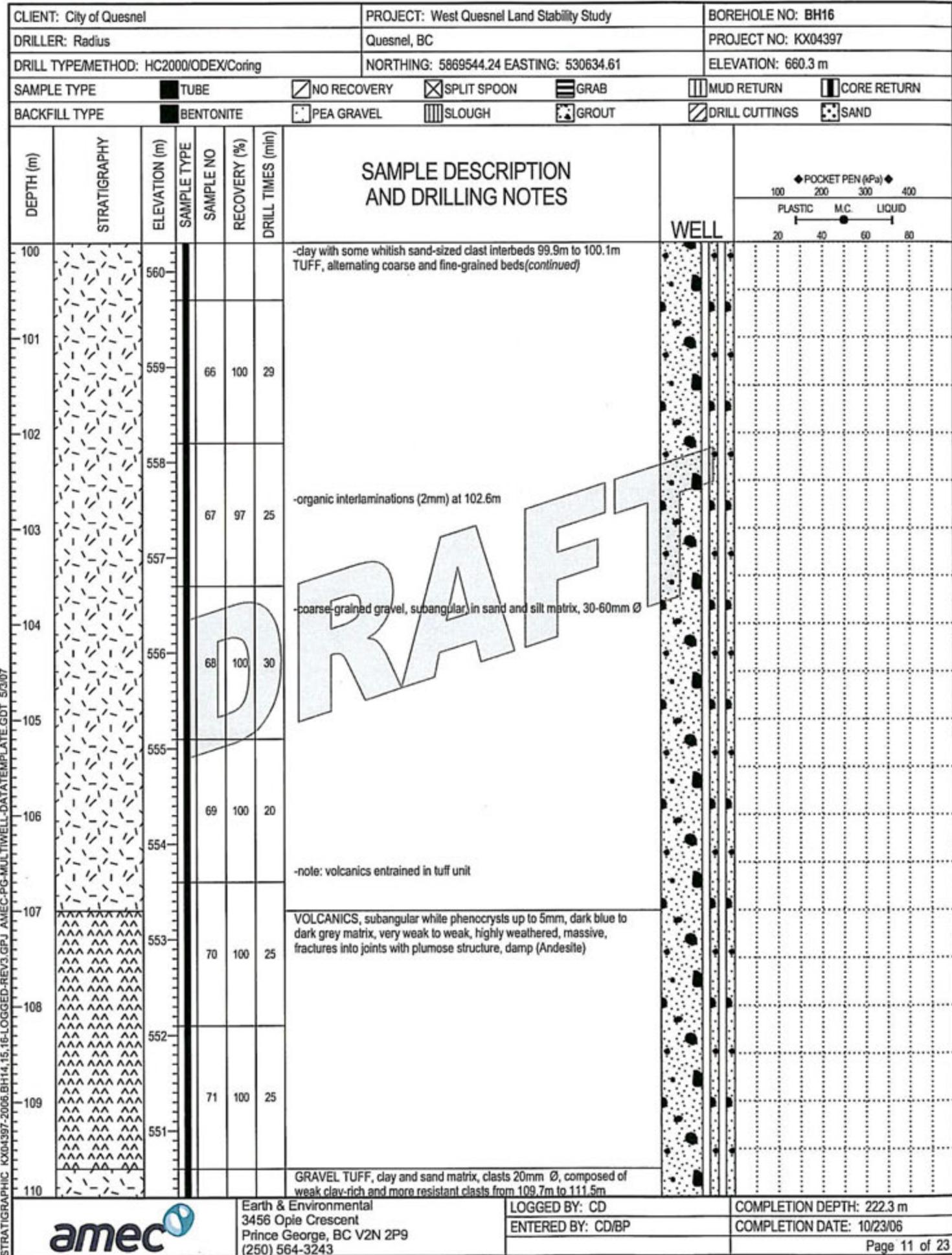


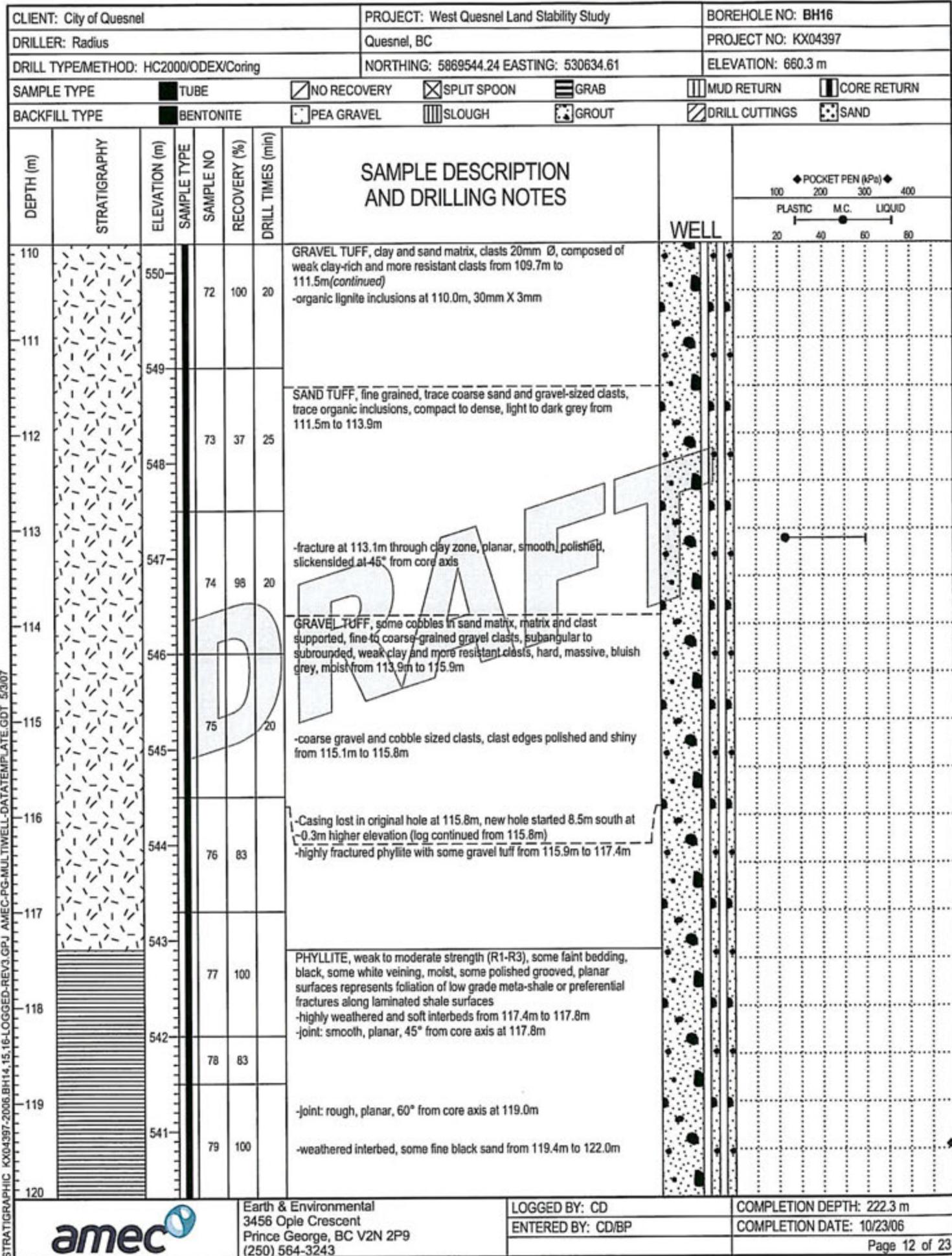


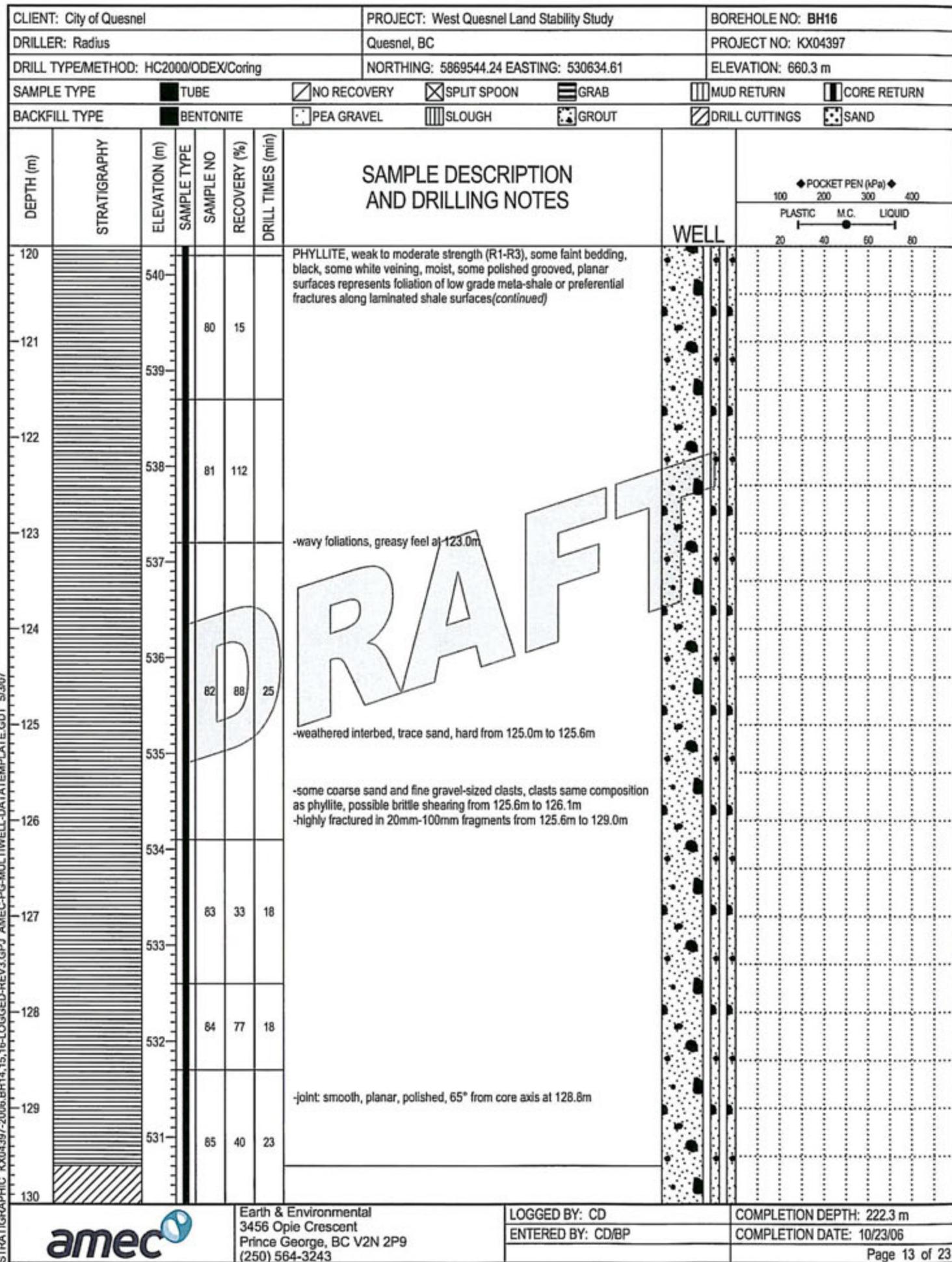


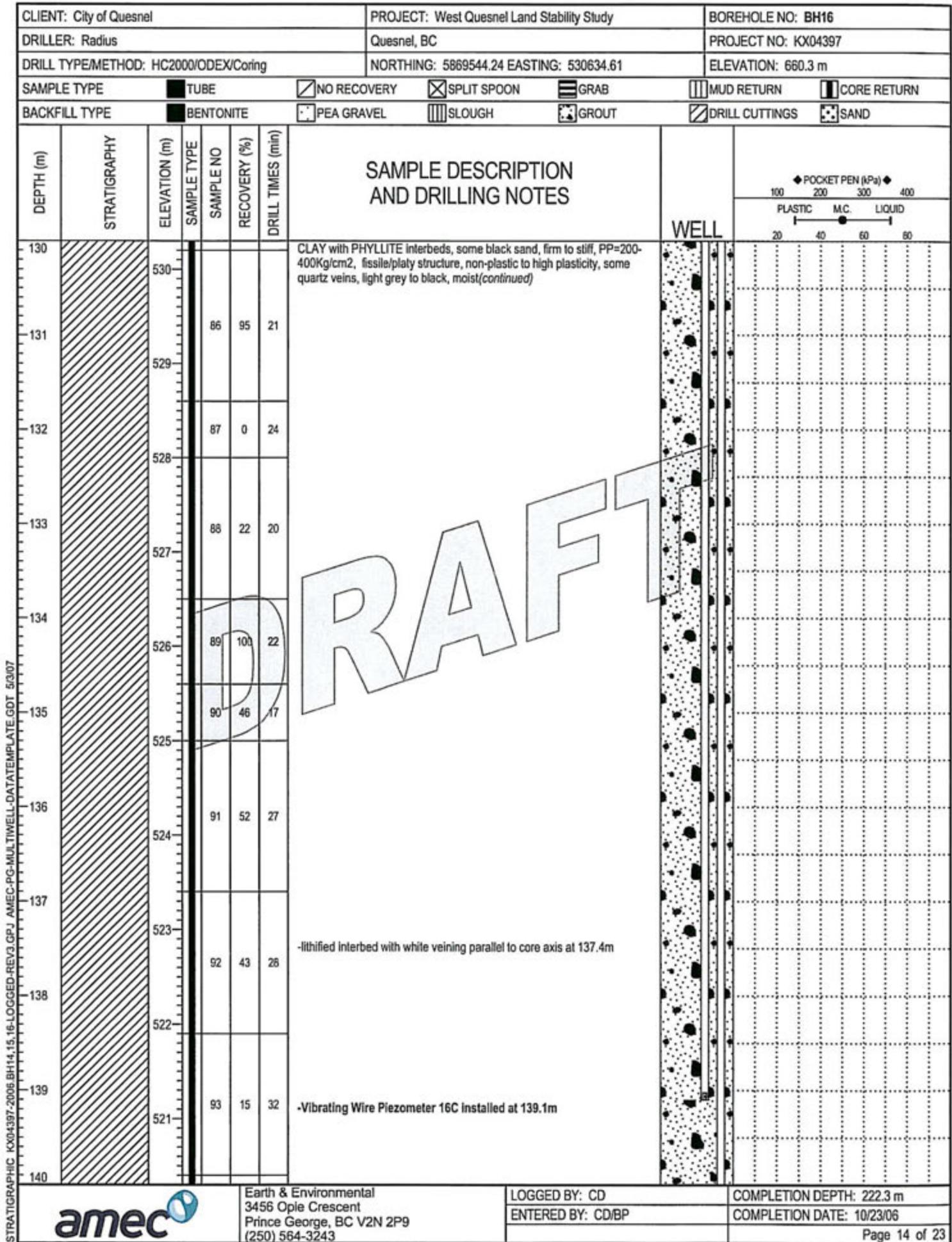




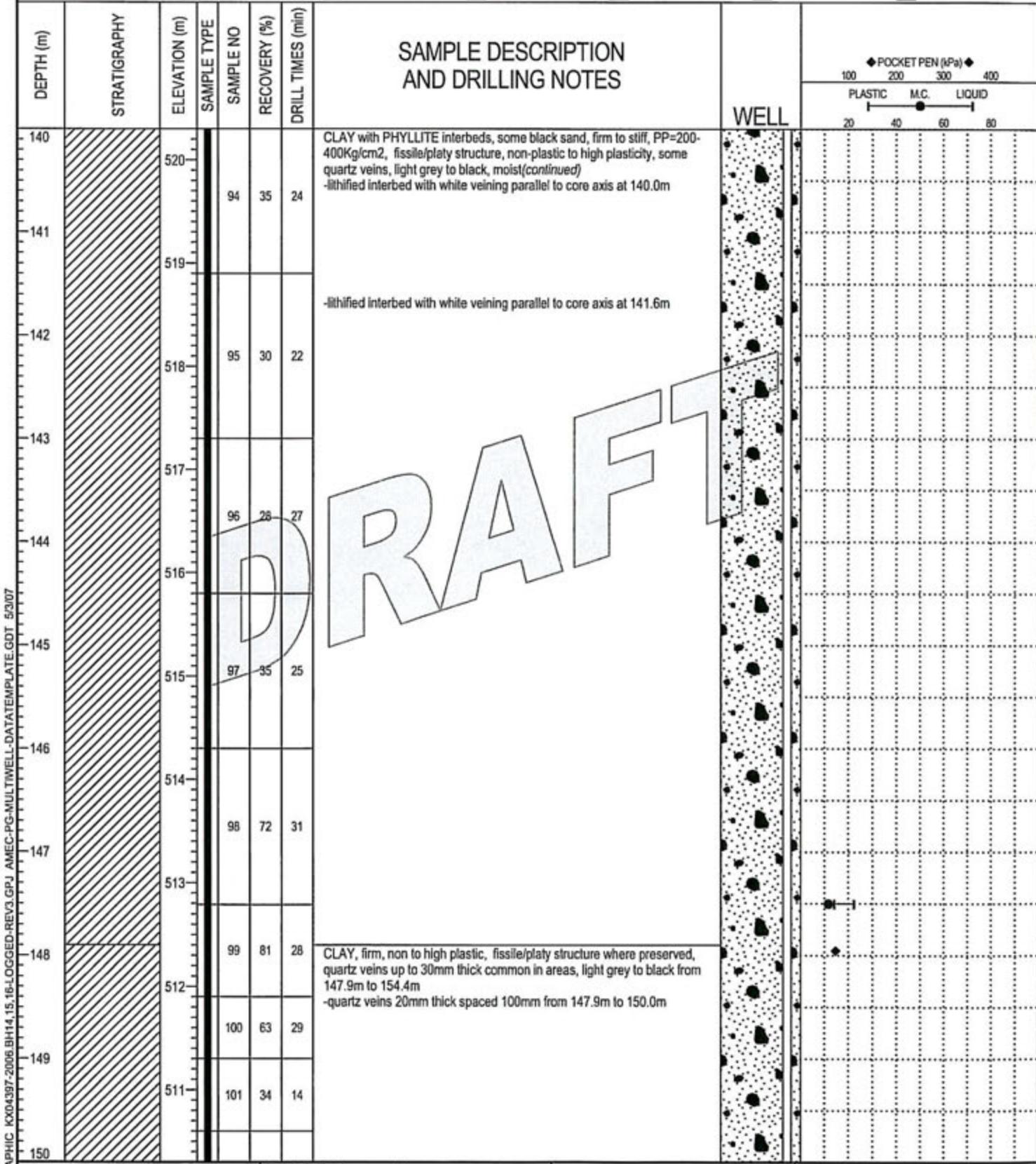








CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH16
DRILLER: Radius	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000/ODEX/Coring	NORTHING: 5869544.24 EASTING: 530634.61	ELEVATION: 660.3 m
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BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



STRATIGRAPHIC LOGGING FORM REV 3 GPJ 15.16 LOGGED 2006 BH14 15.16 AMEC PG MULTIWELL DATA TEMPLATE.GDT 5/3/07



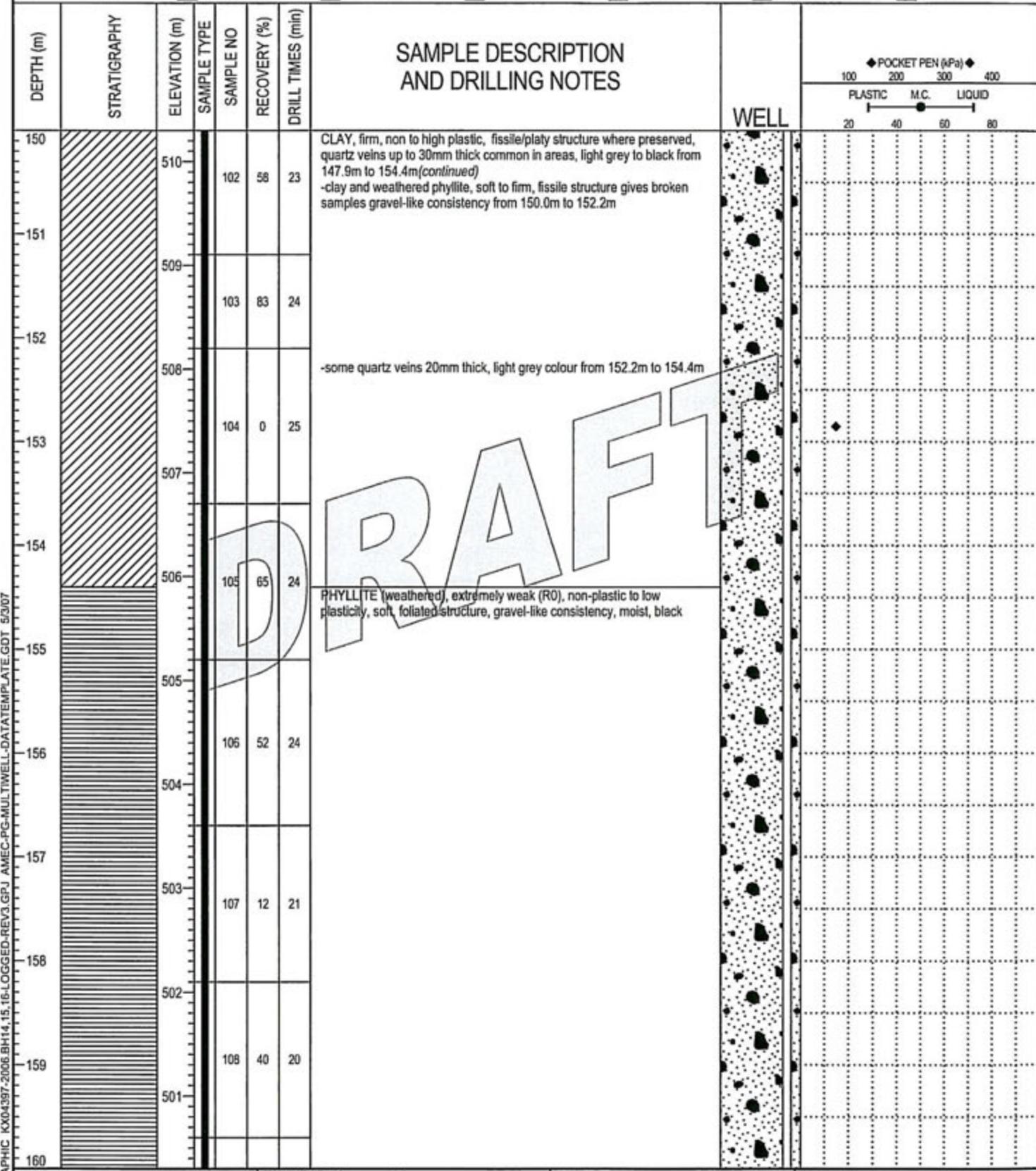
Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: CD
ENTERED BY: CD/BP

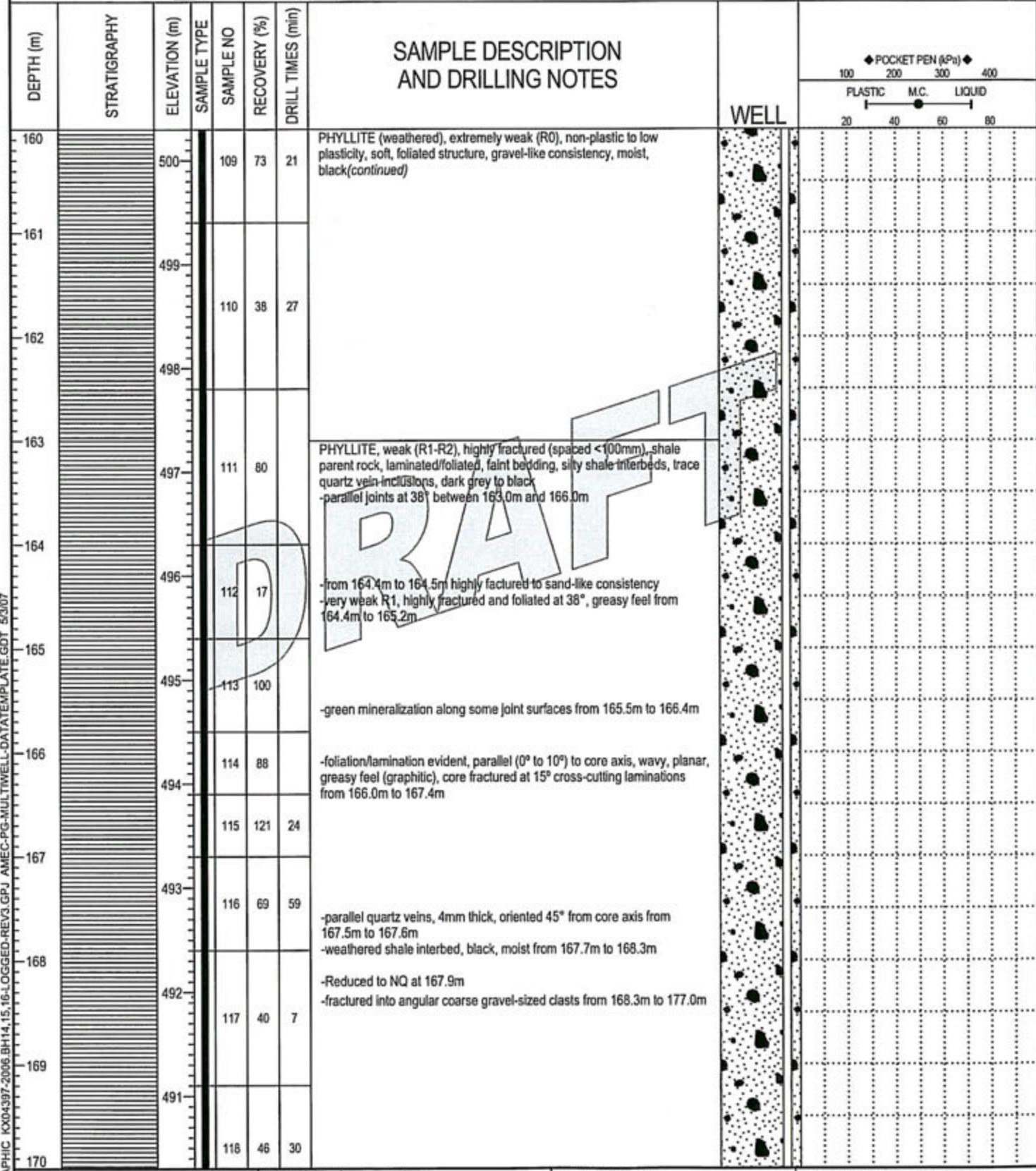
COMPLETION DEPTH: 222.3 m
COMPLETION DATE: 10/23/06

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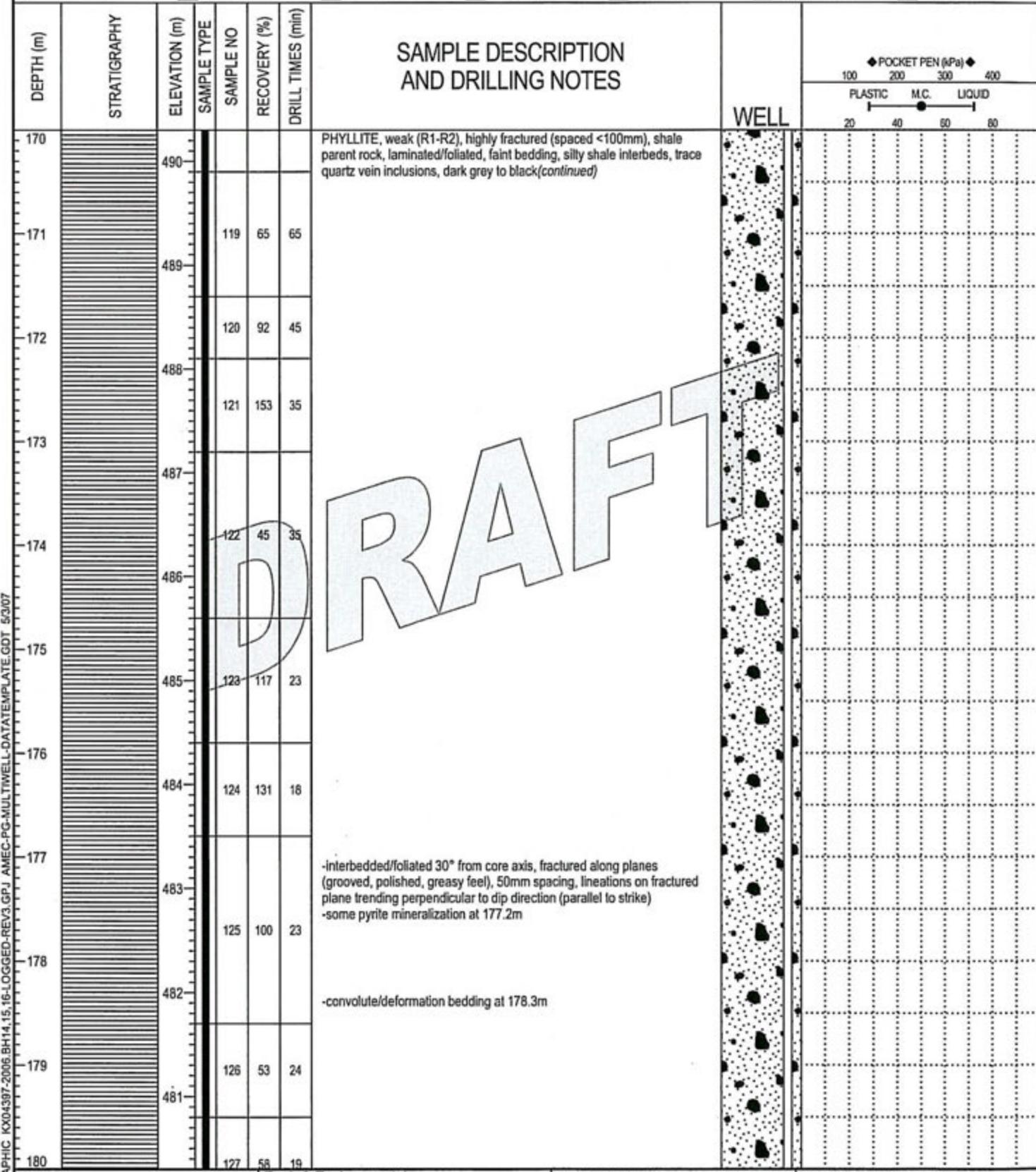
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DRILLER: Radius	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000/ODEX/Coring	NORTHING: 5869544.24 EASTING: 530634.61	ELEVATION: 660.3 m
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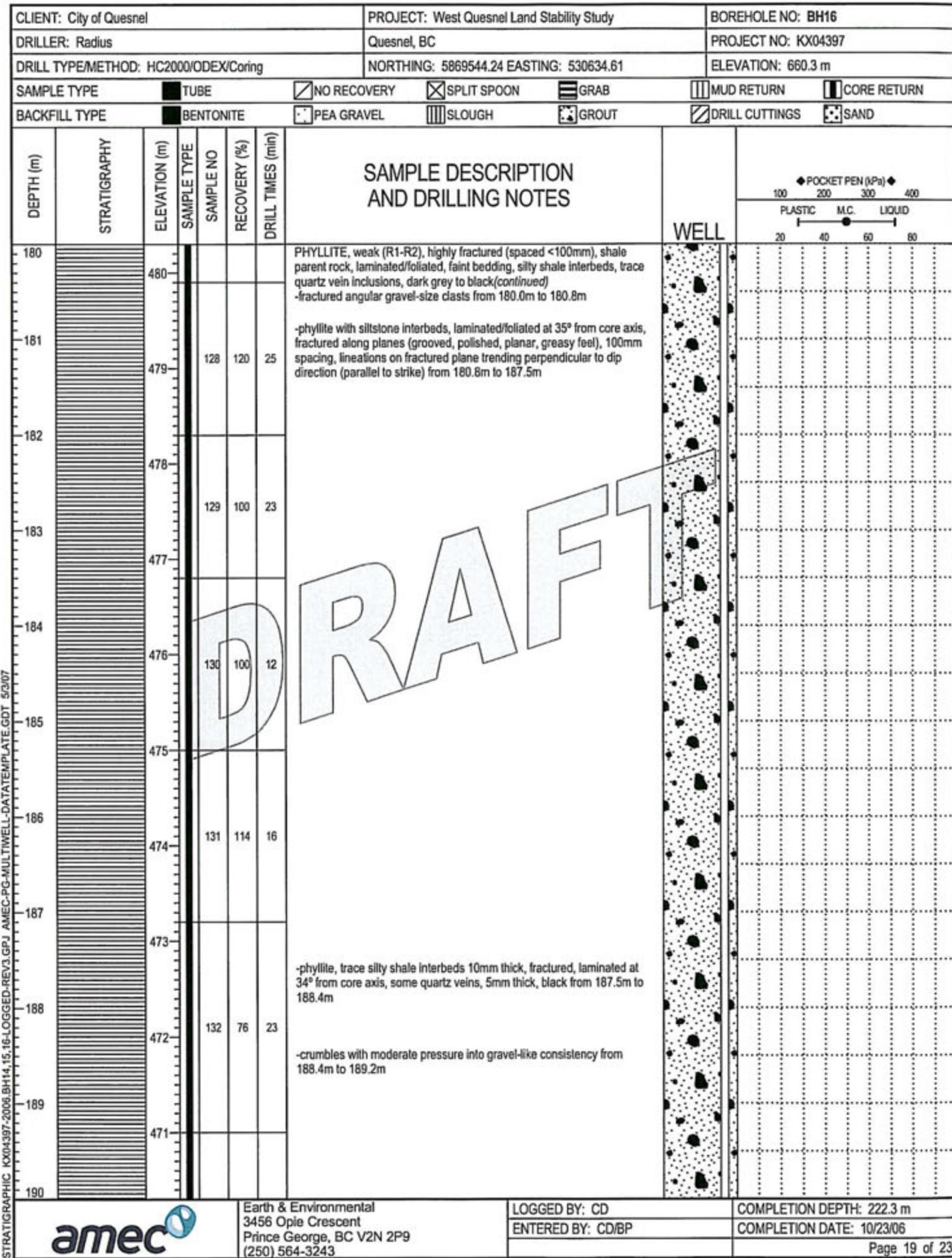


CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH16
DRILLER: Radius	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000/ODEX/Coring	NORTHING: 5869544.24 EASTING: 530634.61	ELEVATION: 660.3 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
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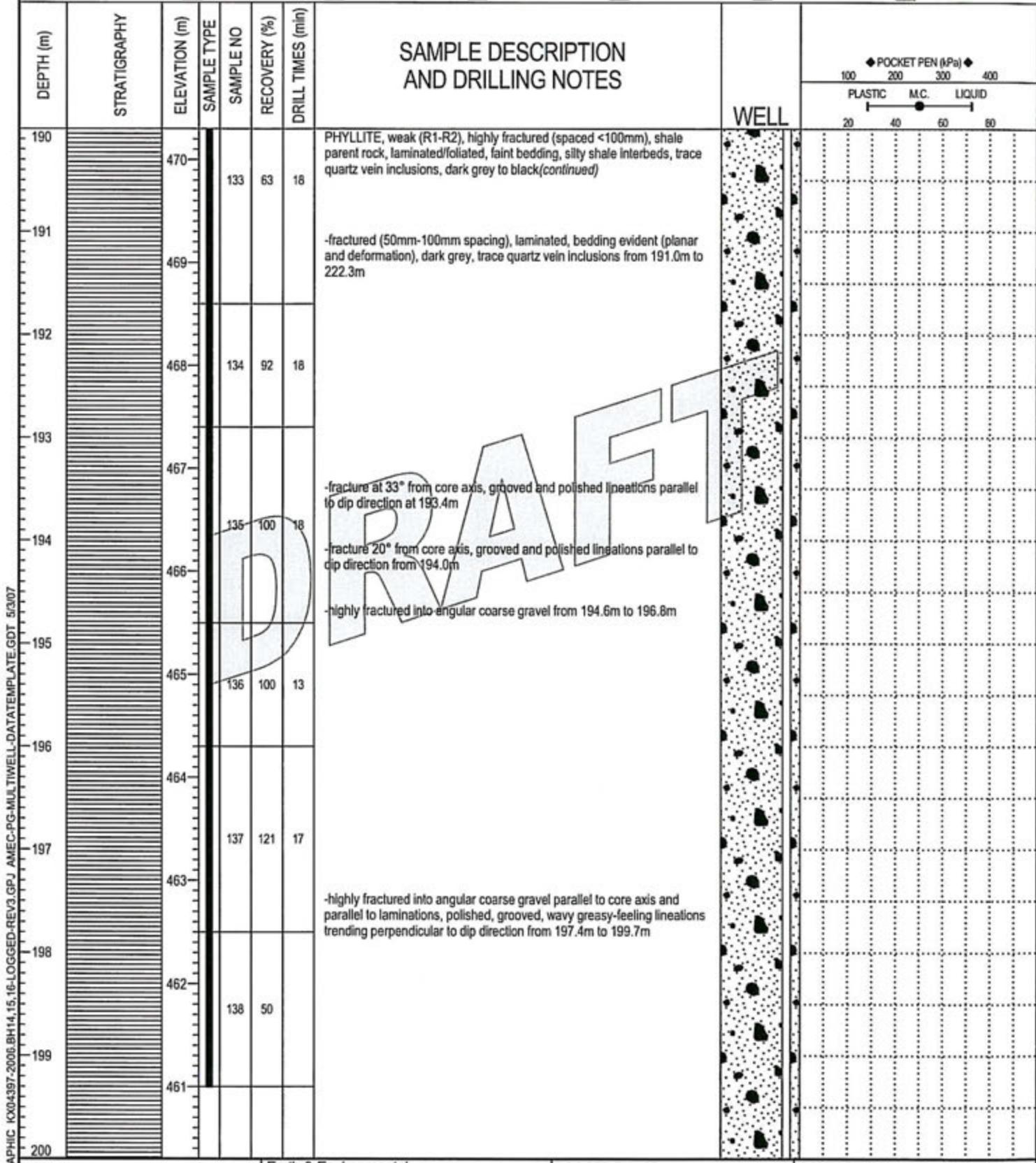


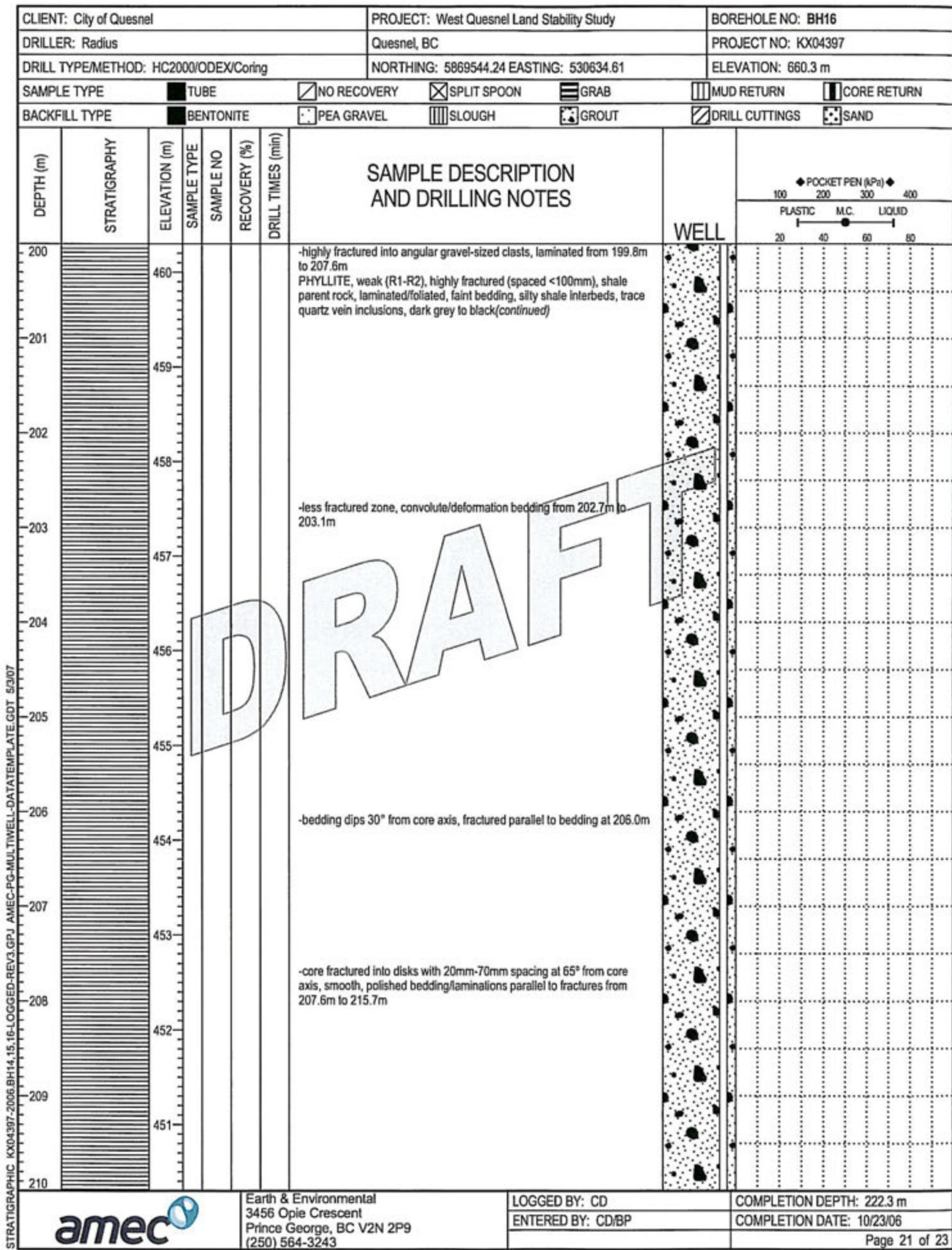
CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH16
DRILLER: Radius	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000/ODEX/Coring	NORTHING: 5869544.24 EASTING: 530634.61	ELEVATION: 660.3 m
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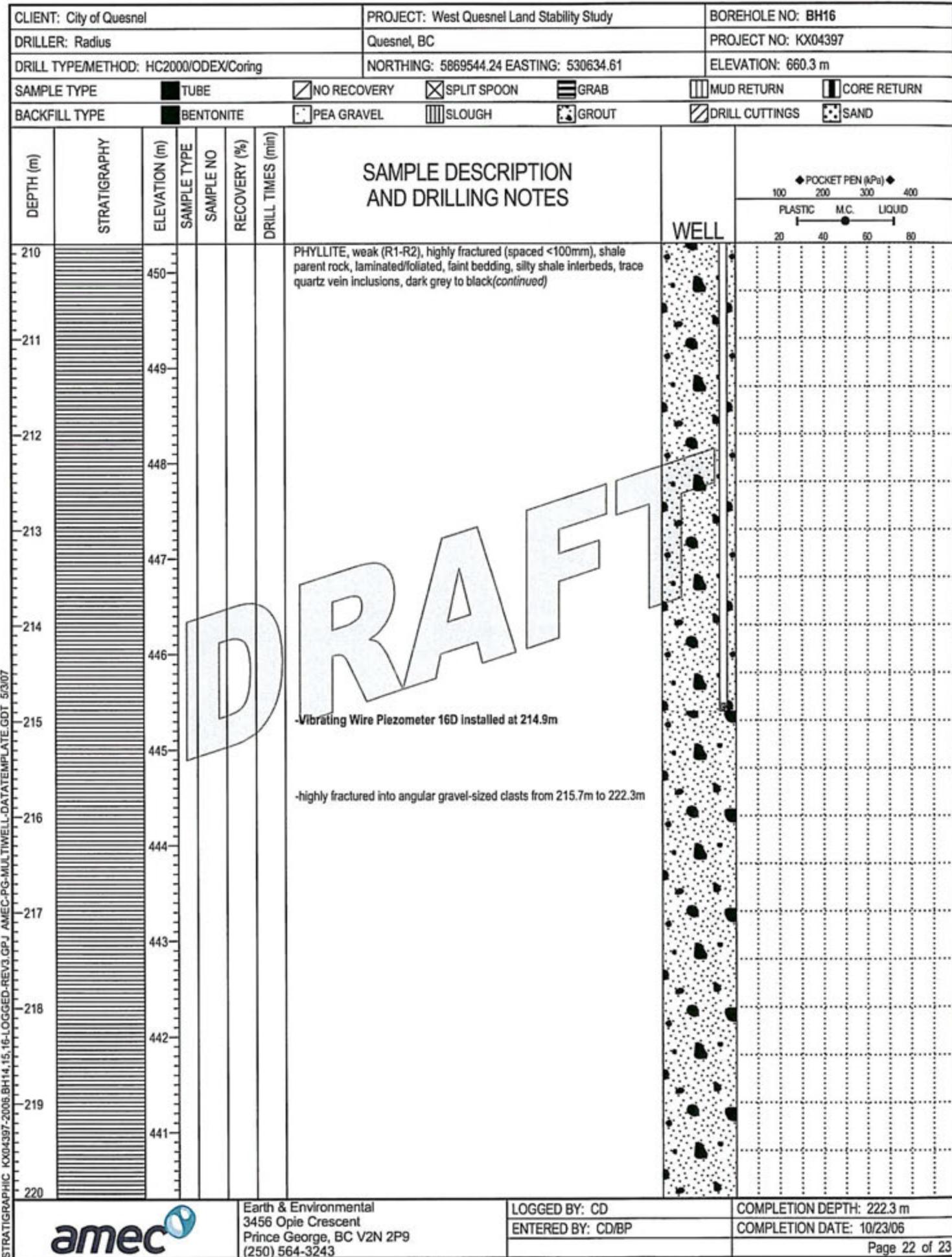


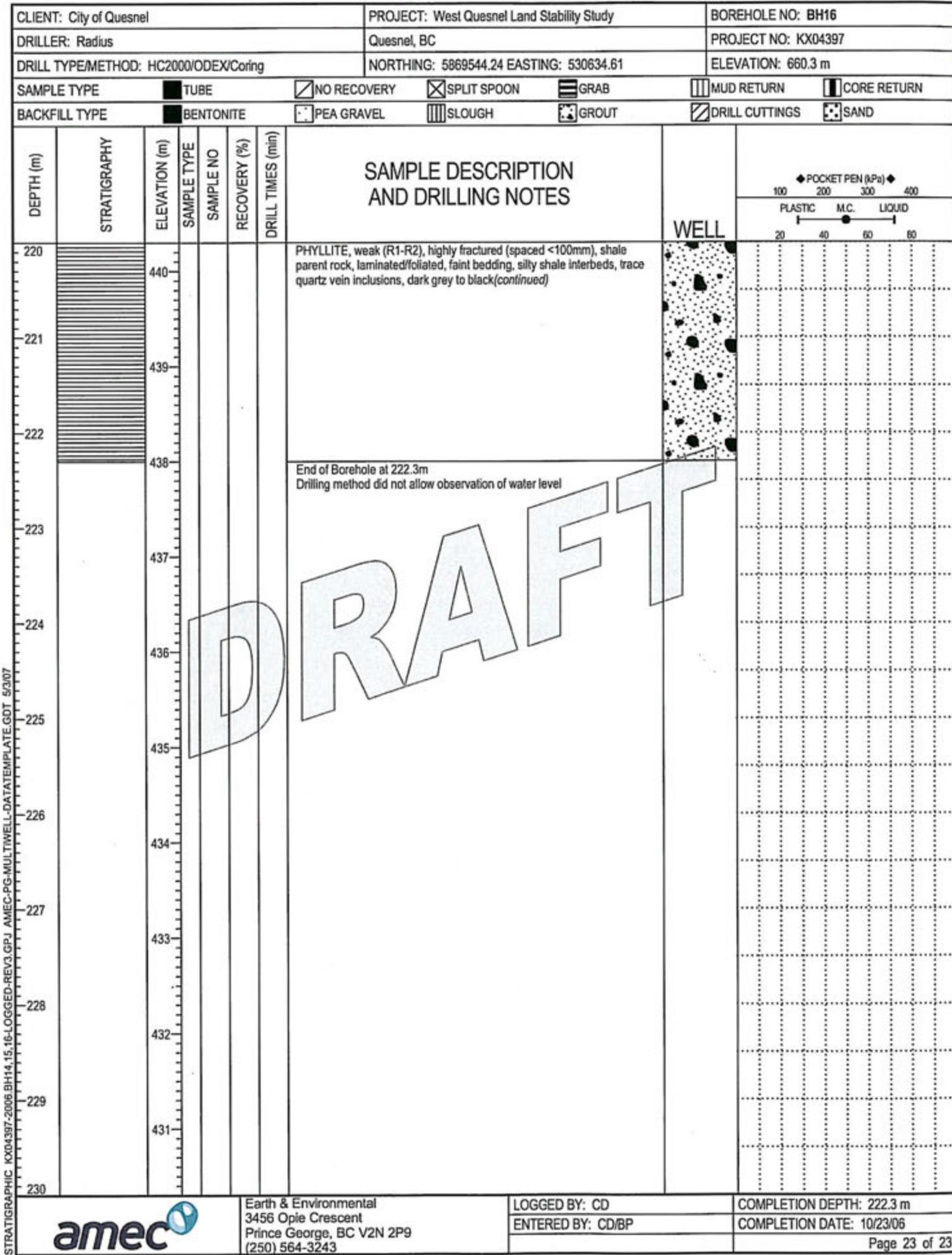


CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: BH16
DRILLER: Radius	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: HC2000/ODEX/Coring	NORTHING: 5869544.24 EASTING: 530634.61	ELEVATION: 660.3 m
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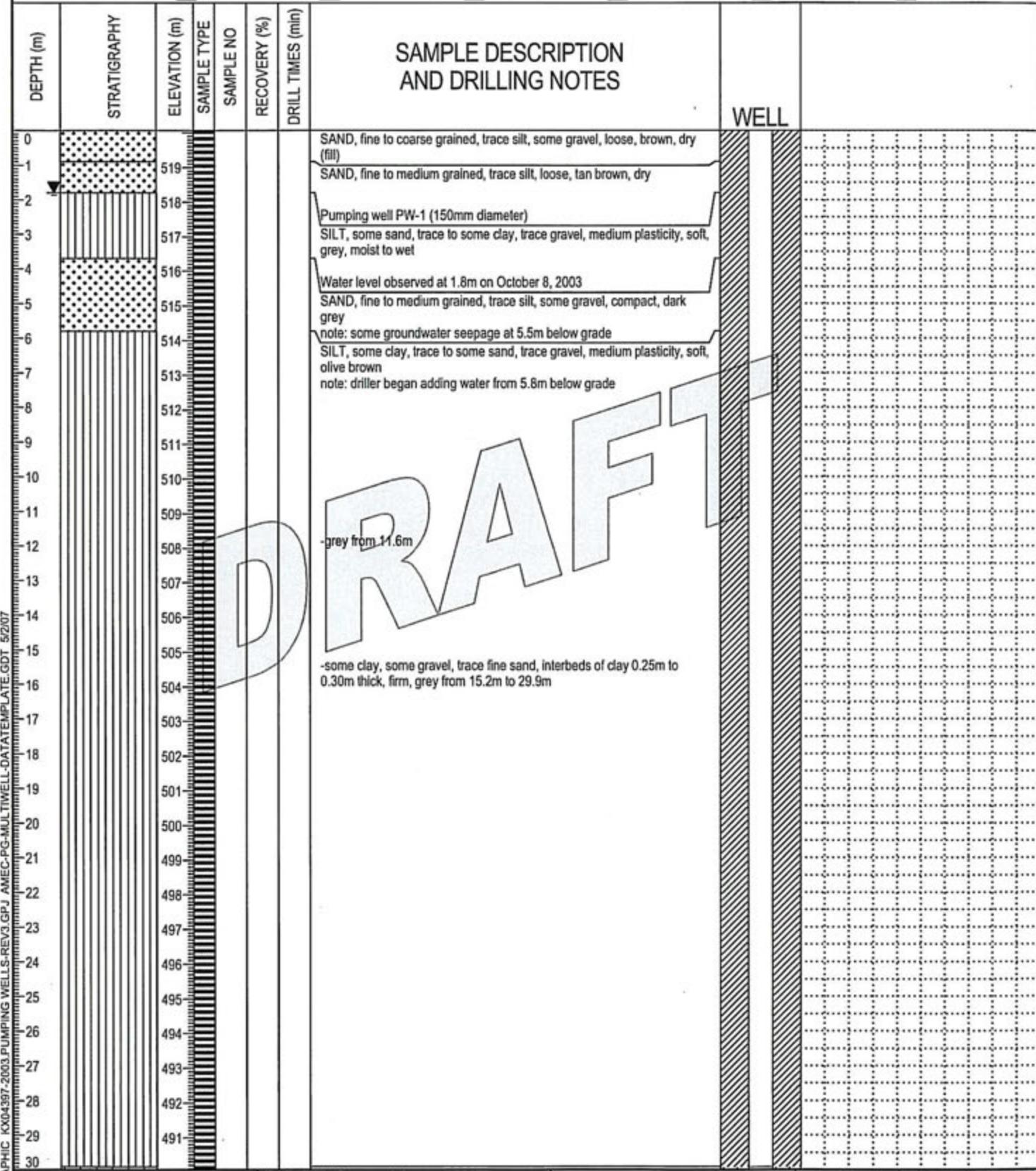








CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: PW-1
DRILLER: Cariboo Water Wells	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary	NORTHING: 5869772.5 EASTING: 531993.42	ELEVATION: 520.1 m
SAMPLE TYPE <input checked="" type="checkbox"/> TUBE <input type="checkbox"/> NO RECOVERY <input checked="" type="checkbox"/> SPLIT SPOON <input checked="" type="checkbox"/> GRAB <input type="checkbox"/> MUD RETURN <input type="checkbox"/> CORE RETURN		
BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input checked="" type="checkbox"/> SAND		



CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: PW-1	
DRILLER: Cariboo Water Wells			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary			NORTHING: 5869772.5 EASTING: 531993.42			ELEVATION: 520.1 m	
SAMPLE TYPE	<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN	
BACKFILL TYPE	<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input checked="" type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input checked="" type="checkbox"/> SAND	
DEPTH (m)	STRATIGRAPHY	ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	SAMPLE DESCRIPTION AND DRILLING NOTES
30		489					GRAVEL, subrounded, trace silt, some fine to coarse grained sand, grey(continued)
-31		489					-dark brown with some lignite below 31.4m
-32		488					LIGNITE, very weak, dark brown to black
-33		487					CLAY, trace to some silt, firm to stiff, high plasticity, light grey
-34		486					
-35		485					
-36		484					LIGNITE, very weak, dark brown to black
-37		483					CLAY, trace silt, trace black coarse grained sand, firm to stiff, high plasticity, blue grey
-38		482					
-39		481					-trace silt, some lignite, high plasticity, dark brown to black- from 39.3m to 40.8m
-40		480					-trace silt, firm, high plasticity, dark brown from 40.8m to 41.6m
-41		479					LIGNITE, clay interbeds <0.10m, very weak, dark brown to black
-42		478					-drilled open hole from 42.7m
-43		477					CLAY, trace silt, trace to some black sand, firm, high plasticity, light brown
-44		476					
-45		475					
-46		474					CLAY TUFF, trace silt, trace to some black and green sand, very stiff to dense
-47		473					-LIGNITE interbeds, thin (<20mm), with 1.5m spacing at 46.5m
-48		472					-light brown from 47.5m to 47.7m
-49		471					
-50		470					
-51		469					
-52		468					
-53		467					
-54		466					
-55		465					End of Borehole 54.9m below grade
-56		464					
-57		463					
-58		462					
-59		461					
-60		460					

STRATIGRAPHIC KX04397-2003 PUMPING WELLS-REV3.GPJ AMEC PG-MULTIWELL-DATATEMPLATE GDT 5/2007

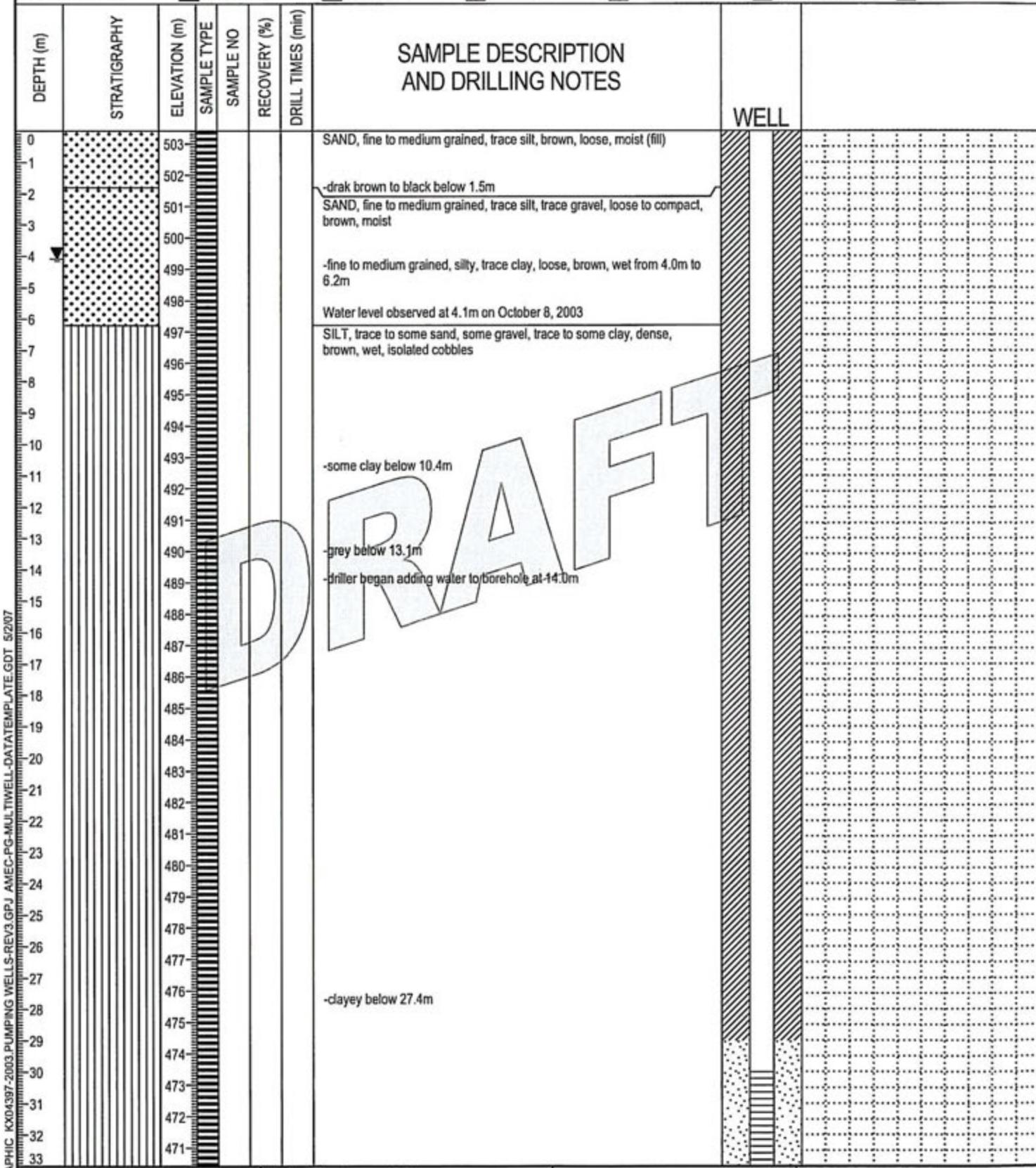


Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: SG
ENTERED BY: CD

COMPLETION DEPTH: 54.9 m
COMPLETION DATE: 9/16/03

CLIENT: City of Quesnel	PROJECT: West Quesnel Land Stability Study	BOREHOLE NO: PW-2
DRILLER: Cariboo Water Wells	Quesnel, BC	PROJECT NO: KX04397
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary	NORTHING: 5869595.41 EASTING: 532272.27	ELEVATION: 503.4 m
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BACKFILL TYPE <input checked="" type="checkbox"/> BENTONITE <input type="checkbox"/> PEA GRAVEL <input type="checkbox"/> SLOUGH <input checked="" type="checkbox"/> GROUT <input type="checkbox"/> DRILL CUTTINGS <input type="checkbox"/> SAND		



Earth & Environmental
3456 Opie Crescent
Prince George, BC V2N 2P9
(250) 564-3243

LOGGED BY: SG
ENTERED BY: CD

COMPLETION DEPTH: 61.0 m
COMPLETION DATE: 9/18/03

CLIENT: City of Quesnel			PROJECT: West Quesnel Land Stability Study			BOREHOLE NO: PW-2	
DRILLER: Cariboo Water Wells			Quesnel, BC			PROJECT NO: KX04397	
DRILL TYPE/METHOD: Ingersoll-Rand TH-60/Air Rotary			NORTHING: 5869595.41 EASTING: 532272.27			ELEVATION: 503.4 m	
SAMPLE TYPE		<input checked="" type="checkbox"/> TUBE	<input type="checkbox"/> NO RECOVERY	<input checked="" type="checkbox"/> SPLIT SPOON	<input checked="" type="checkbox"/> GRAB	<input type="checkbox"/> MUD RETURN	<input type="checkbox"/> CORE RETURN
BACKFILL TYPE		<input checked="" type="checkbox"/> BENTONITE	<input type="checkbox"/> PEA GRAVEL	<input type="checkbox"/> SLOUGH	<input type="checkbox"/> GROUT	<input type="checkbox"/> DRILL CUTTINGS	<input type="checkbox"/> SAND
DEPTH (m)	STRATIGRAPHY	SAMPLE DESCRIPTION AND DRILLING NOTES					WELL
		ELEVATION (m)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%)	DRILL TIMES (min)	
33		470				SILT, trace to some sand, some gravel, trace to some clay, dense, brown, wet, isolated cobbles(continued)	
34		469					
35		468					
36		467					
37		466					
38		465				-GRAVEL layer, 15cm thick, at 37.5m	
39		464				CLAY TUFF, trace sill, some sand, very dense, green, moist	
40		463					
41		462				-no sand from 41.1m to 43.3m	
42		461					
43		460				-driller drilled open hole below 42.7m below grade with casing drive shoe at 42.7m no groundwater seepage was observed in borehole after being left undisturbed overnight	
44		459					
45		458					
46		457					
47		456				-LIGNITE layers, thin (<5cm), at 46.6m and 48.5m	
48		455				-sandy between 47.2m and 48.2m	
49		454					
50		453					
51		452					
52		451					
53		450					
54		449					
55		448					
56		447					
57		446					
58		445					
59		444					
60		443					
61		442				End of Borehole 61.0m below grade	
62		441					
63		440					
64		439					
65		438					
STRATIGRAPHIC LOGGING SHEET - PW-2							
amec			Earth & Environmental 3456 Opie Crescent Prince George, BC V2N 2P9 (250) 564-3243		LOGGED BY: SG ENTERED BY: CD		COMPLETION DEPTH: 61.0 m COMPLETION DATE: 9/18/03
Page 2 of 2							

BH15 (65.8m to 100.6m)

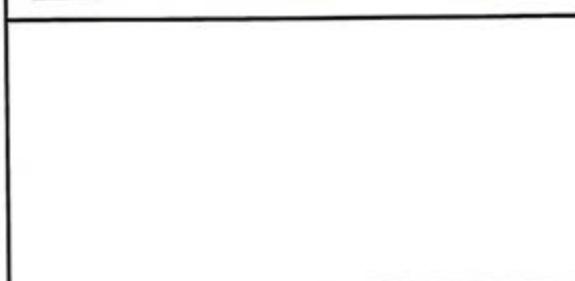
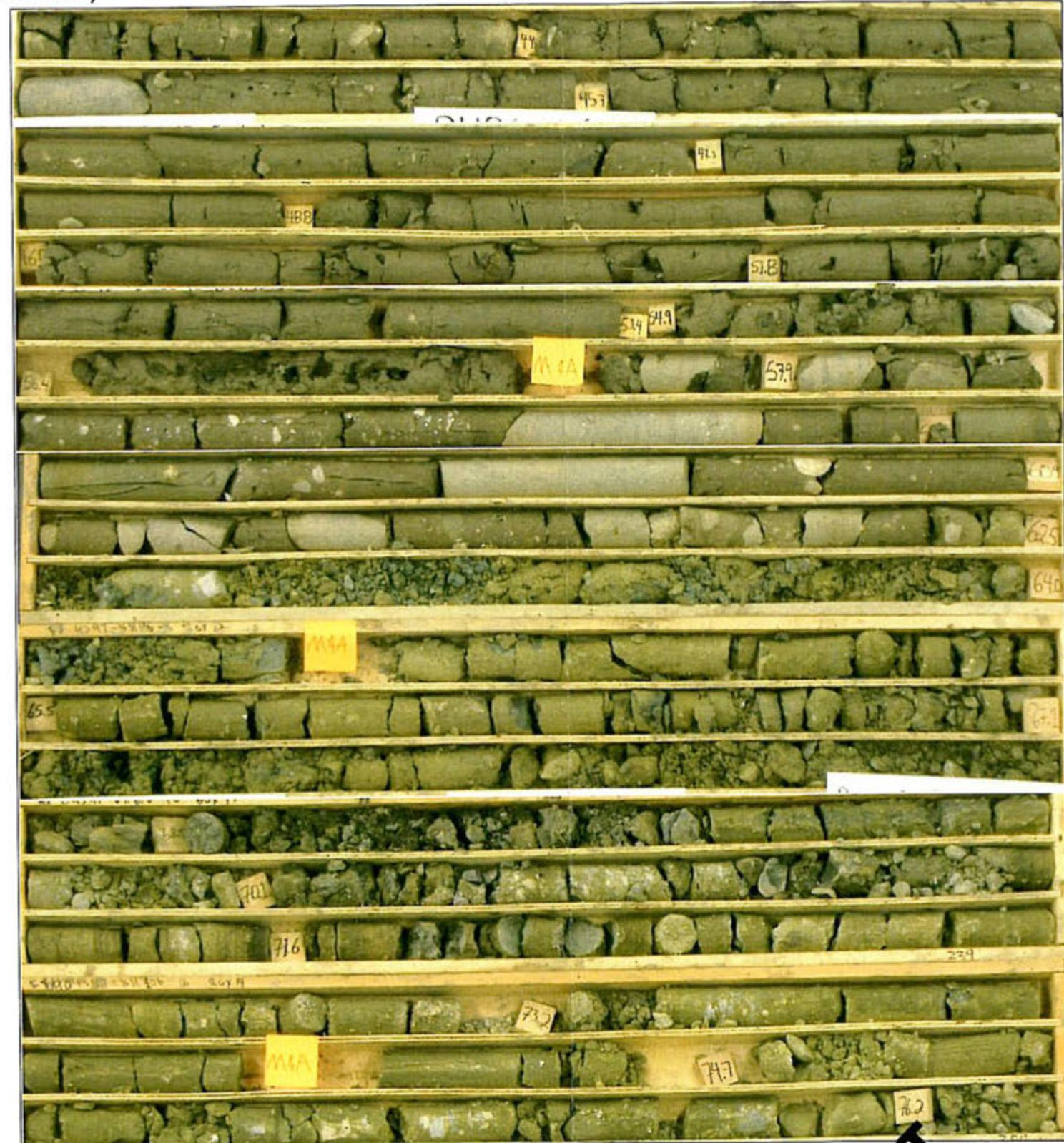
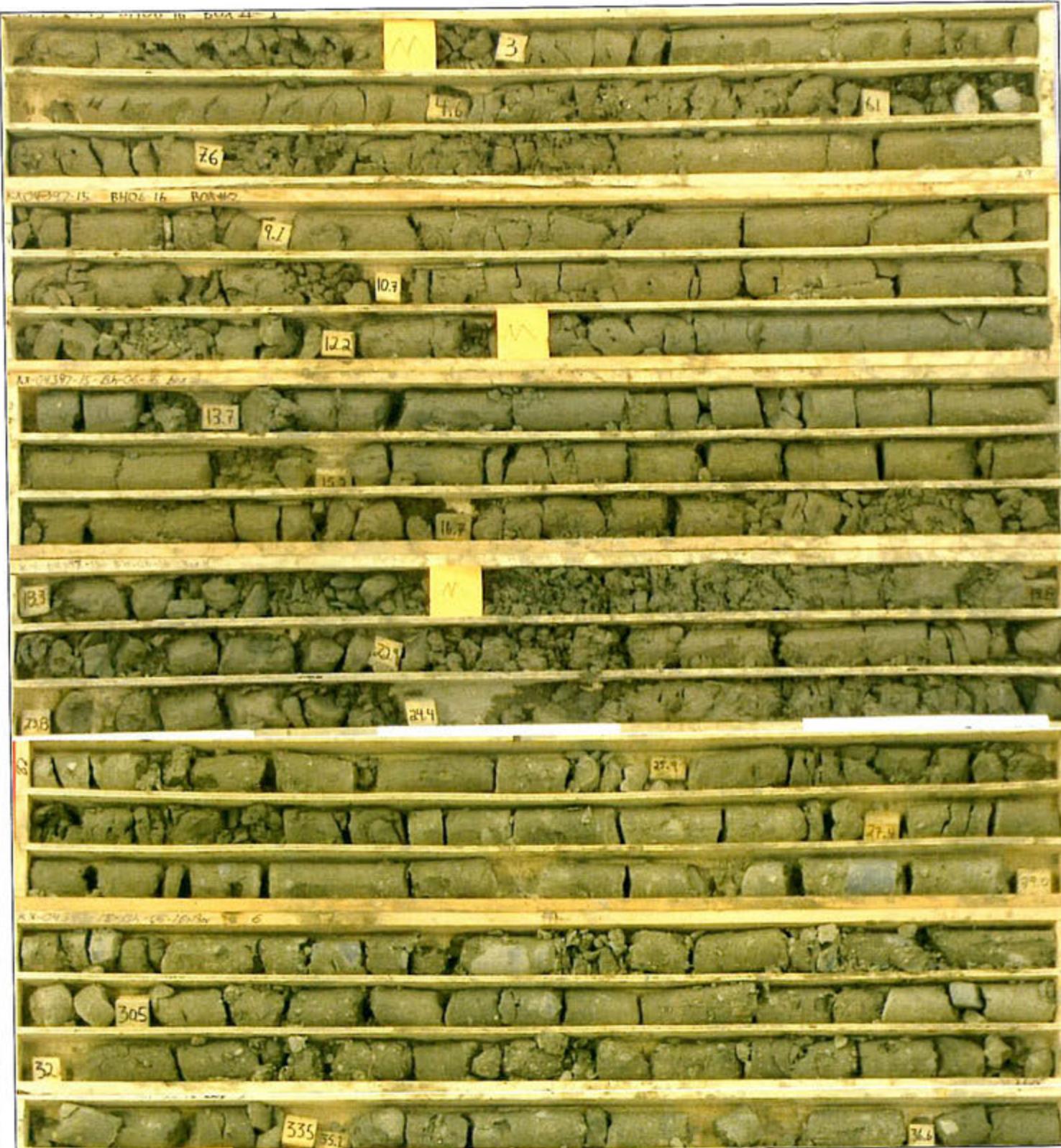


CLIENT: CITY OF QUESNEL	AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	amec	DWN BY: S.Ruiz	TITLE DRILL CORE PHOTOS BH15 (2 of 2)	DATE: APRIL 2007
			CHK'D BY: S.Kelly		PROJECT NO: KX04397
			DATUM: NAD83	PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	REV. NO.: A
			PROJECTION: UTM Zone 10		FIGURE No. BH15
			SCALE: NOT TO SCALE		

DRAFT

This drawing was originally produced in colour.

BH16 (0.0m to 76.5m)



**West Quesnel
Land Stability Program**

CITY OF QUESNEL
AMEC Earth & Environmental
3456 Opie Crescent
Prince George, BC, CANADA V2N 2P9
Tel. (250) 564-3243
Fax (250) 562-7045



DWN BY:	S.Ruiz	TITLE	DRILL CORE PHOTOS	DATE:	APRIL 2007
CHK'D BY:	S.Kelly		BH16 (1 of 3)	PROJECT NO:	KX04397
DATUM:	NAD83	PROJECT	WEST QUESNEL LAND STABILITY STUDY	REV. NO.:	A
PROJECTION:	UTM Zone 10		QUESNEL, BC	FIGURE No.	BH16
SCALE:	NOT TO SCALE				

DRAFT

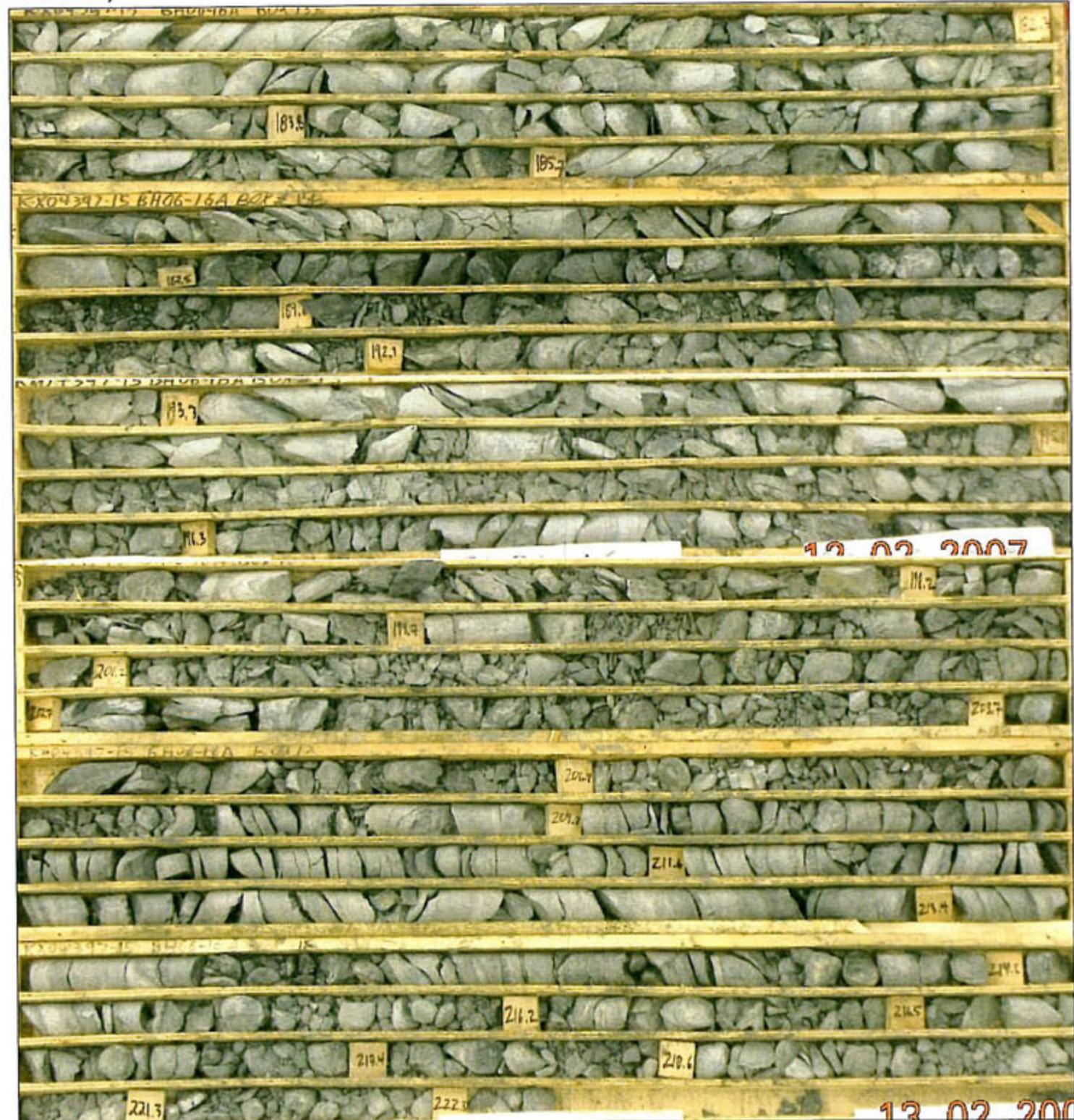
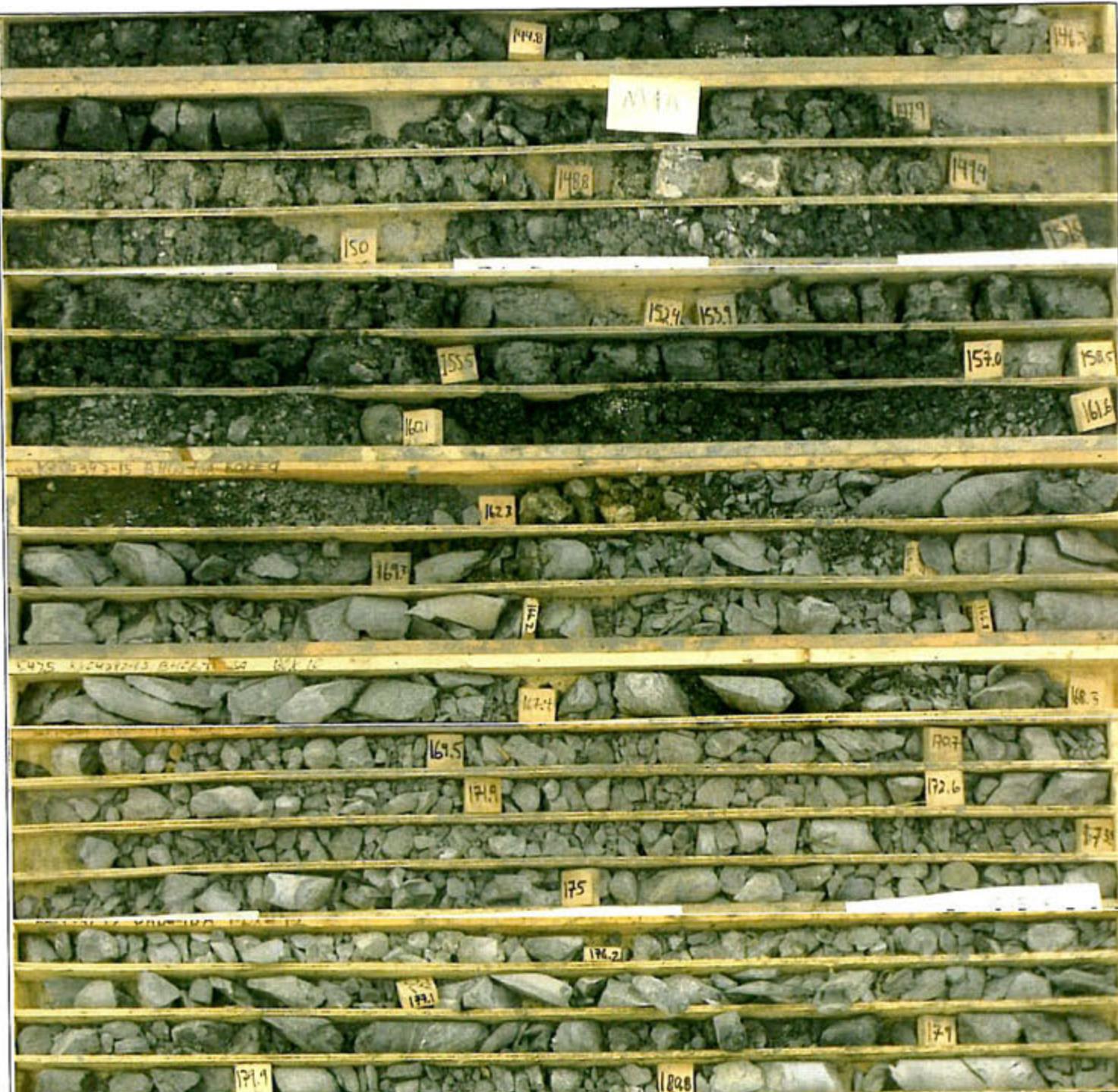
BH16 (76.5m to 143.3m)



	 West Quesnel Land Stability Program	CLIENT:	CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045			DWN BY: S.Ruiz CHK'D BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	DRILL CORE PHOTOS BH16 (2 of 3) WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007 PROJECT NO: KX04397 REV. NO.: A FIGURE No. BH16

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BH16 (143.3m to 223.3m)



	 West Quesnel Land Stability Program	CLIENT: CITY OF QUESNEL	DWN BY: S.Ruiz	TITLE	DATE: APRIL 2007				
			CHK'D BY: S.Kelly						
	AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045		DATUM: NAD83	PROJECT	PROJECT NO: KX04397				
			PROJECTION: UTM Zone 10						
			SCALE: NOT TO SCALE	WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	REV. NO.: A				
DRAFT									
This drawing was originally produced in colour.									

BH7 (0.0m to 52.3m)



West Quesnel
Land Stability Program

CLIENT: CITY OF QUESNEL

AMEC Earth & Environmental
3456 Opie Crescent
Prince George, BC, CANADA V2N 2P9
Tel. (250) 564-3243
Fax (250) 562-7045



DWN BY: S.Rui
CHK'D BY: S.Kell
DATUM: NAD83
PROJECTION: UTM Zone 10
SCALE: NOT TO SCALE

DRILL CORE PHOTOS
BH7 (1 of 2)

WEST QUESNEL LAND STABILITY STUDY
QUESNEL, BC

DATE:	APRIL 2007
PROJECT NO.:	KX04397
REV. NO.:	A
FIGURE No.	BH7

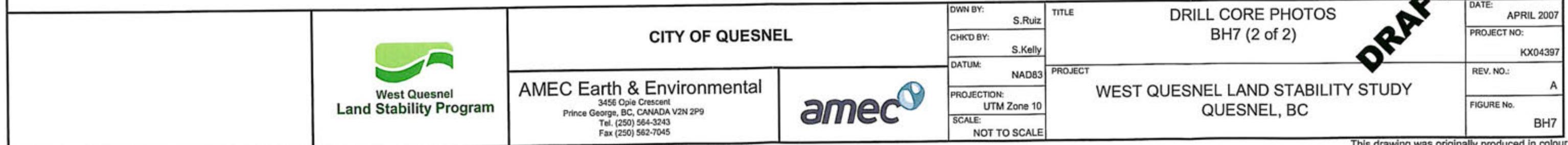
DRAFT

This drawing was originally produced in colour

BH7 (52.3m to 78.9m)



11.07.20



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This drawing was originally produced in colour

BH8 (0.0m to 65.4m)



BH9 (0.0m to 47.4m)



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	 West Quesnel Land Stability Program	CITY OF QUESNEL	DWN BY: S.Ruiz	DRILL CORE PHOTOS BH9 (1 of 2)	DATE: APRIL 2007
			CHK'D BY: S.Kelly		
		DATUM: NAD83	PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	REV. NO.: A	
		PROJECTION: UTM Zone 10		FIGURE No. BH9	
		SCALE: NOT TO SCALE			

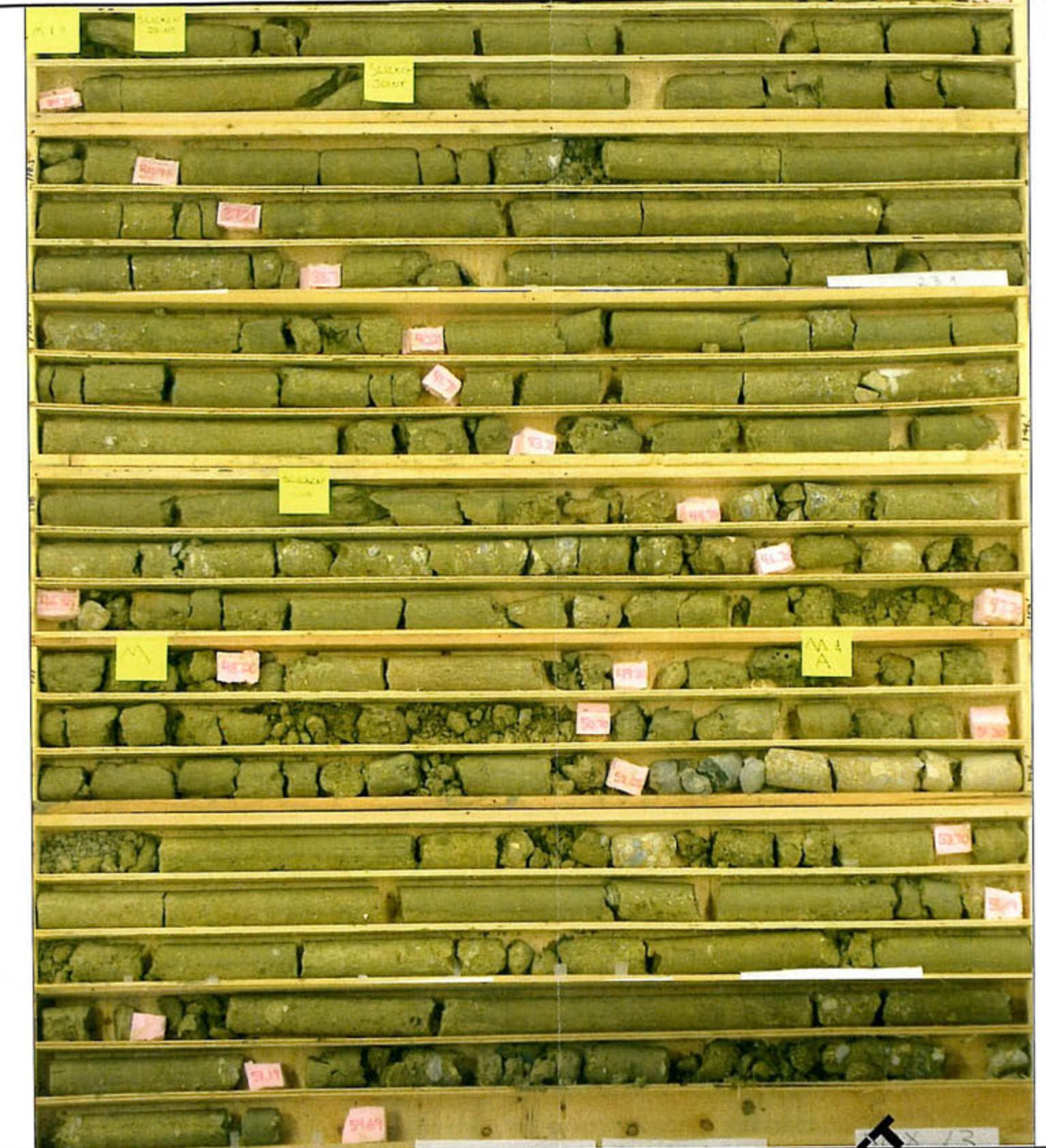
BH9 (47.4m to 69.3m)



West Quesnel Land Stability Program	CLIENT: CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	DWN BY: S.Ruiz	TITLE PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007	
		CHK'D BY: S.Kelly		PROJECT NO: KX04397	
		DATUM: NAD83		REV. NO.: A	
		PROJECTION: UTM Zone 10		FIGURE No. BH9	
		SCALE: NOT TO SCALE			

DRAFT

BH10 (0.0m to 59.7m)



	 West Quesnel Land Stability Program	CLIENT:	CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	DWN BY: S.Ruiz CHK'D BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	DRILL CORE PHOTOS BH10 (1 of 1) WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007 PROJECT NO: KX04397 REV. NO.: A FIGURE No. BH10

BH11 (0.0m to 45.3m)



DRAFT

	 West Quesnel Land Stability Program	CLIENT: CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	DWN BY: S.Ruiz CHKD BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	DRILL CORE PHOTOS BH11 (1 of 2) WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007 PROJECT NO.: KX04397 REV. NO.: A FIGURE No. BH11
				This drawing was originally produced in colour.	

BH11 (45.3m to 70.2m)



CLIENT: CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	OWN BY: S.Ruiz CHK'D BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	TITLE DRILL CORE PHOTOS BH11 (2 of 2) PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007
			PROJECT NO: KX04397
West Quesnel Land Stability Program	amec	REV. NO.: A	
		FIGURE No. BH11	

DRAFT

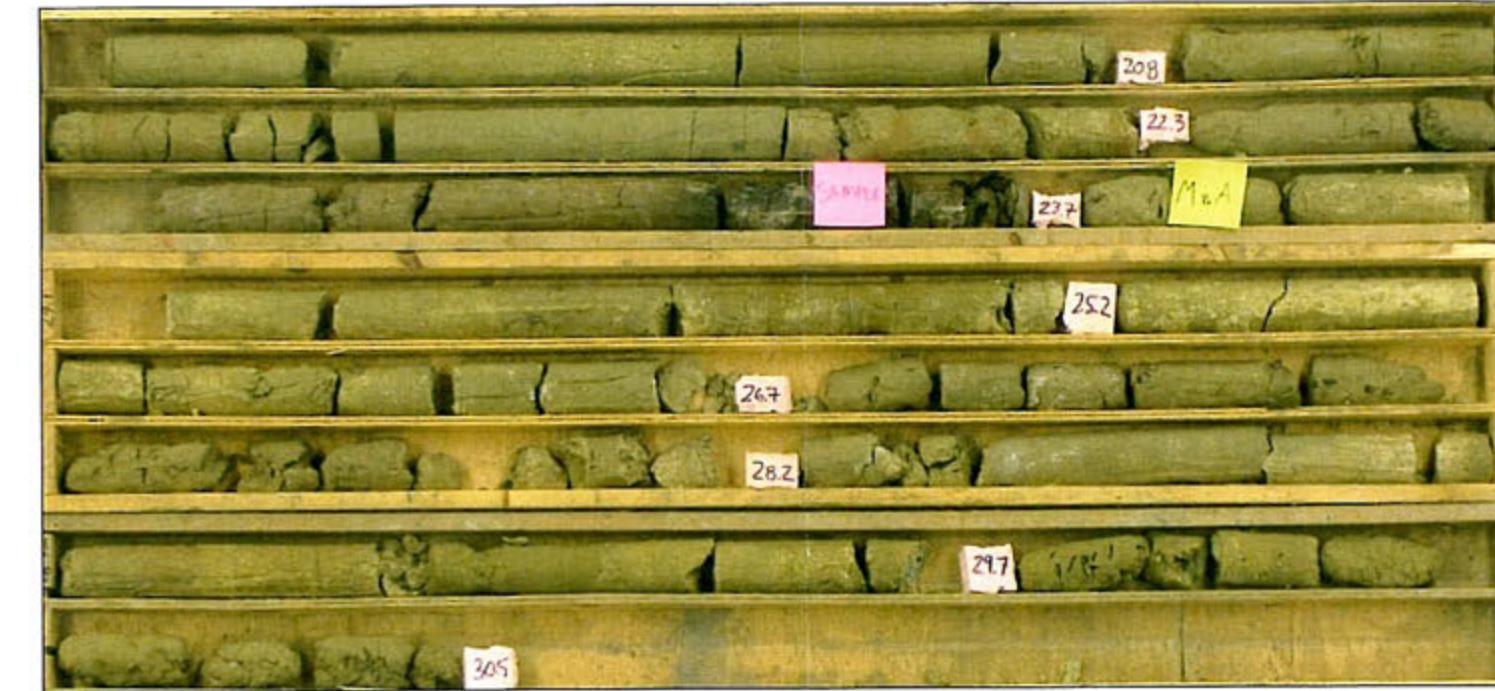
BH12 (0.0m to 70.2m)



	 West Quesnel Land Stability Program	CLIENT: CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	DWN BY: S.Ruiz CHK'D BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	TITLE DRILL CORE PHOTOS BH12 (1 of 1) PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	DATE: APRIL 2007				
					PROJECT NO: KX04397				
					REV. NO.: A				
					FIGURE No. BH12				

DRAFT

BH13 (0.0m to 30.5m)



CLIENT: CITY OF QUESNEL	AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045		DWN BY: S.Ruiz	TITLE DRILL CORE PHOTOS BH13 (1 of 1)	DATE: APRIL 2007
			CHK'D BY: S.Kelly		
DATUM: NAD83	PROJECT	PROJECT NO.: KX04397			
PROJECTION: UTM Zone 10	WEST QUESNEL LAND STABILITY STUDY	REV. NO.: A			
SCALE: NOT TO SCALE	QUESNEL, BC	FIGURE No. BH13			

DRAFT

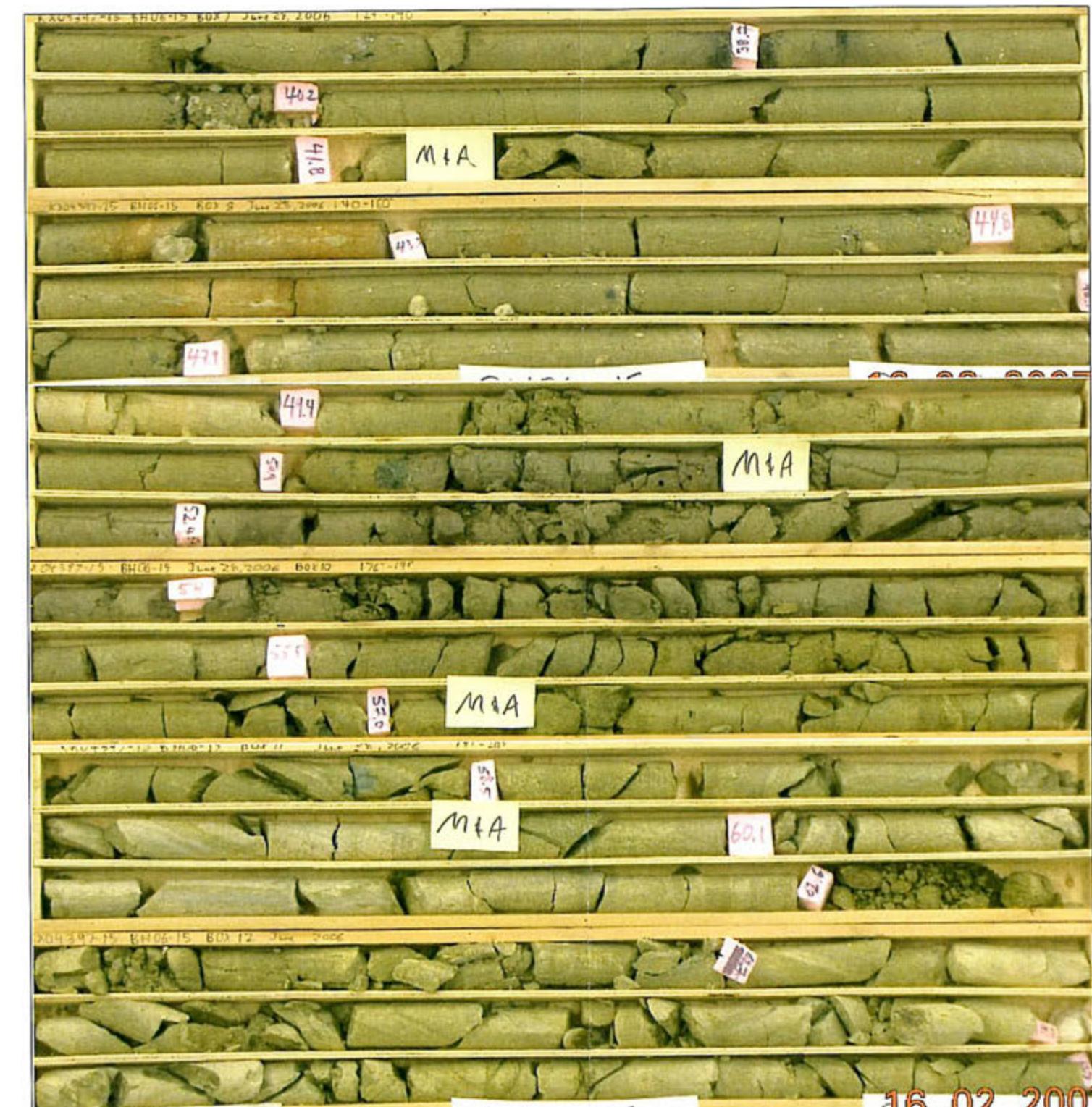
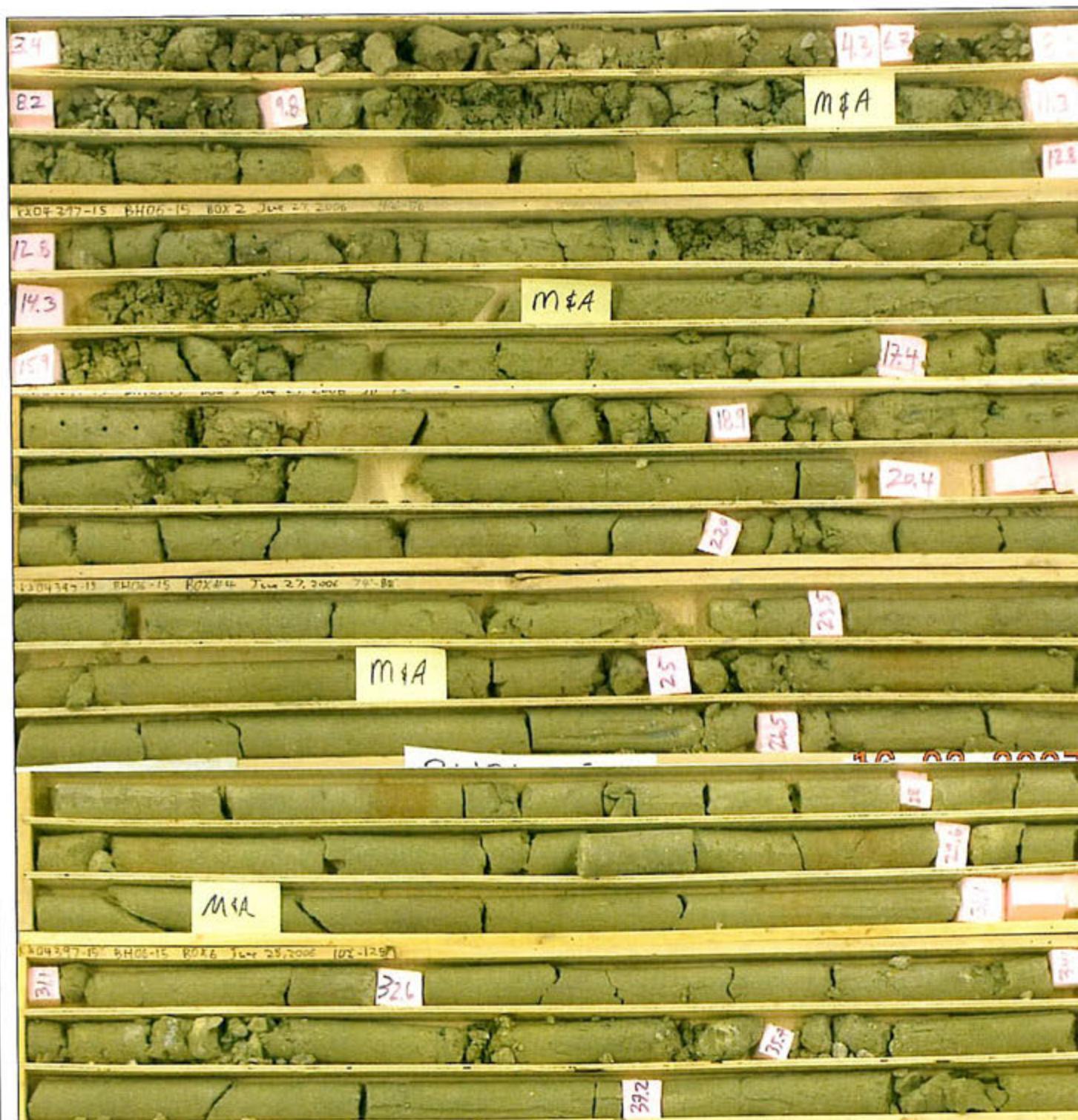
BH14 (0.0m to 100.8m)



CLIENT: CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	amec	DWN BY: S.Ruiz	TITLE DRILL CORE PHOTOS BH14 (1 of 1)	DATE: APRIL 2007
		CHK'D BY: S.Kelly		PROJECT NO: KX04397
		DATUM: NAD83	PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC	REV. NO.: A
		PROJECTION: UTM Zone 10		FIGURE No. BH14
		SCALE: NOT TO SCALE		
				This drawing was originally produced in colour.

DRAFT

BH15 (3.4m to 65.8m)



	 West Quesnel Land Stability Program	CLIENT:	CITY OF QUESNEL AMEC Earth & Environmental 3456 Opie Crescent Prince George, BC, CANADA V2N 2P9 Tel. (250) 564-3243 Fax (250) 562-7045	DWN BY: S.Ruiz CHK'D BY: S.Kelly DATUM: NAD83 PROJECTION: UTM Zone 10 SCALE: NOT TO SCALE	TITLE	DATE:				
						APRIL 2007				
			DRILL CORE PHOTOS BH15 (1 of 2) WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC			PROJECT NO: KX04397				
			PROJECT WEST QUESNEL LAND STABILITY STUDY QUESNEL, BC			REV. NO.: A				
						FIGURE No. BH15				

DRAFT

This drawing was originally produced in colour.

TABLE E1 – Atterberg Limit Index Testing Results

STRATIGRAPHIC UNIT	SAMPLING JUSTIFICATION	BOREHOLE ID	DEPTH (m)	ELEVATION (masl)	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	PLASTICITY INDEX (%)	MOISTURE CONTENT	USC DESIGNATION
Glaciolacustrine Silts and Clays	Soil/rock type	BH8	27.3	498.8	27	20	7	-	CL
	Soil/rock type	BH10	19.0	525.8	38	15	23	14	CI
	Soil/rock type	BH14	7.0	565.3	31	22	10	28	CI
	Soil/rock type	BH16	12.5	648.2	37	22	15	32	CI
	Soil/rock type	BH16	37.0	623.7	47	20	27	29	CI
	Soil/rock type	BH16	42.2	618.5	61	23	38	36	CH
	Soil/rock type	BH16	56.6	604.1	38	14	24	28	CI
Clay Tuff	Proximate to SI	BH8	31.2	484.9	38	14	24	49	CI
	Proximate to SI	BH10	19.9	524.9	109	39	70	46	CH
	Proximate to SI	BH11	19.1	511.8	98	35	63	33	CH
	Proximate to SI	BH11	27.7	503.2	93	38	55	32	CH
	Proximate to SI	BH11	51.9	479	87	33	54	31	CH
	Proximate to SI	BH12	62.3	494.1	73	23	50	39	CH
	Proximate to SI	BH13	11.0	471.0	97	31	66	27	CH
	Proximate to SI	BH13	17.3	464.7	77	24	52	23	CH
	Soft zone, Proximate to SI	BH8	56.0	470.1	117	41	76	112	CH
	Soft zone	BH7	53.4	488.0	69	35	34	40	MH
	Soft zone	BH10	32.5	512.3	115	38	77	58	CH
	Soft zone	BH10	49.4	495.4	61	16	45	25	CH
	Soft zone	BH15	11.0	554.2	77	36	41	42	MH
	Slickensided	BH14	32.8	539.5	94	35	59	40	CH
	Slickensided	BH14	43.5	528.8	65	31	34	28	CH
	Slickensided	BH14	45.5	526.8	83	37	46	39	CH
	Slickensided	BH15	14.8	550.4	70	30	40	38	CH
	Slickensided	BH15	42.0	523.2	70	28	42	32	CH
	Slickensided	BH15	57.2	508.0	89	40	50	46	MH
	Slickensided	BH16	64.5	506.2	67	27	41	28	CH
	Slickensided	BH16	60.1	580.6	67	25	42	23	CH
	Soil/rock type	BH2	48.1	436.5	65	45	20	35	MH
	Soil/rock type	BH2	55.7	428.9	108	51	57	36	MH
	Soil/rock type	BH2	58.8	425.8	98	33	65	37	CH
	Soil/rock type	BH2	69.4	415.2	83	49	34	31	MH
	Soil/rock type	BH2	77.0	407.6	76	49	27	33	MH
	Soil/rock type	BH3	35.2	468.0	35	20	15	13	CI
	Soil/rock type	BH3	41.0	462.2	70	49	21	43	MH
	Soil/rock type	BH3	48.0	455.2	59	43	16	31	MH
	Soil/rock type	BH3	55.7	447.5	66	48	18	35	MH
	Soil/rock type	BH3	67.8	435.4	75	49	26	34	MH
	Soil/rock type	BH3	81.4	521.8	85	45	40	38	MH
	Soil/rock type	BH3	89.2	414.0	93	52	41	31	MH
	Soil/rock type	BH4	46.8	489.6	54	29	25	27	CH
	Soil/rock type	BH4	49.4	487.0	53	29	29	33	CH
	Soil/rock type	BH4	66.4	470.0	105	49	56	14	MH
	Soil/rock type	BH4	84.7	451.7	92	42	50	12	MH
	Soil/rock type	BH4	107.6	428.8	68	41	27	16	MH
	Soil/rock type	BH6	27.0	529.9	73	39	34	31	MH
	Soil/rock type	BH6	32.8	524.1	60	39	21	27	MH
	Soil/rock type	BH9	51.3	483.4	76	34	42	48	CH
	Soil/rock type	BH9	54.9	479.8	70	30	39	65	CH
	Soil/rock type	BH12	33.0	523.4	77	34	44	31	CH
	Soil/rock type	BH13	23.8	458.2	78	31	48	34	CH
	Soil/rock type	BH14	29.7	542.6	95	46	48	48	MH
	Soil/rock type	BH15	24.7	540.5	85	33	53	44	CH
	Soil/rock type	BH15	29.8	535.4	69	25	43	31	CH
	Soil/rock type	BH15	51.4	513.8	66	37	30	36	MH
	Soil/rock type	BH15	59.7	505.5	90	41	50	41	MH
	Soil/rock type	BH15	72.5	492.7	89	48	41	58	MH
	Soil/rock type	BH16	74.5	586.2	100	40	60	36	CH
Sand and gravel tuff	Soil/rock type	BH8	65.4	460.7	70	27	43	43	CH
	Soil/rock type	BH14	87.5	484.8	109	54	54	55	MH
	Soil/rock type	BH16	113.1	547.6	60	23	36	23	CH
Siliceous White Clay Shale	Soil/rock type	BH15	83.0	482.2	23	16	7	13	CL
Phyllite	Soil/rock type	BH16	147.5	513.2	22	14	8	12	CL
Siltstone	Soil/rock type	BH4	130.5	405.9	113	47	66	14	MH
	Soil/rock type	BH4	137.4	399.0	26	21	5	14	CL

**Figure E1: Plasticity Chart for Soils Passing No. 40 Sieve
City of Quesnel, West Quesnel Slide
(by Borehole Location)**

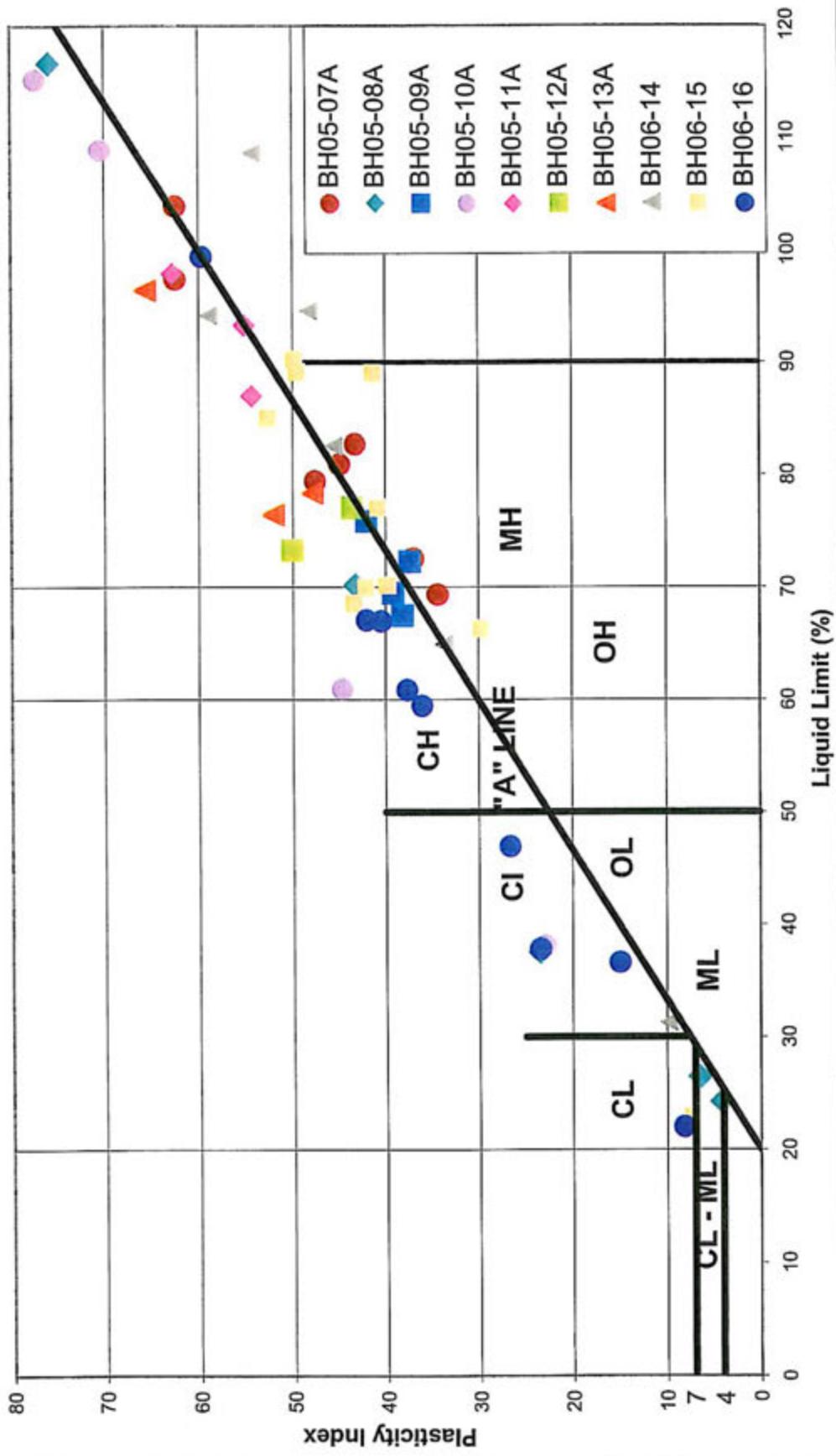


Figure E2: Plasticity Chart for Soils Passing No. 40 Sieve
 City of Quesnel, West Quesnel Slide
 (by Soil Type)

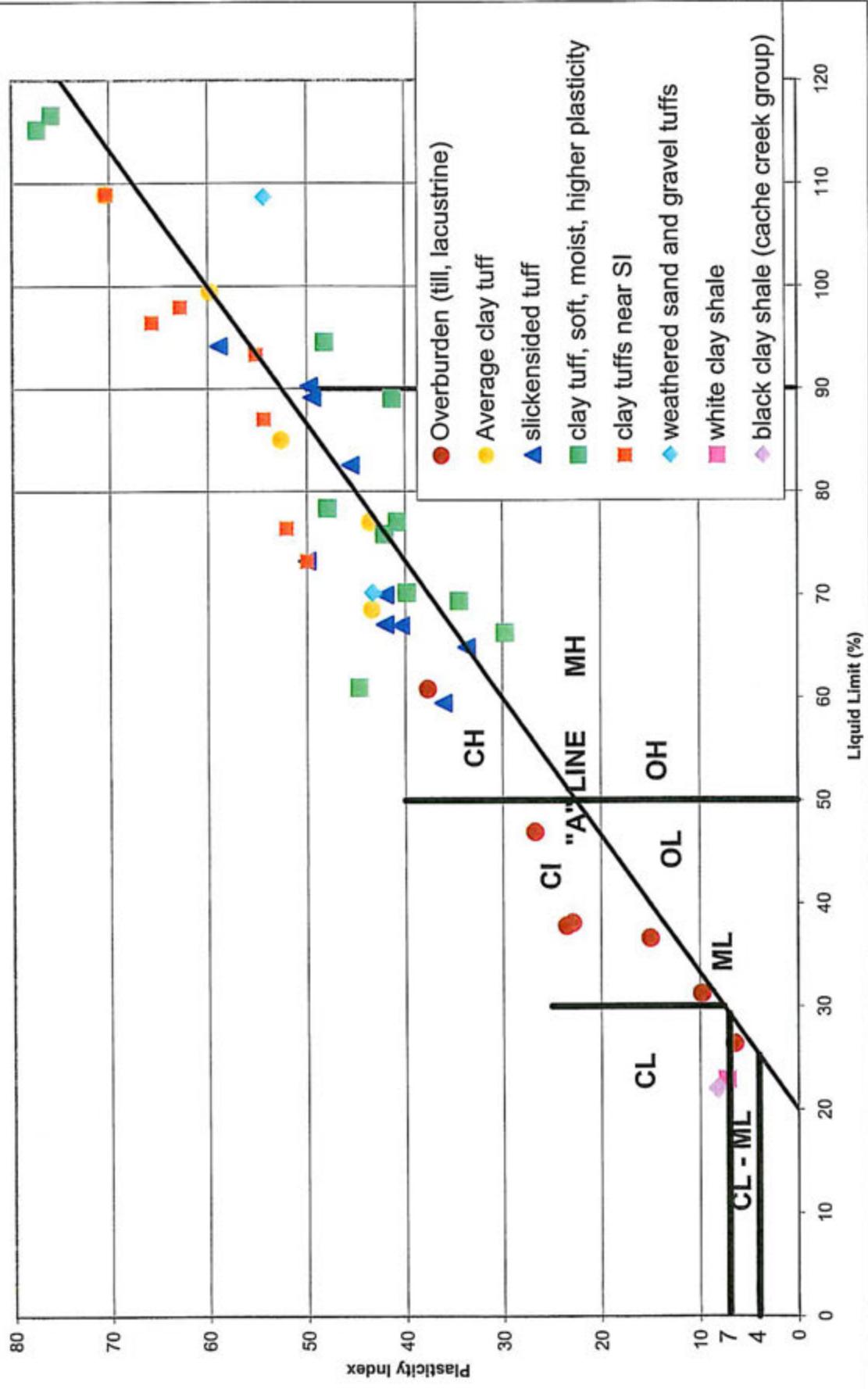
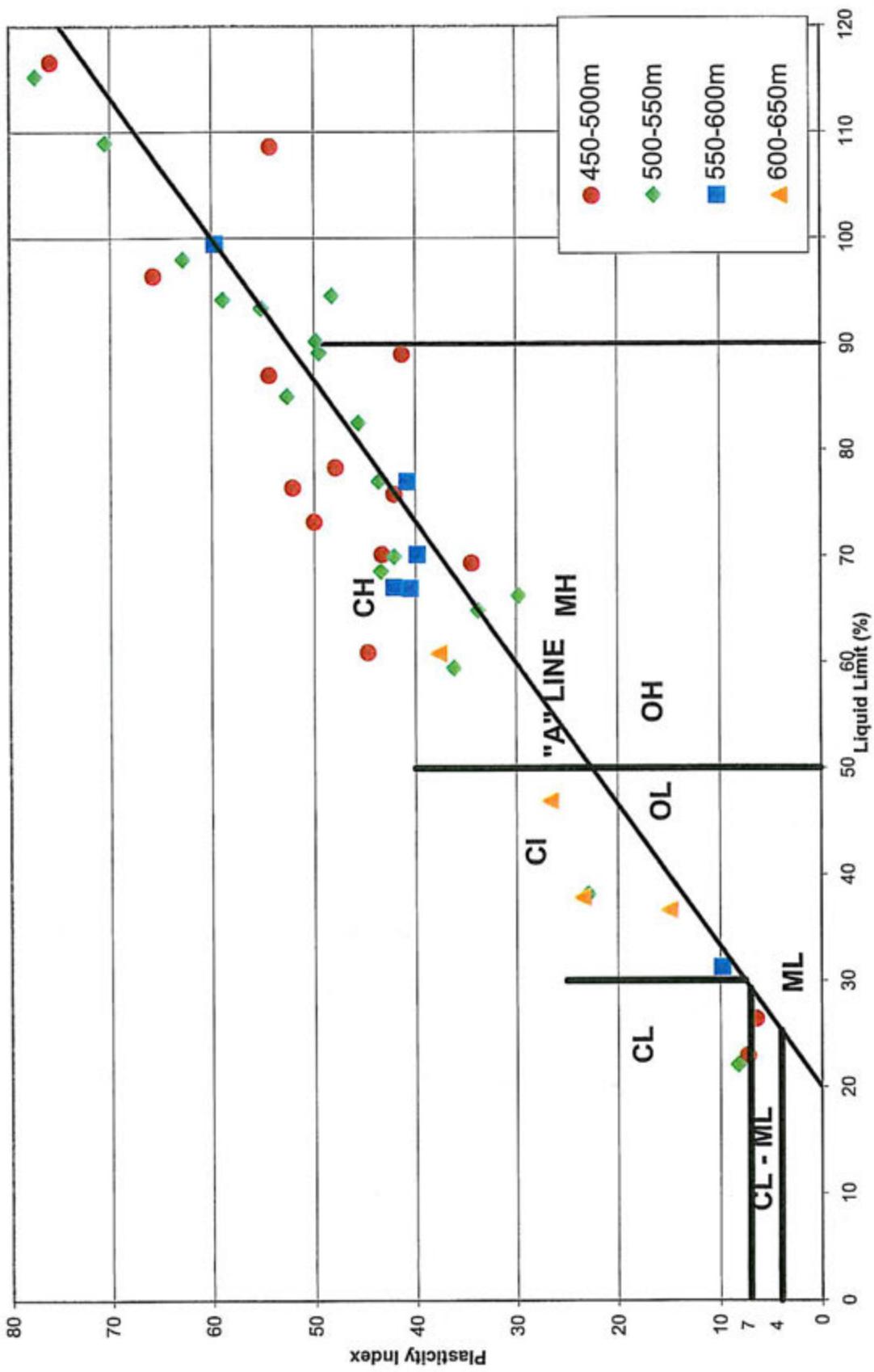


Figure E3: Plasticity Chart for Soils Passing No. 40 Sieve
City of Quesnel, West Quesnel Slide
(by Elevation)



**STANDPIPE PIEZOMETER /
OBSERVATION WELL INSTALLATION DETAIL**



Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 7 AUGUST 2003 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: BH2AB

Install Method / Driller: TH-60 - CARIBOO WATER WELLS Lock/Key No: _____

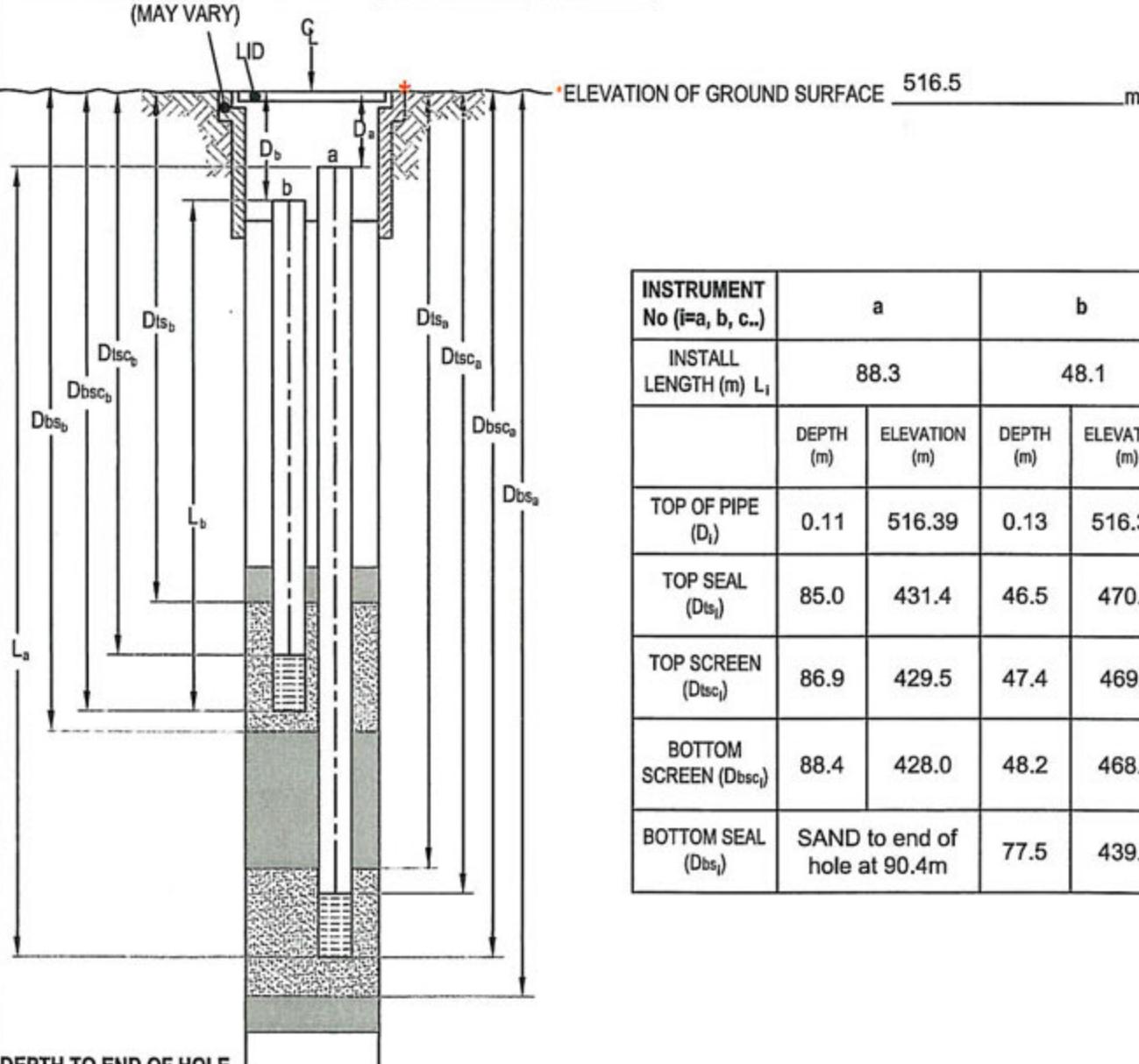
Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER

Tip Backfill: _____

NORTHING: 5869742.41

STEEL PROTECTIVE BOX (MAY VARY) EASTING: 532207.57



INSTRUMENT No (i=a, b, c..)	a		b	
INSTALL LENGTH (m) L _i	88.3		48.1	
	DEPTH (m)	ELEVATION (m)	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D _i)	0.11	516.39	0.13	516.37
TOP SEAL (D _{ls_i})	85.0	431.4	46.5	470.0
TOP SCREEN (D _{ls_i})	86.9	429.5	47.4	469.1
BOTTOM SCREEN (D _{ls_i})	88.4	428.0	48.2	468.3
BOTTOM SEAL (D _{bs_i})	SAND to end of hole at 90.4m		77.5	439.0

STANDPIPE PIEZOMETER / OBSERVATION WELL INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 8 AUGUST 2003 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: BH2C

Install Method / Driller: TH-60 - CARIBOO WATER WELLS Lock/Key No: _____

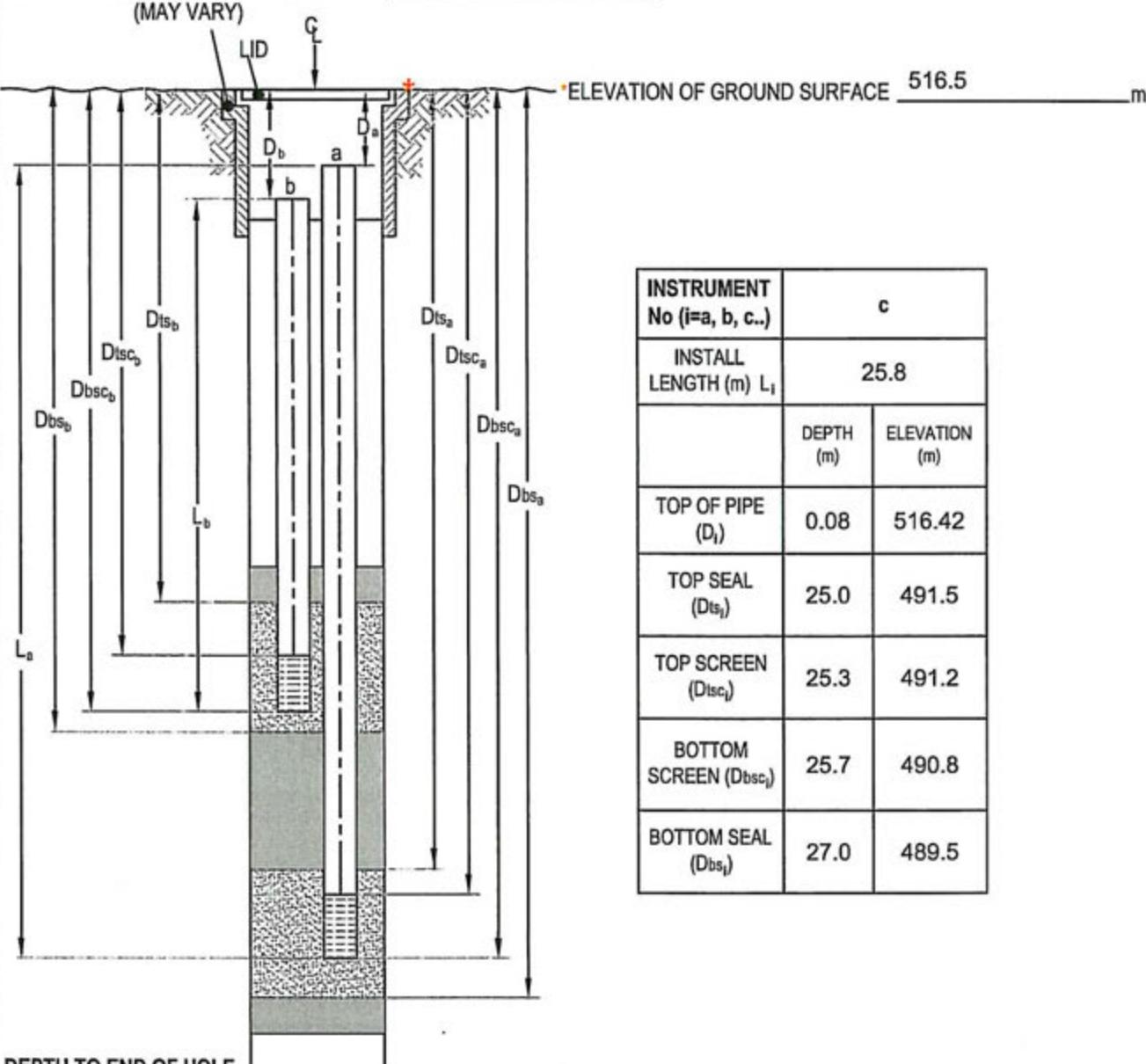
Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER

Tip Backfill: _____

NORTHING: 5869742.28

STEEL PROTECTIVE BOX (MAY VARY) EASTING: 532210.04



INSTRUMENT No (i=a, b, c..)	c	
INSTALL LENGTH (m) L _i	25.8	
	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D _i)	0.08	516.42
TOP SEAL (D _{ts_i})	25.0	491.5
TOP SCREEN (D _{sc_i})	25.3	491.2
BOTTOM SCREEN (D _{bsc_i})	25.7	490.8
BOTTOM SEAL (D _{bs_i})	27.0	489.5

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

STANDPIPE PIEZOMETER / OBSERVATION WELL INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 24 JULY 2003 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: BH3A

Install Method / Driller: SILVERADO - GEOTECH DRILLING Lock/Key No: _____

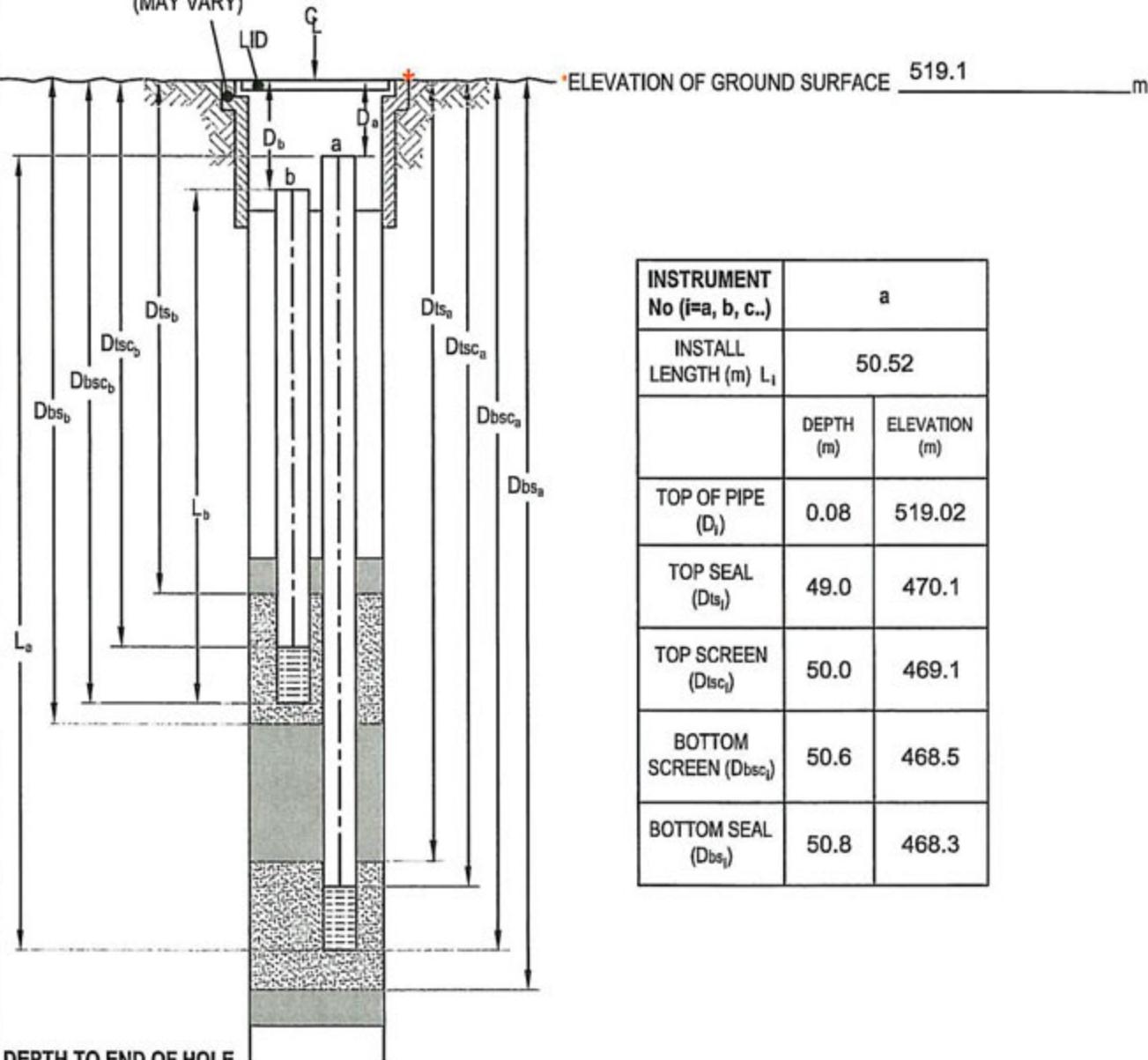
Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER

Tip Backfill: _____

NORTHING: 5869770.82

STEEL PROTECTIVE BOX
(MAY VARY) EASTING: 532006.53



INSTRUMENT No (i=a, b, c..)	a	
INSTALL LENGTH (m) L _i	50.52	
	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D _i)	0.08	519.02
TOP SEAL (Dts _i)	49.0	470.1
TOP SCREEN (Disc _i)	50.0	469.1
BOTTOM SCREEN (Dbsc _i)	50.6	468.5
BOTTOM SEAL (Dbs _i)	50.8	468.3

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SP-FRUSH

**STANDPIPE PIEZOMETER /
OBSERVATION WELL INSTALLATION DETAIL**

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 22 AUGUST 2003 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: BH3BC

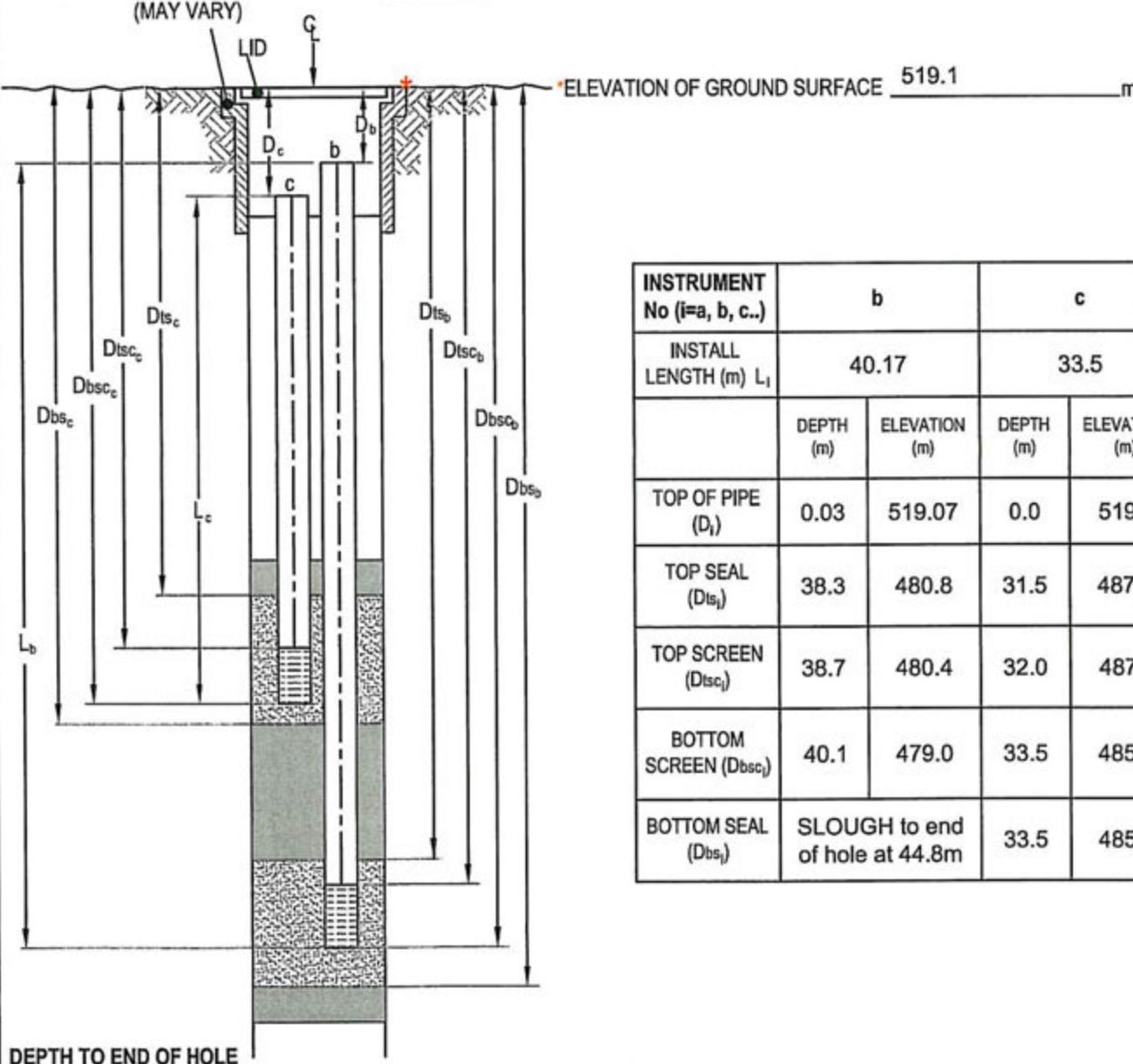
Install Method / Driller: SILVERADO - GEOTECH DRILLING Lock/Key No: _____

Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER Tip Backfill: _____

NORTHING: 5869770.43

STEEL PROTECTIVE BOX (MAY VARY) EASTING: 532005.22



INSTRUMENT No (i=a, b, c..)	b		c	
INSTALL LENGTH (m) L _i	40.17		33.5	
	DEPTH (m)	ELEVATION (m)	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D _i)	0.03	519.07	0.0	519.1
TOP SEAL (D _{tsi})	38.3	480.8	31.5	487.6
TOP SCREEN (Dtsc _i)	38.7	480.4	32.0	487.1
BOTTOM SCREEN (Dbsc _i)	40.1	479.0	33.5	485.6
BOTTOM SEAL (Dbsi)	SLOUGH to end of hole at 44.8m		33.5	485.6

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

STANDPIPE PIEZOMETER / OBSERVATION WELL INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 17 JULY 2003 Project No: KX04397

Supervised by AMEC: S.JORGENSEN Install'n No: BH4AB

Install Method / Driller: SILVERADO - GEOTECH DRILLING Lock/Key No: _____

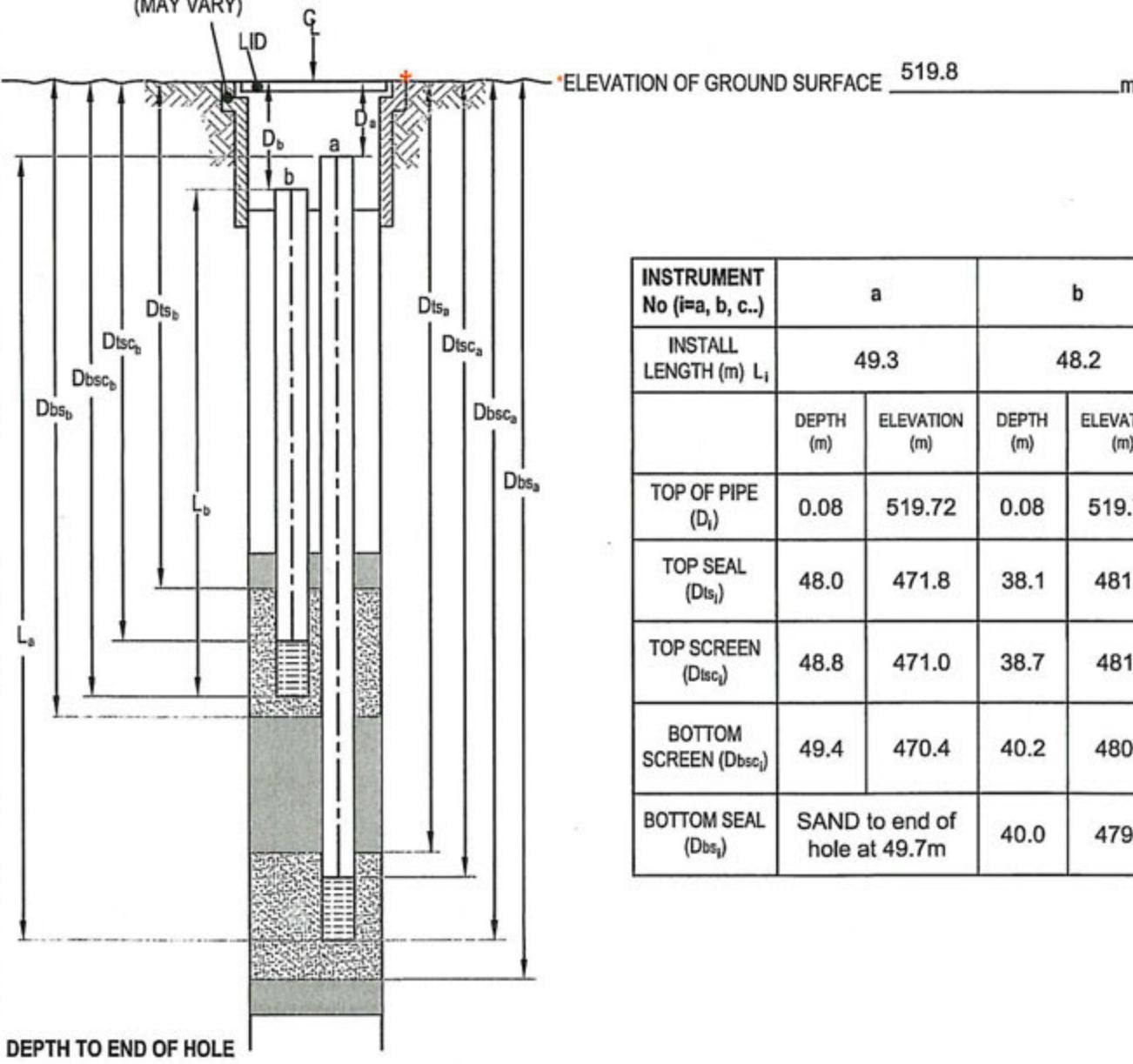
Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER

Tip Backfill: _____

NORTHING: 5869782.81

STEEL PROTECTIVE BOX (MAY VARY) EASTING: 532041.00



INSTRUMENT No (i=a, b, c..)	a		b	
INSTALL LENGTH (m) L_i	49.3		48.2	
	DEPTH (m)	ELEVATION (m)	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D_i)	0.08	519.72	0.08	519.72
TOP SEAL (D_{ts_i})	48.0	471.8	38.1	481.7
TOP SCREEN (D_{ts_i})	48.8	471.0	38.7	481.1
BOTTOM SCREEN (D_{bsc_i})	49.4	470.4	40.2	480.4
BOTTOM SEAL (D_{bs_i})	SAND to end of hole at 49.7m		40.0	479.8

DEPTH TO END OF HOLE

49.7

m

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SP-FLUSH

**STANDPIPE PIEZOMETER /
OBSERVATION WELL INSTALLATION DETAIL**

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 18 JULY 2003 Project No: KX04397

Supervised by AMEC: S.JORGENSEN Install'n No: BH4C

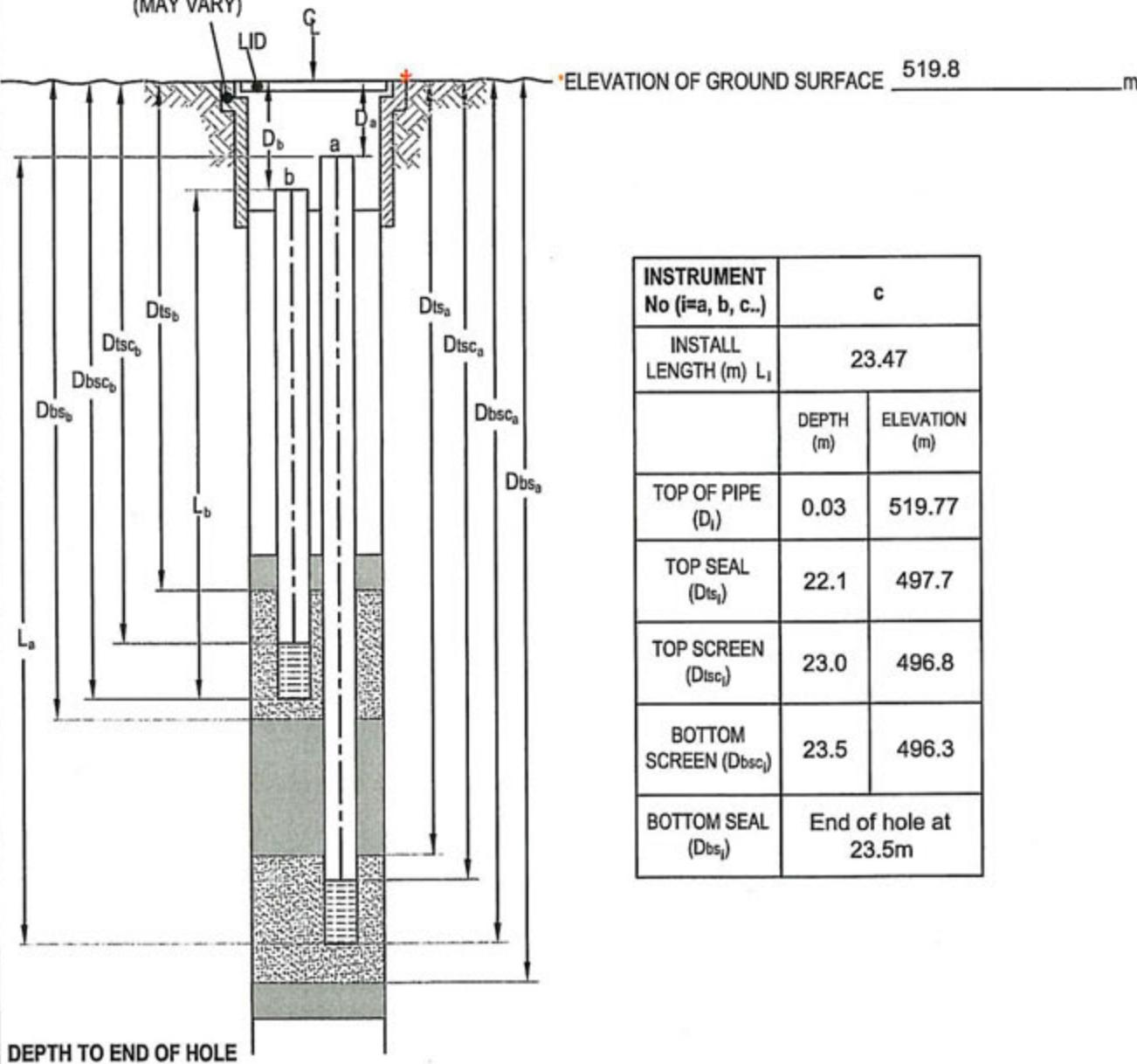
Install Method / Driller: SILVERADO - GEOTECH DRILLING Lock/Key No: _____

Screen / Tip Type: SLOTTED Pipe Diameter: 25mm Seal Mixture: _____

ROAD BOX / FLUSH MOUNT COVER Tip Backfill: _____

NORTHING: 5869783.48

STEEL PROTECTIVE BOX (MAY VARY) EASTING: 532042.93



INSTRUMENT No (i=a, b, c..)	c	
INSTALL LENGTH (m) L _i	23.47	
	DEPTH (m)	ELEVATION (m)
TOP OF PIPE (D _i)	0.03	519.77
TOP SEAL (D _{ts_i})	22.1	497.7
TOP SCREEN (D _{sc_i})	23.0	496.8
BOTTOM SCREEN (D _{bs_i})	23.5	496.3
BOTTOM SEAL (D _{bs_i})	End of hole at 23.5m	

STANDPIPE PIEZOMETER / OBSERVATION WELL INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 2 NOVEMBER 2001 Project No: KX04397

Supervised by AMEC: _____ Install'n No: BH2

Install Method / Driller: B53 MOBILE - GEOTECH DRILLING Lock/Key No: _____

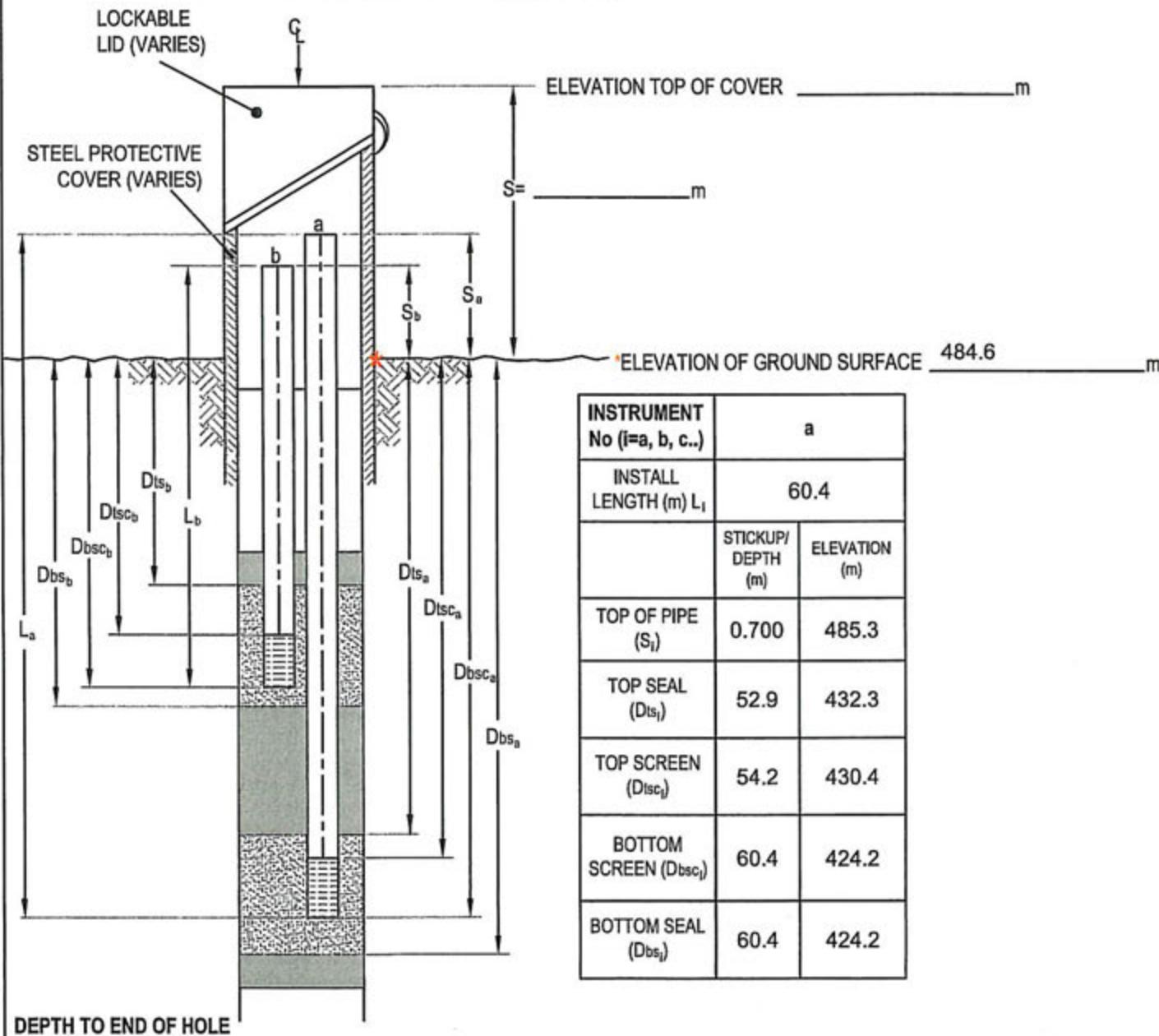
Screen / Tip Type: _____ Pipe Diameter: 25mm Seal Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5869340

Tip Backfill: PEA GRAVEL

EASTING: 532480



VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

15 NOVEMBER 2001

Project No:

KX04397

Supervised by AMEC:

H.ARABSHAH

Install'n No:

BH3

Install Method / Driller:

HQ/B-53 - GEOTECH DRILLING

Lock/Key No:

VW Make/Model:

SINCO

Data Logger:

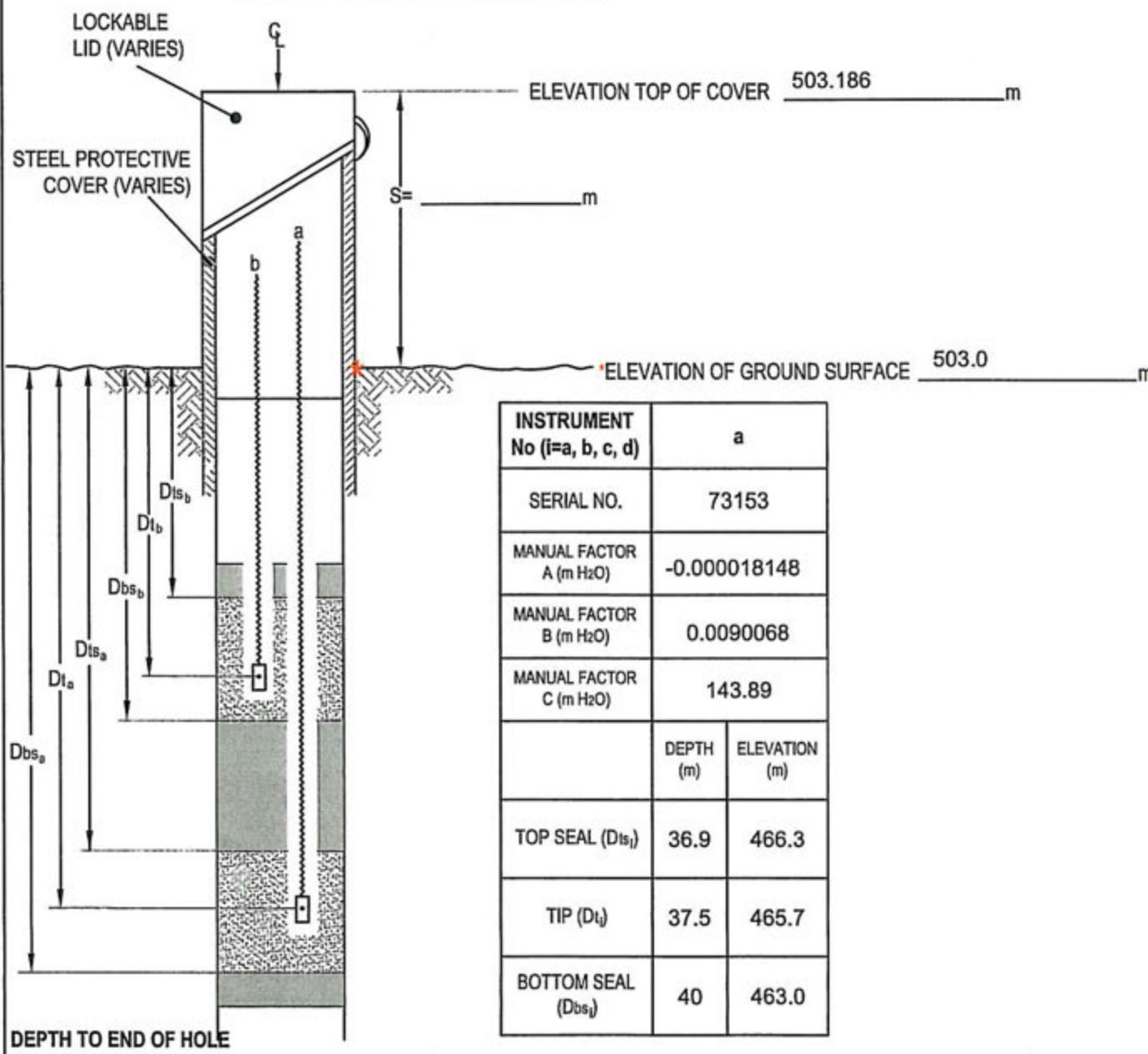
Seal Mixture:

STICK-UP CASING PROTECTOR:

Tip Backfill: SAND

NORTHING: 5869597

EASTING: 532264



DEPTH TO END OF HOLE

95.1

m

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VW-STICKUP

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

15 NOVEMBER 2001

Project No: KX04397

Supervised by AMEC:

D.DEWAR

Install'n No: BH4

Install Method / Driller:

B-53 - GEOTECH DRILLING

Lock/Key No:

VW Make/Model:

Data Logger: MINILOGGER

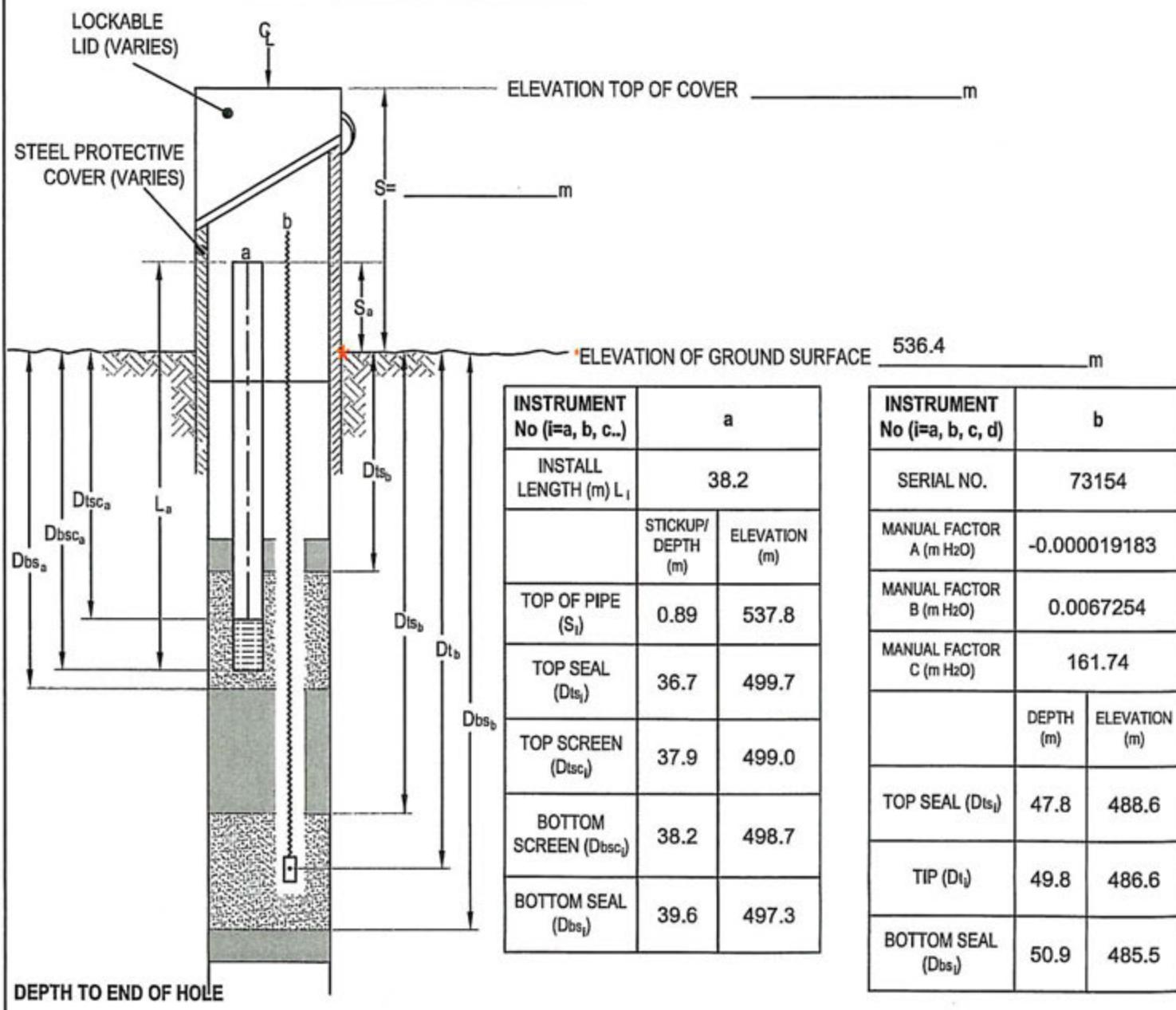
Seal Mixture:

STICK-UP CASING PROTECTOR:

Tip Backfill: PEA GRAVEL

NORTHING: 5869868

EASTING: 531829



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

STANDPIPE PIEZOMETER / OBSERVATION WELL INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 18 DECEMBER 2001 Project No: KX04397

Supervised by AMEC: D.DEWAR Install'n No: BH6

Install Method / Driller: B53 MOBILE - GEOTECH DRILLING Lock/Key No: _____

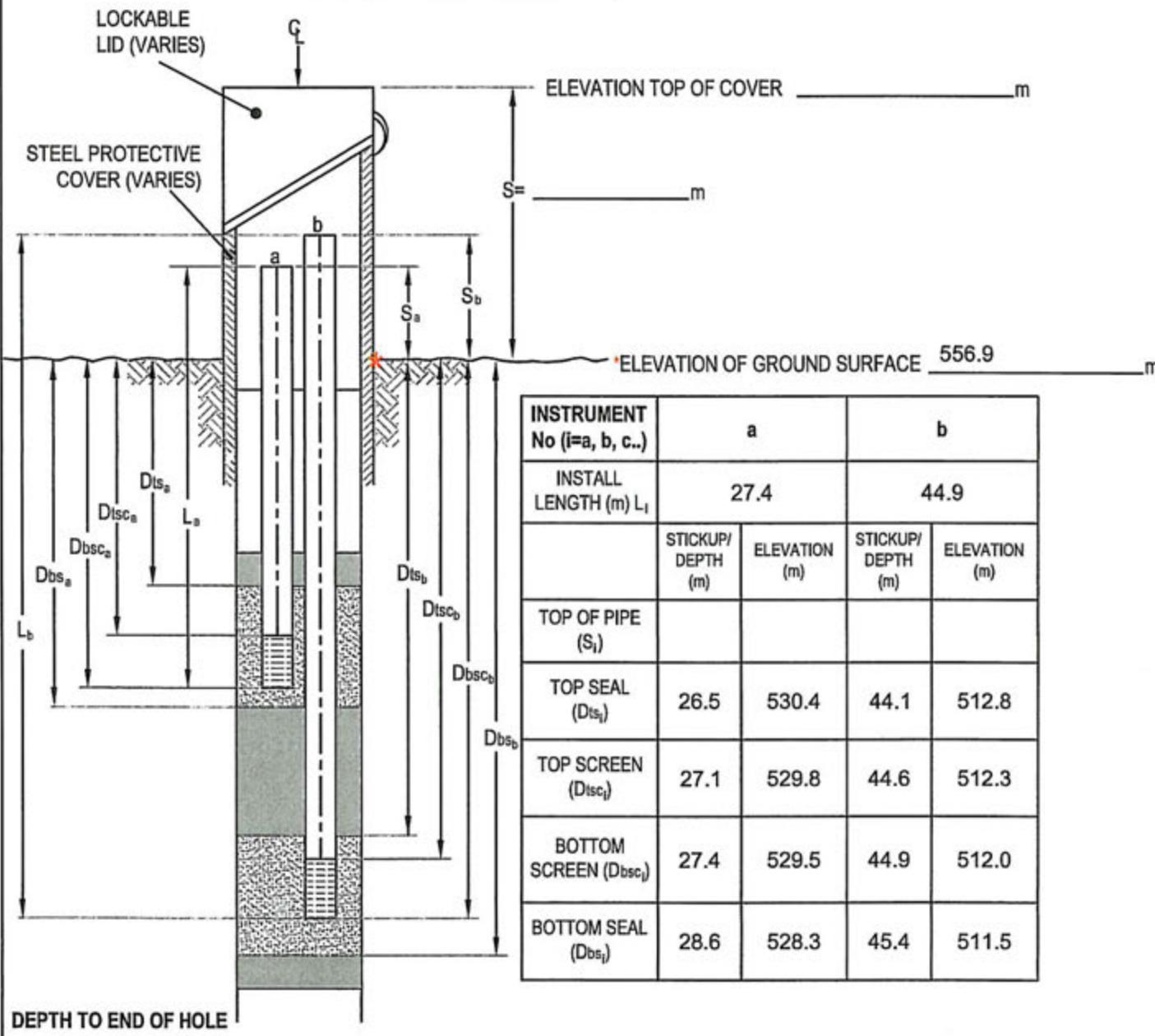
Screen / Tip Type: SLOTTED Pipe Diameter: 30mm Seal Mixture: BENTONITE

STICK-UP CASING PROTECTOR:

NORTHING: 5869940

Tip Backfill: _____

EASTING: 531514



DEPTH TO END OF HOLE

45.9
m

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SP-STICKUP

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 2 JULY 2005

Project No: KX04397

Supervised by AMEC: S.CARLSON / B.GAUSS

Install'n No: BH7

Install Method / Driller: GEOTECH DRILLING SERVICES

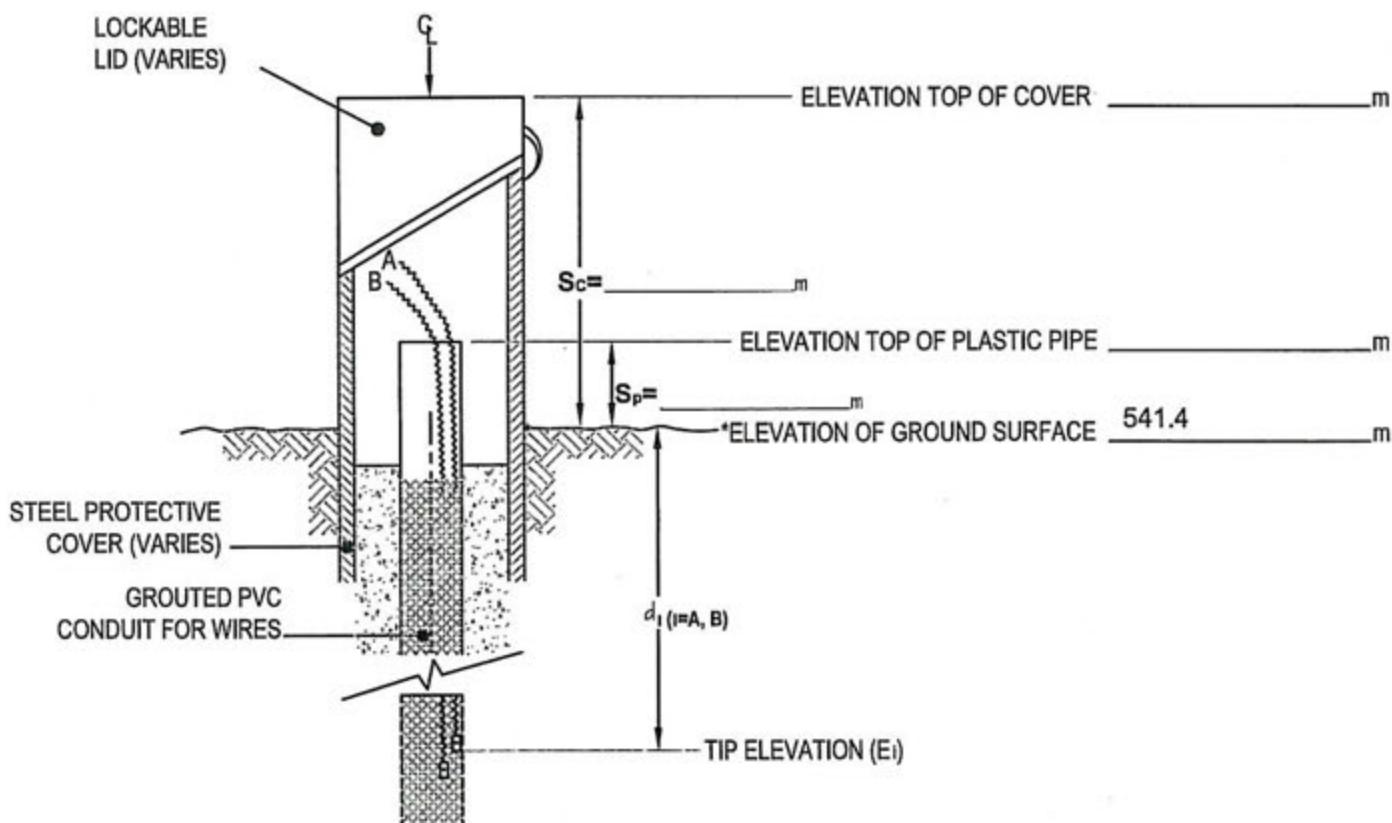
Lock/Key No: _____

VW Make / Type: SINCO Grout Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5870225.57

EASTING: 531798.95



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_1	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	27.0	514.4	82515	-0.000011997	-0.00071701	114.14
B	64.9	476.5	82505	-0.000010485	-0.0020117	107.11

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 15 JUNE 2005

Project No: KX04397

Supervised by AMEC: S.CARLSON / N.EKMAN

Install'n No: BH8

Install Method / Driller: GEOTECH DRILLING SERVICES

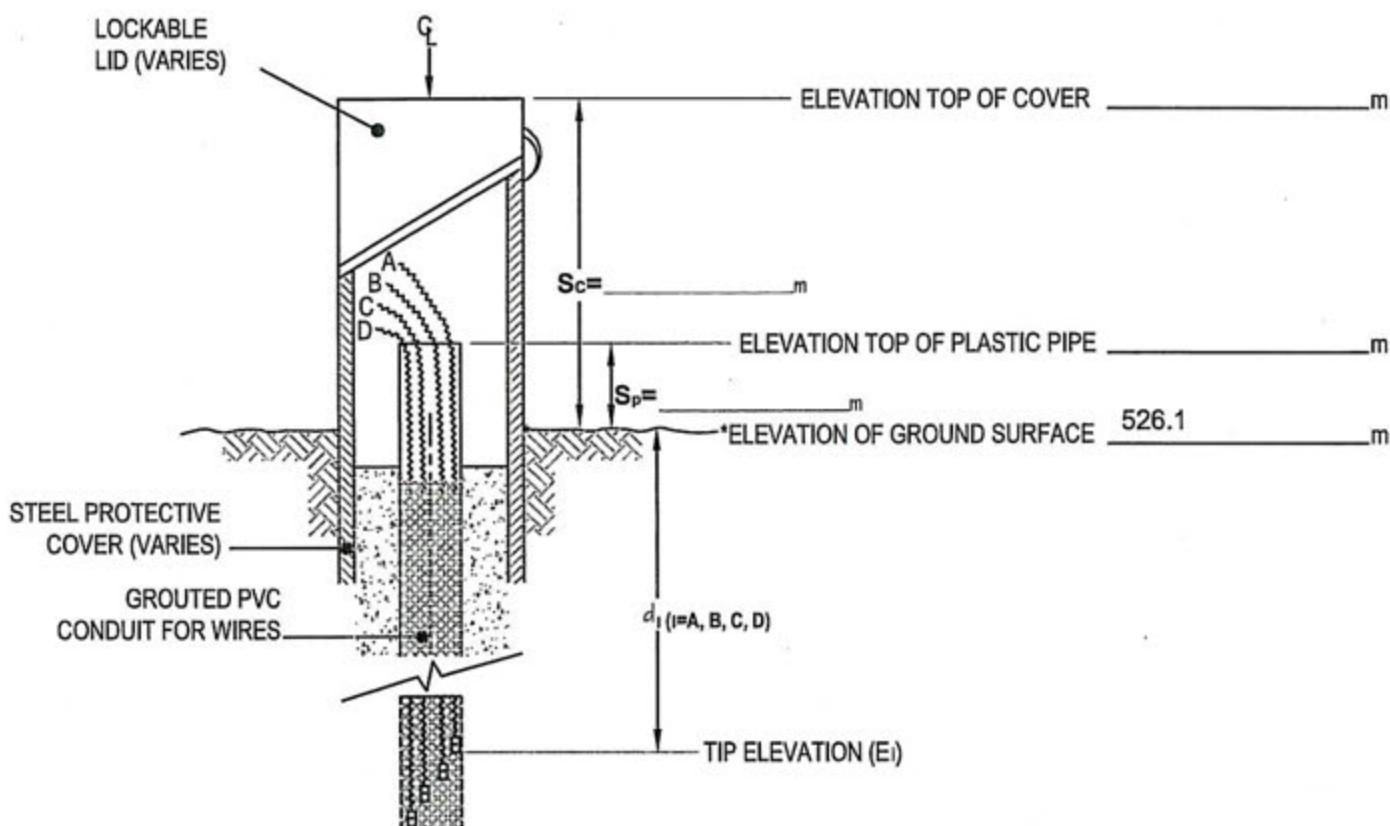
Lock/Key No: _____

VW Make / Type: SINCO Grout Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5870042.20

EASTING: 532161.22



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	13.1	513.0	86207	-0.0000092187	-0.000086190	74.336
B	13.0	513.1	82508	-0.000010436	-0.0037525	102.54
C	28.0	498.1	82507	-0.0000094174	-0.0068098	105.80
D	56.2	469.9	82501	-0.000012001	-0.0034358	102.03

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

29 MAY 2006

Project No:

KX04397

Supervised by AMEC:

S.CARLSON / R.GUSTAFSON

Install'n No:

BH9

Install Method / Driller:

GEOTECH DRILLING SERVICES

Lock/Key No:

VW Make / Type:

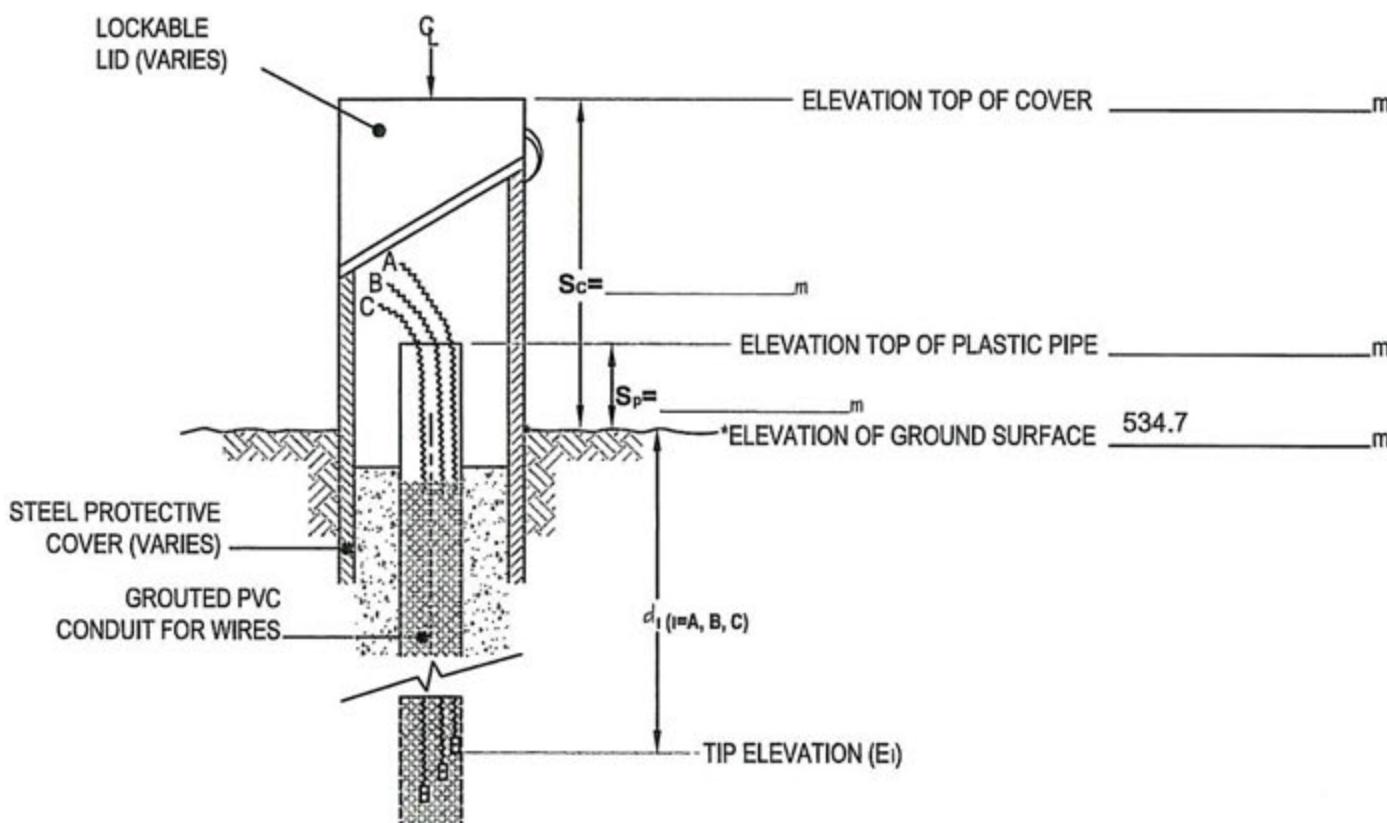
SINCO

Grout Mixture:

STICK-UP CASING PROTECTOR:

NORTHING: 5870120.36

EASTING: 531883.33



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	15.1	519.6	86206	-0.0000089271	-0.0031765	91.884
B	30.6	504.1	82510	-0.000012535	0.00089219	111.86
C	50.1	484.6	86210	-0.000015490	-0.0064010	149.72

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 5 JULY 2005 Project No: KX04397

Supervised by AMEC: N.EKMAN Install'n No: BH10

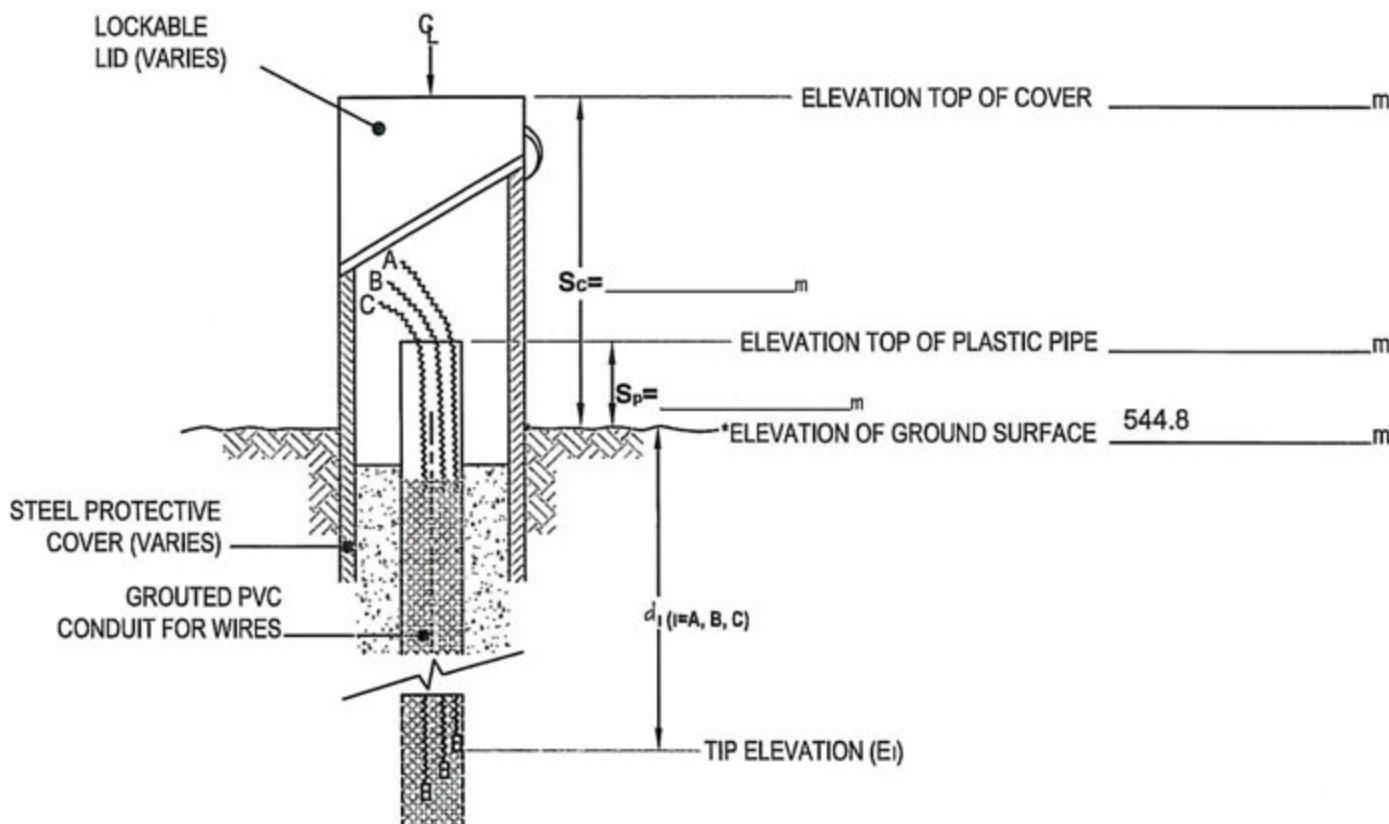
Install Method / Driller: GEOTECH DRILLING SERVICES Lock/Key No: _____

VW Make / Type: SINCO Grout Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5869655.38

EASTING: 531591.83



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	11.6	533.2	82758	-0.000012935	0.0018914	115.81
B	20.4	524.4	82511	-0.000010929	-0.0021866	105.81
C	48.6	496.2	82504	-0.000012802	0.000056544	122.96

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

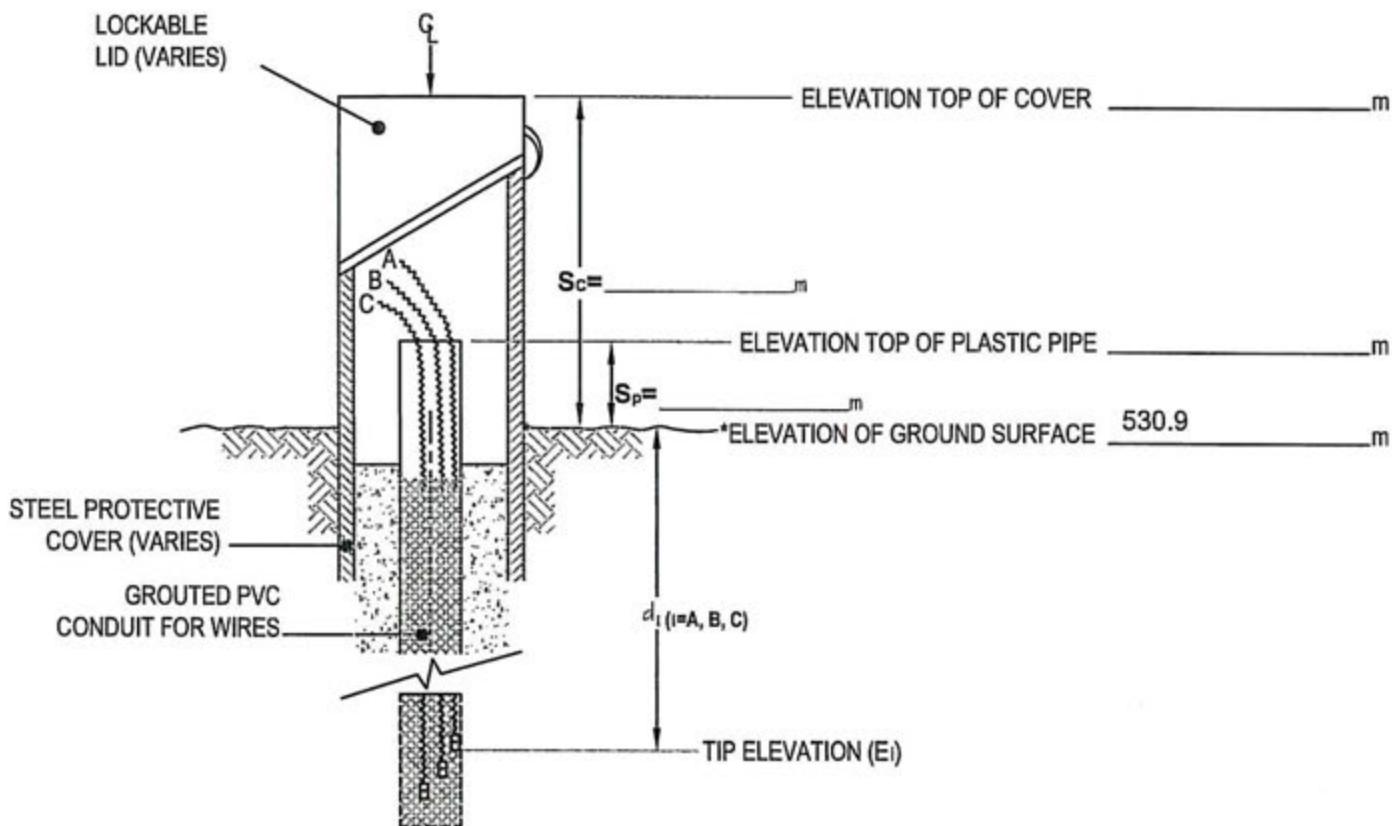


Project Name: WEST QUESNEL SLOPE STABILITY
 Date (s) Installed: 10 JUNE 2005 Project No: KX04397
 Supervised by AMEC: S.CARLSON / N.EKMAN Install'n No: BH11
 Install Method / Driller: GEOTECH DRILLING SERVICES Lock/Key No:
 VW Make / Type: SINCO Grout Mixture:

STICK-UP CASING PROTECTOR:

NORTHING: 5870360.16

EASTING: 532036.58



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	14.3	516.6	82513	-0.000010701	0.0023936	80.96
B	32.0	498.9	82514	-0.000012057	0.0025357	111.36
C	50.3	480.6	82502	-0.000012722	0.0038803	111.73

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

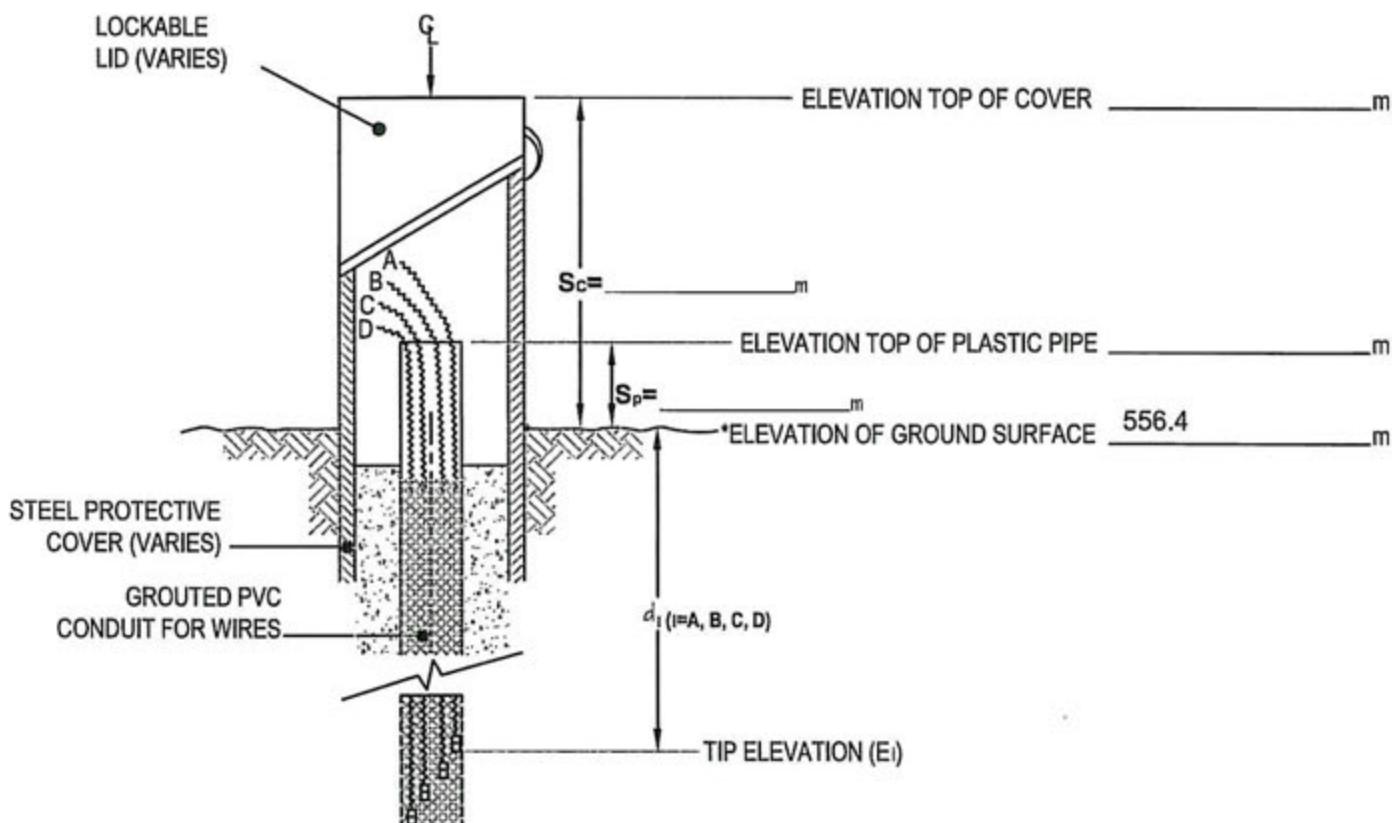


Project Name: WEST QUESNEL SLOPE STABILITY
 Date (s) Installed: 21 JUNE 2005 Project No: KX04397
 Supervised by AMEC: S.CARLSON / N.EKMAN Install'n No: BH12
 Install Method / Driller: GEOTECH DRILLING SERVICES Lock/Key No:
 VW Make / Type: SINCO Grout Mixture:

STICK-UP CASING PROTECTOR:

NORTHING: 5870271.58

EASTING: 531620.09



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	18.0	538.4	82760	-0.000012275	-0.0012922	118.28
B	22.9	533.5	82517	-0.000012605	-0.0028334	135.80
C	26.5	529.9	82516	-0.000012238	0.00036981	115.34
D	54.9	501.5	82506	-0.000012076	0.00035765	117.45

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 8 JULY 2005 Project No: KX04397

Supervised by AMEC: N.EKMAN Install'n No: BH13

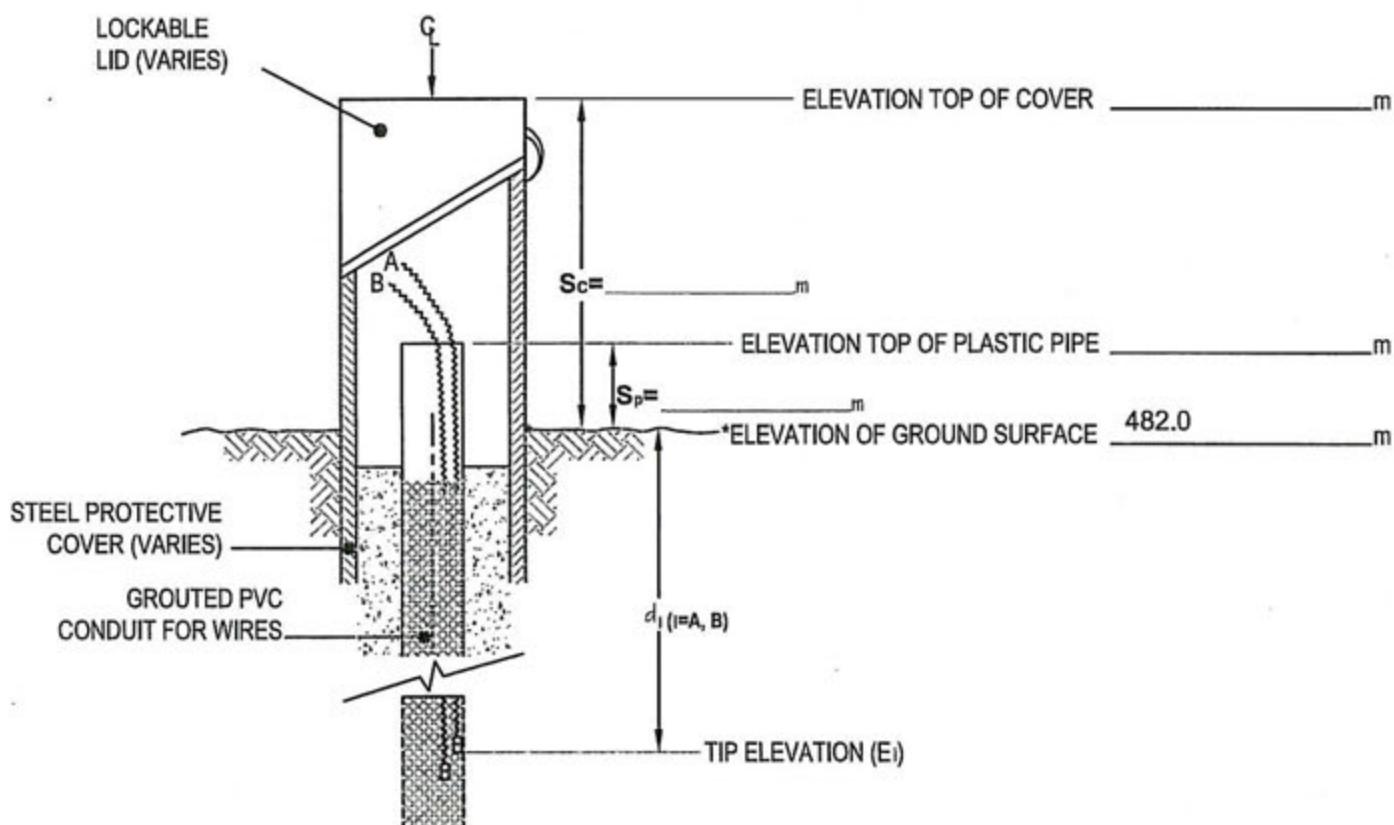
Install Method / Driller: GEOTECH DRILLING SERVICES Lock/Key No: _____

VW Make / Type: SINCO Grout Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5869941.11

EASTING: 532460.93



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	5.1	476.9	82759	-0.000011740	-0.0049154	128.58
B	14.4	467.6	82512	-0.000013035	0.0021625	100.42

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 23 JUNE 2006

Project No: KX04397

Supervised by AMEC: R.GUSTAFSON

Install'n No: BH14

Install Method / Driller: GEOTECH DRILLING SERVICES

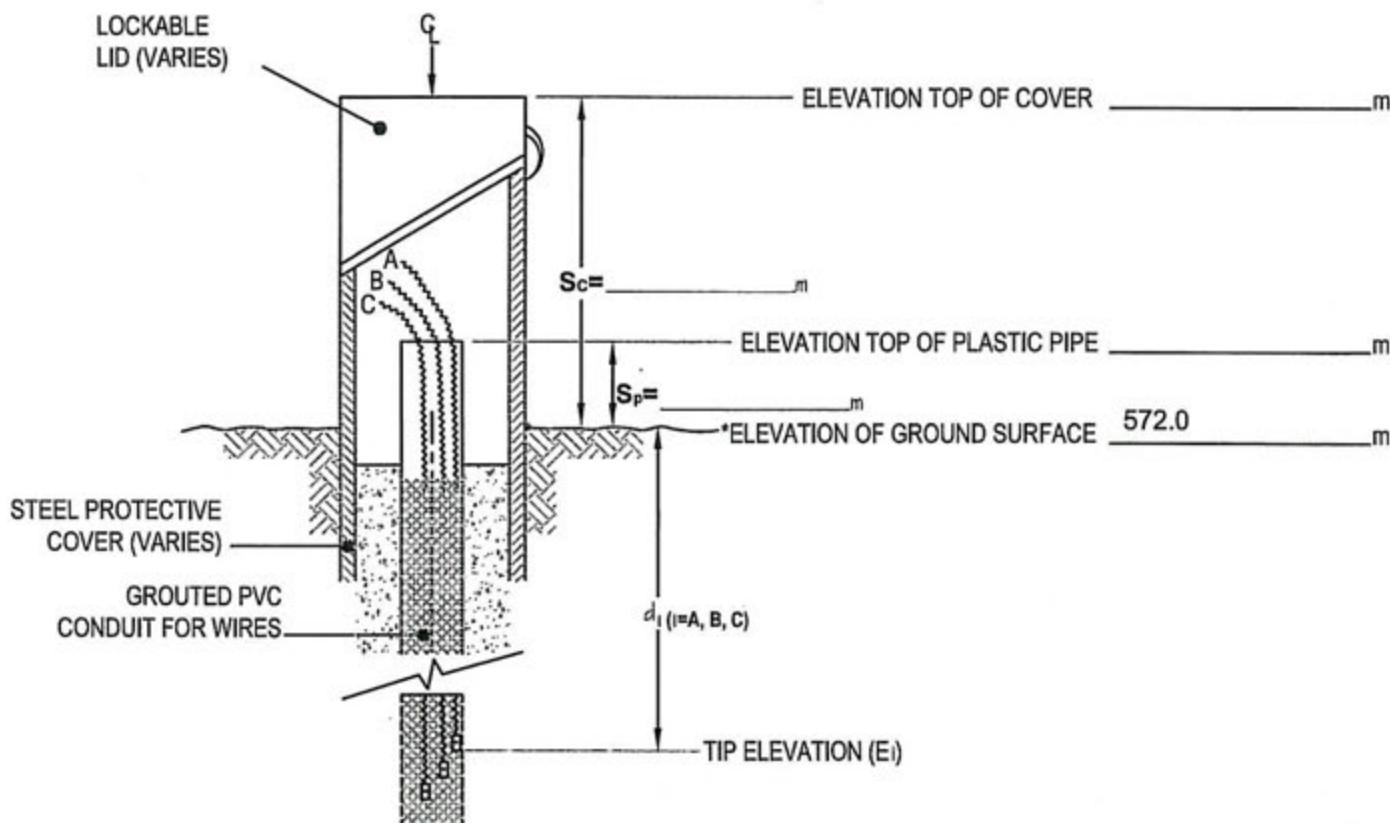
Lock/Key No: _____

VW Make / Type: SINCO Grout Mixture: _____

STICK-UP CASING PROTECTOR:

NORTHING: 5869692.84

EASTING: 531276.21



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	20.1	551.9	86208	-0.0000083033	-0.0050388	84.487
B	65.9	506.1	86211	-0.000016544	-0.0019229	159.14
C	98.7	473.3	86213	-0.000027912	-0.026608	356.17

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

19 JULY 2006

Project No:

KX04397

Supervised by AMEC:

R.GUSTAFSON

Install'n No:

BH15

Install Method / Driller:

GEOTECH DRILLING SERVICES

Lock/Key No:

VW Make / Type:

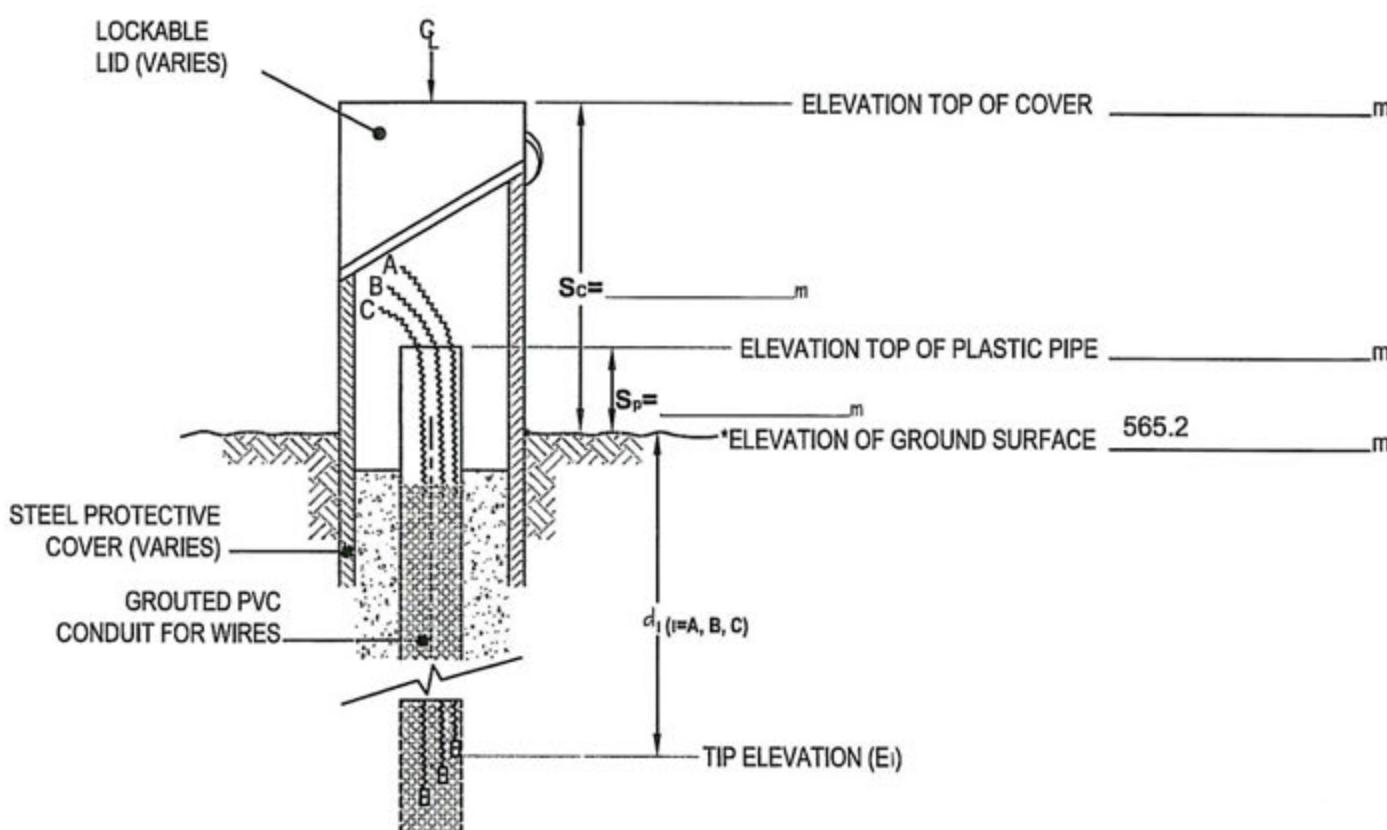
SINCO

Grout Mixture:

STICK-UP CASING PROTECTOR:

NORTHING: 5870278.18

EASTING: 531417.20



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	8.8	556.4	86209	-0.0000097174	0.0010648	80.114
B	63.3	501.9	86212	0.000017528	-0.00046492	161.27
C	83.2	482.0	86214	-0.000029246	-0.012026	297.22

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

VIBRATING WIRE PIEZOMETER INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 23 OCTOBER 2006 Project No: KX04397

Supervised by AMEC: S.CARLSON / N.EKMAN Install'n No: BH16

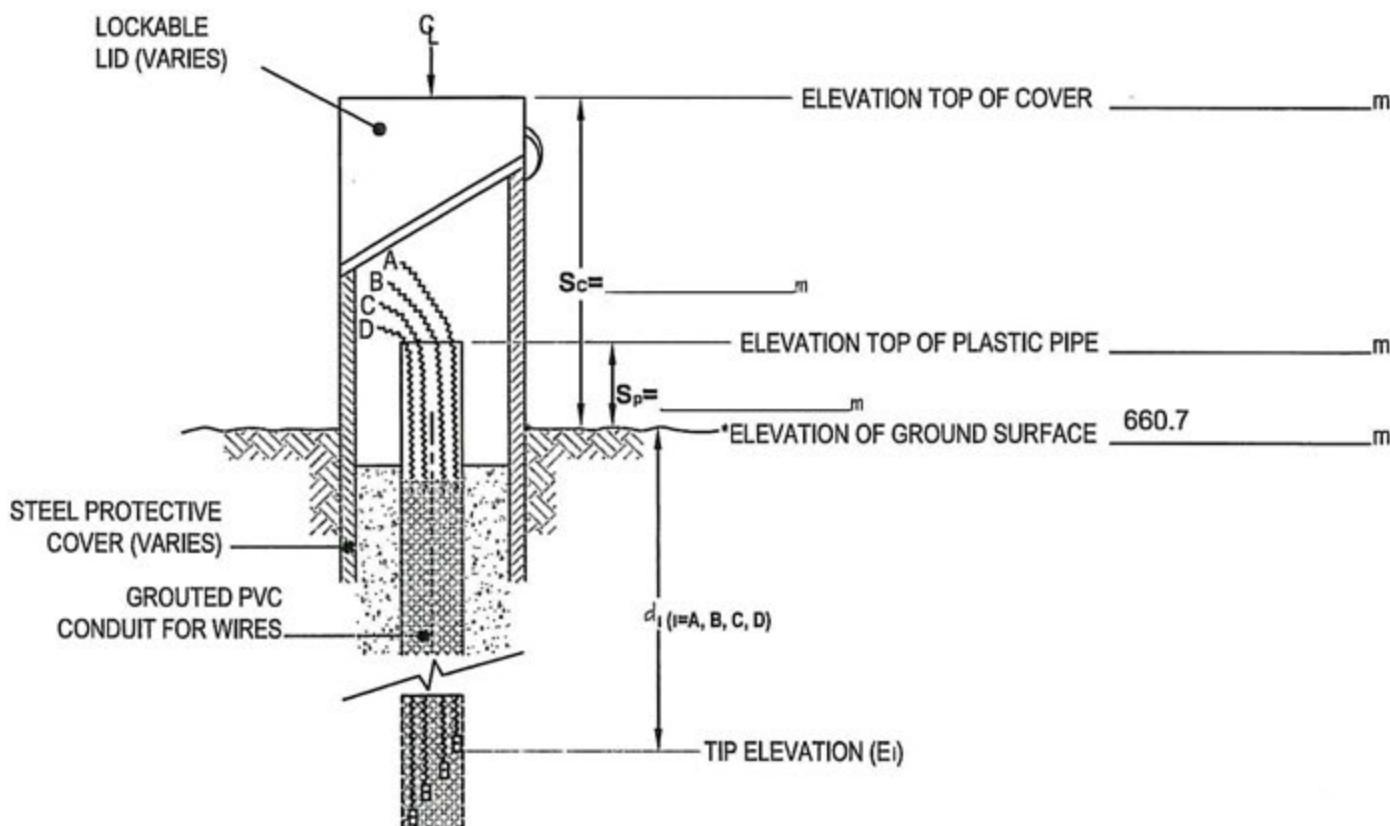
Install Method / Driller: GEOTECH DRILLING SERVICES Lock/Key No:

VW Make / Type: SINCO Grout Mixture:

STICK-UP CASING PROTECTOR:

NORTHING: 5869544.24

EASTING: 530634.61



INSTRUMENT NO.	INSTALLED TIP DEPTH (m) d_i	TIP ELEVATION (m) E_i	SERIAL NO.	MANUAL FACTOR A (m H ₂ O)	MANUAL FACTOR B (m H ₂ O)	MANUAL FACTOR C (m H ₂ O)
A	42.7	618.0	82509	-0.000010267	-0.000085463	97.557
B	85.5	575.2	86215	-0.000029148	-0.026148	332.81
C	139.1	521.6	86216	-0.000029390	-0.028715	340.86
D	214.9	445.8	86217	-0.000092198	0.032651	671.88

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 25 OCTOBER 2000 Project No: KX04397

Supervised by AMEC: D.DEWAR Install'n No: SI1

Install Method / Driller: CARIBOU WATER WELLS Lock/Key No: _____

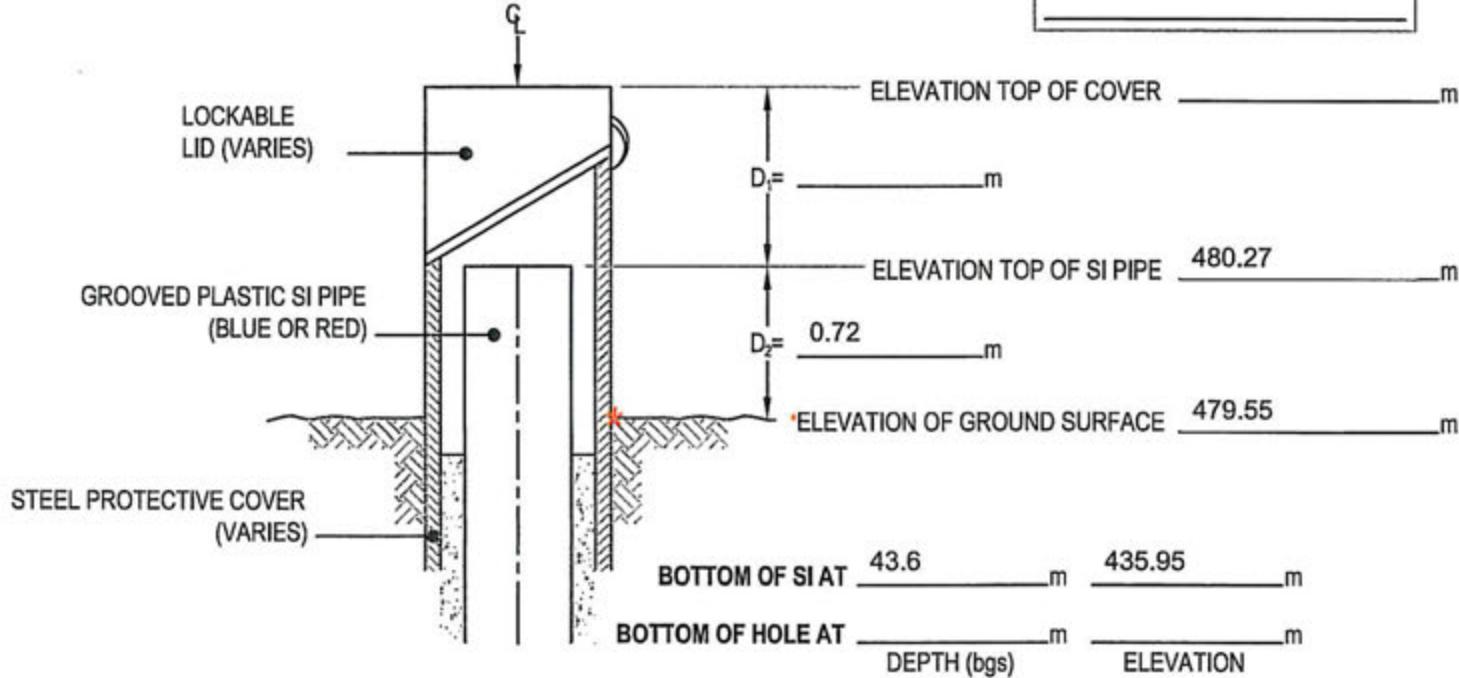
SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 128 °mag _____ °true

A). STICK-UP CASING PROTECTOR

NORTHING: 5869298.11

EASTING: 532568.71

Anchor/Grout/backfill details:



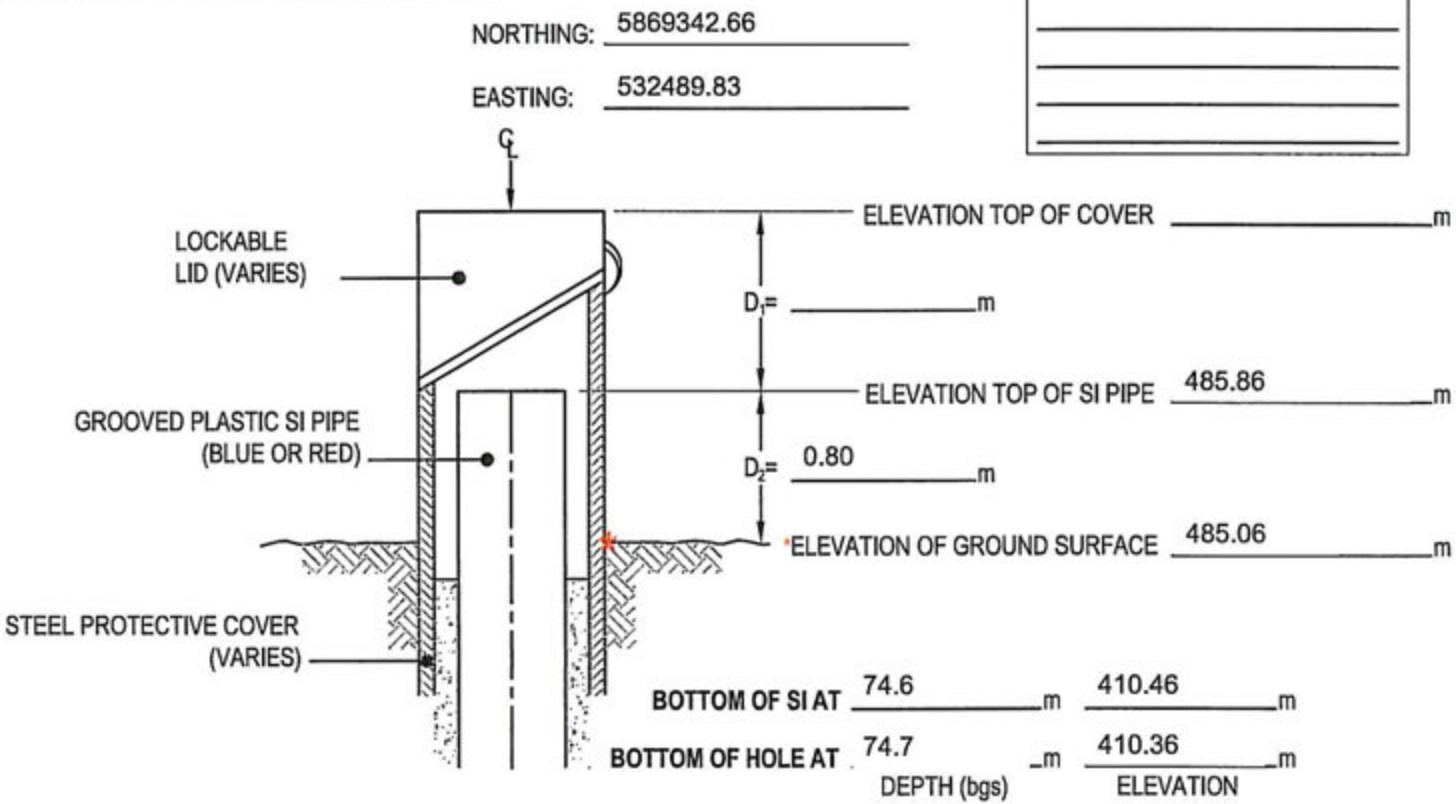
GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL



Project Name: WEST QUESNEL SLOPE STABILITY
 Date (s) Installed: 23 OCTOBER 2000 Project No: KX04397
 Supervised by AMEC: D.DEWAR Install'n No: SI2
 Install Method / Driller: CARIBOU WATER WELLS Lock/Key No:
 SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 138 °mag _____ °true

A). STICK-UP CASING PROTECTOR



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 27 OCTOBER 2000 Project No: KX04397

Supervised by AMEC: D.DEWAR Install'n No: SI3

Install Method / Driller: CARIBOU WATER WELLS Lock/Key No:

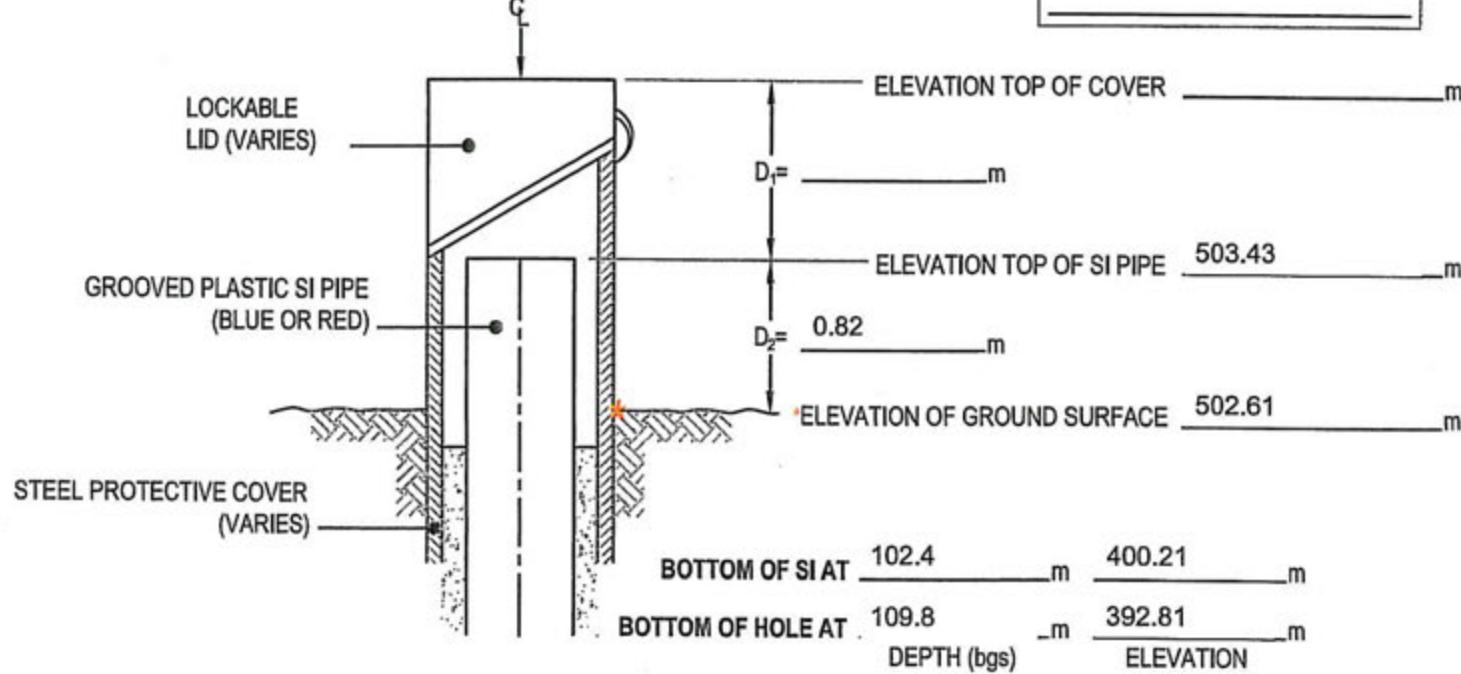
SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 108 °mag _____ °true

A). STICK-UP CASING PROTECTOR

NORTHING: 5869598.07

EASTING: 532273.05

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL



Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 13 OCTOBER 2000 Project No: KX04397

Supervised by AMEC: D.DEWAR Install'n No: SI4

Install Method / Driller: CARIBOU WATER WELLS Lock/Key No: _____

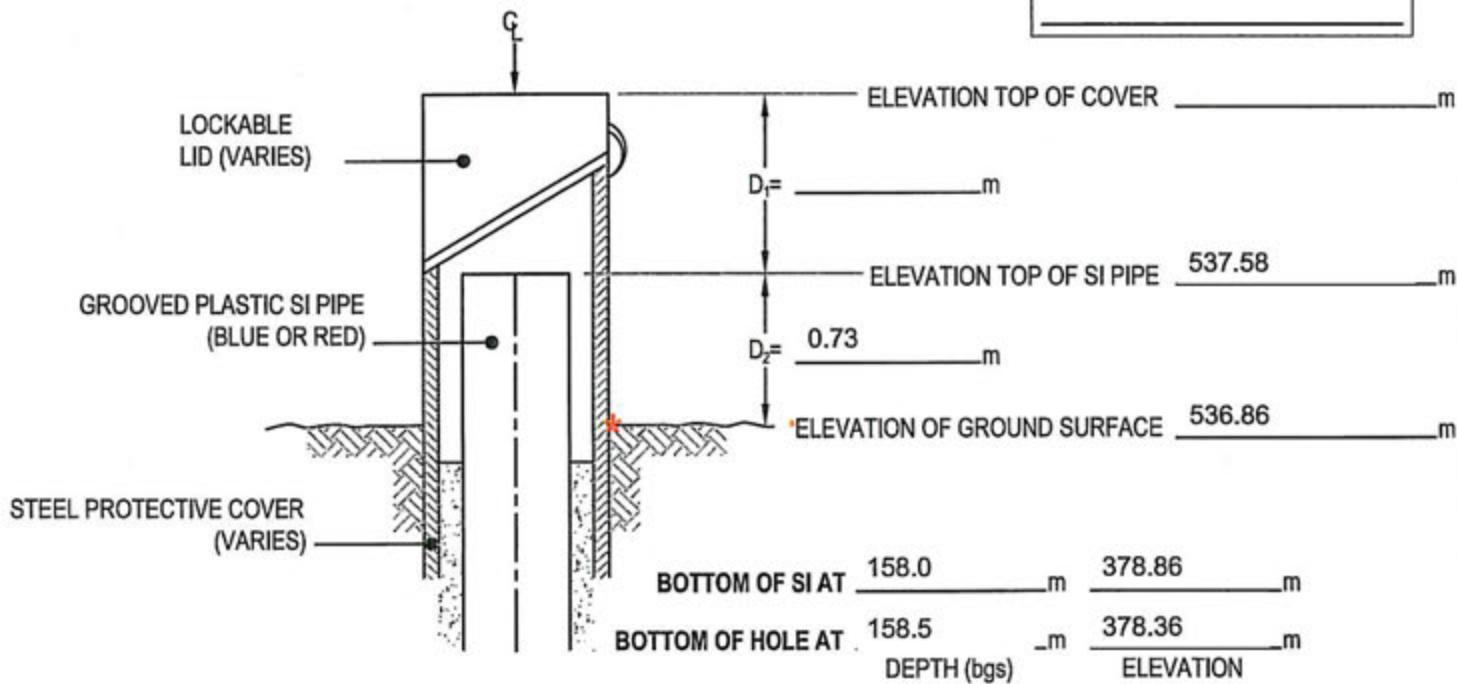
SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 128 °mag _____ °true

A). STICK-UP CASING PROTECTOR

NORTHING: 5869878.16

EASTING: 531829.52

Anchor/Grout/backfill details:



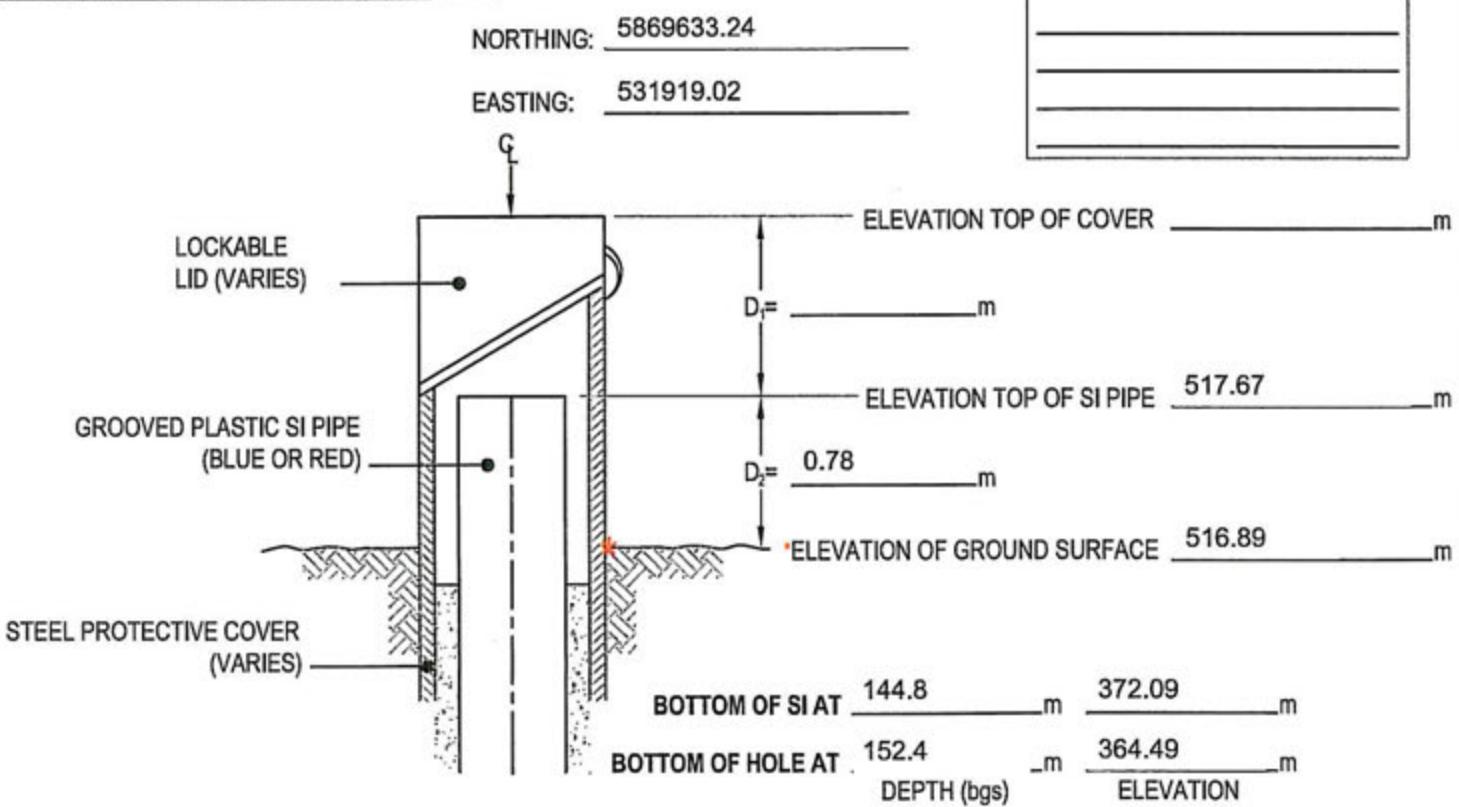
GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL



Project Name: WEST QUESNEL SLOPE STABILITY
 Date (s) Installed: 5 OCTOBER 2000 Project No: KX04397
 Supervised by AMEC: D.DEWAR Install'n No: SI5
 Install Method / Driller: CARIBOU WATER WELLS Lock/Key No:
 SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 110 °mag _____ °true

A). STICK-UP CASING PROTECTOR



Anchor/Grout/backfill details:

GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

16 OCTOBER 2000

Project No:

KX04397

Supervised by AMEC:

D.DEWAR

Install'n No:

SI6

Install Method / Driller:

CARIBOU WATER WELLS

Lock/Key No:

SI Casing Make / Type:

Dia:

Length:

Az of A0:

105

°mag

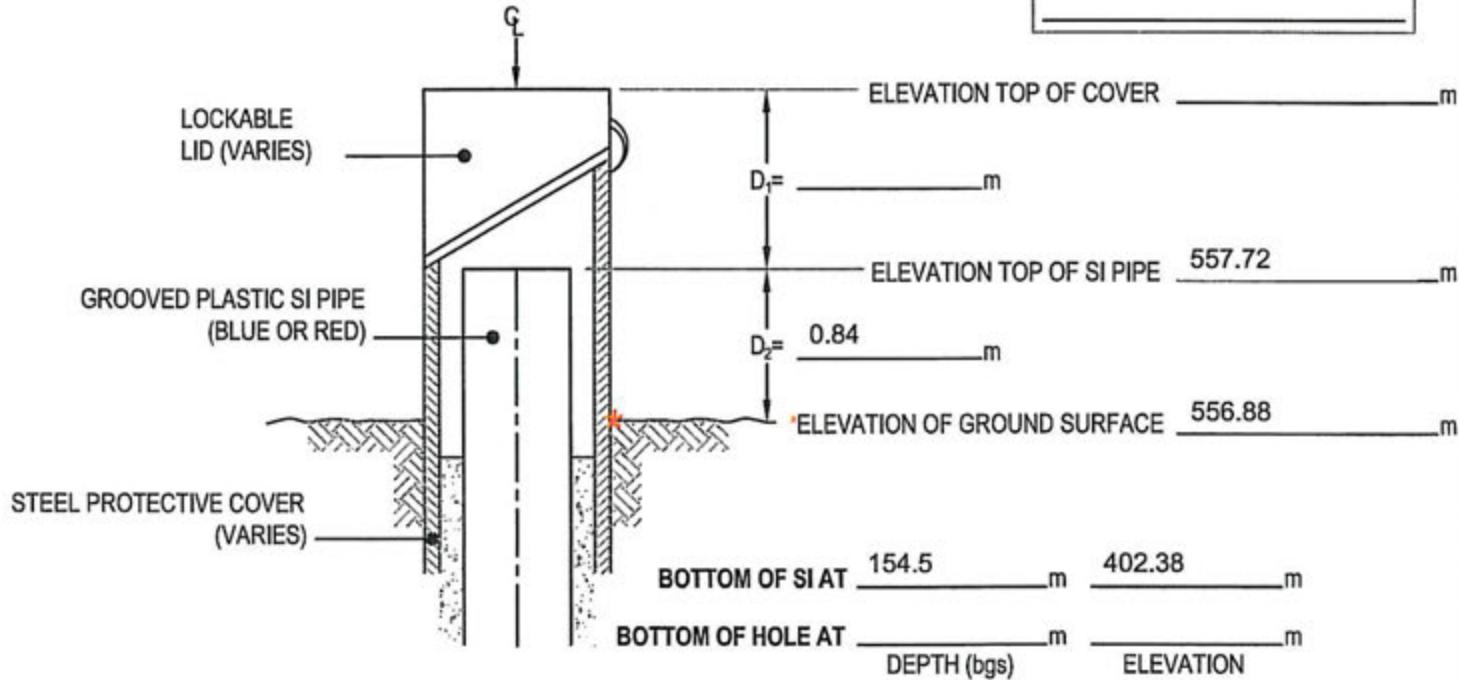
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A). STICK-UP CASING PROTECTOR

NORTHING: 5869933.44

EASTING: 531509.91

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

19 OCTOBER 2000

Project No: KX04397

Supervised by AMEC:

D.DEWAR

Install'n No: SI7

Install Method / Driller:

CARIBOU WATER WELLS

Lock/Key No:

SI Casing Make / Type:

Dia:

Length:

Az of A0:

84

°mag

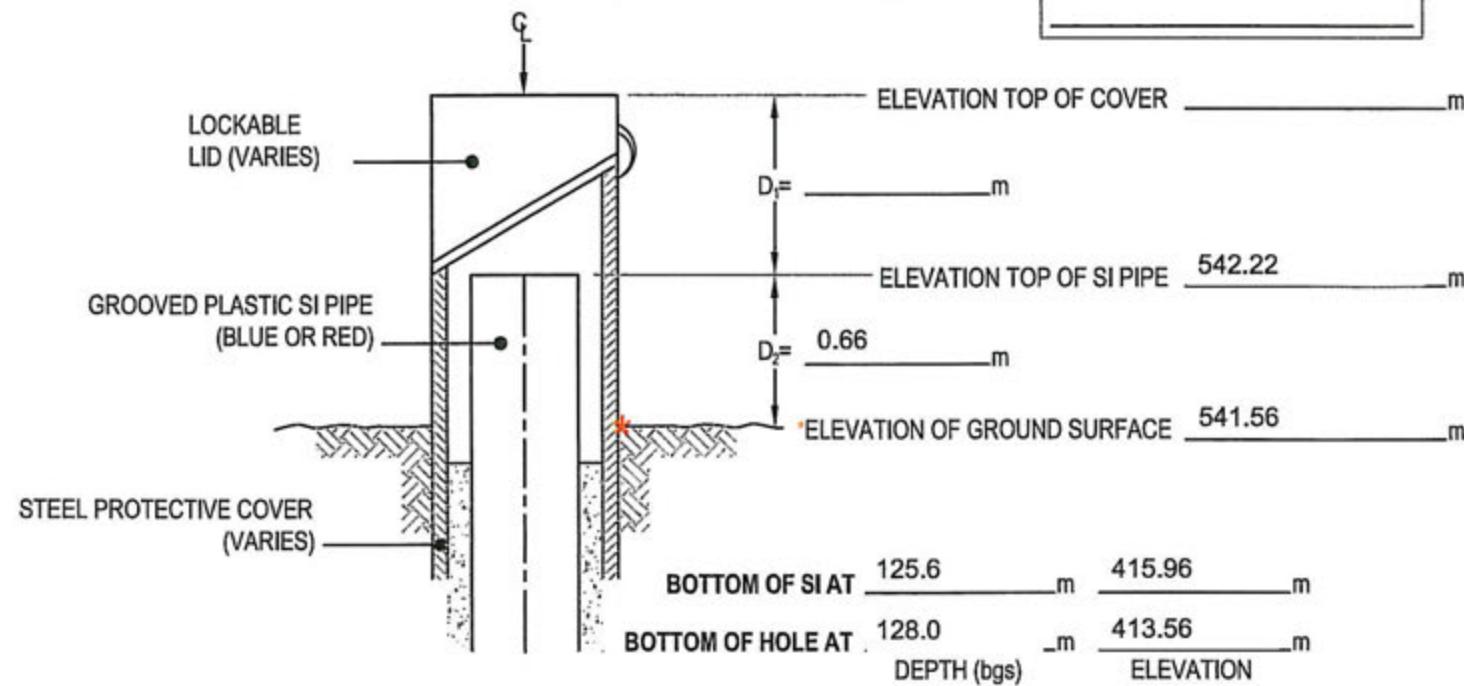
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A). STICK-UP CASING PROTECTOR

NORTHING: 5870231.73

EASTING: 531798.61

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL



Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 30 MARCH 2005 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: SI8

Install Method / Driller: INGERSOLL RAND TH-60
CARIBOU WATER WELLS Lock/Key No: _____

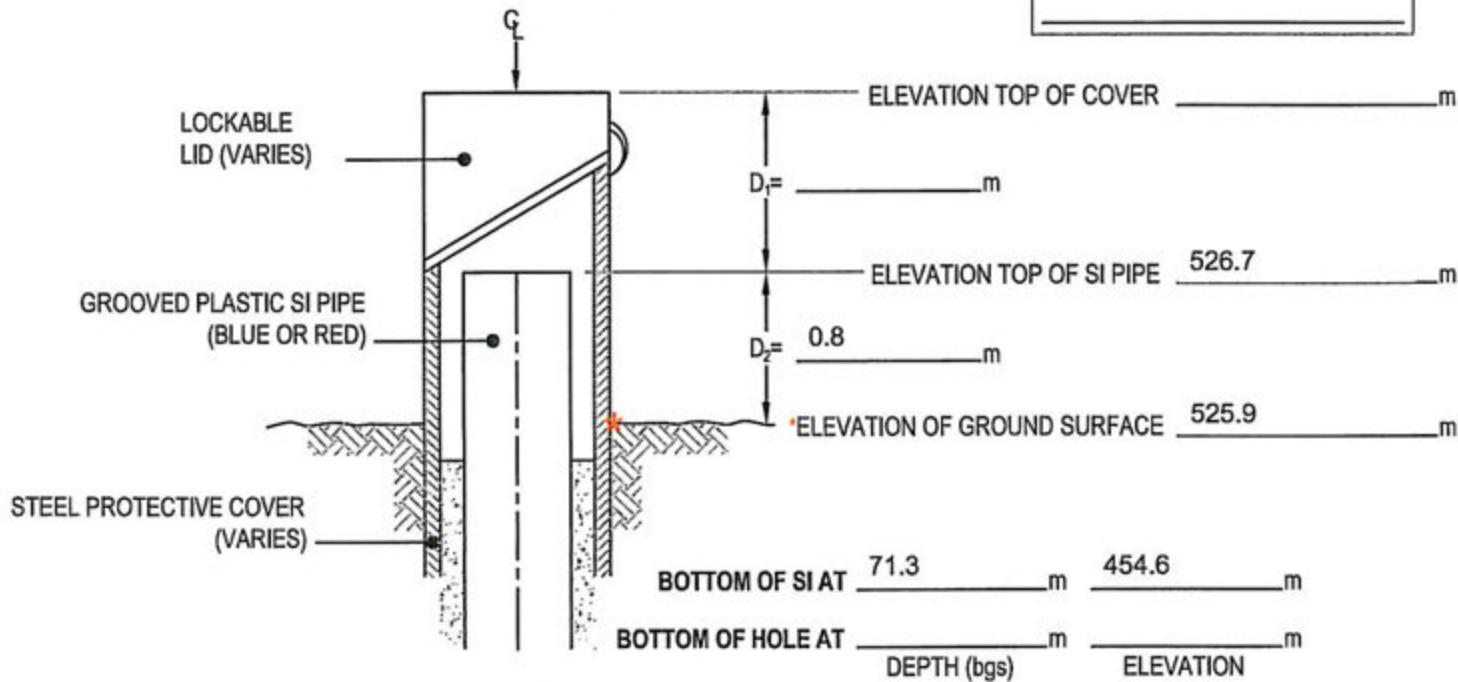
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A). STICK-UP CASING PROTECTOR

NORTHING: 5870035.94

EASTING: 532164.87

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

1 APRIL 2005

Project No:

KX04397

Supervised by AMEC:

S.GREEN

Install'n No:

SI9

Install Method / Driller:

INGERSOLL RAND TH-60
CARIBOU WATER WELLS

Lock/Key No:

SI Casing Make / Type:

Dia:

Length:

Az of A0:

146

°mag

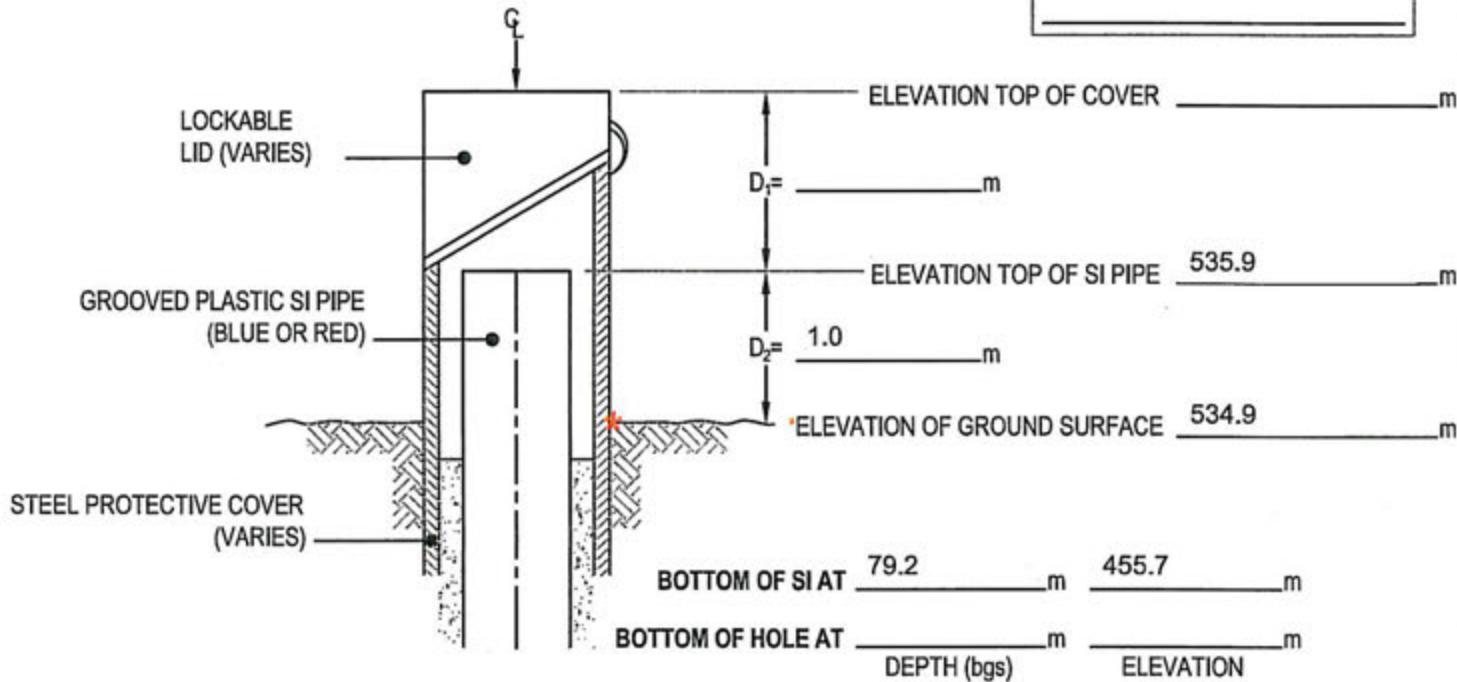
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A). STICK-UP CASING PROTECTOR

NORTHING: 5870113.71

EASTING: 531881.71

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 3 APRIL 2005 Project No: KX04397

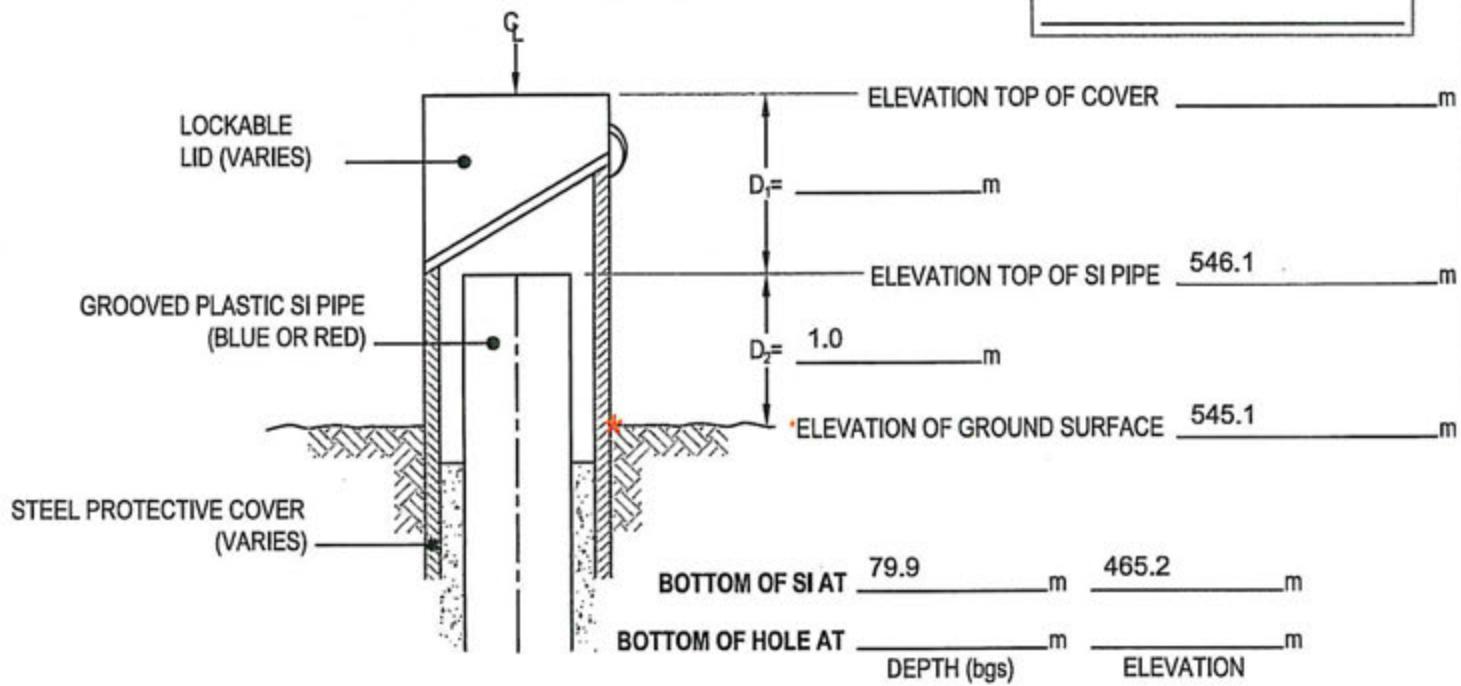
Supervised by AMEC: S.GREEN Install'n No: SI10

Install Method / Driller: INGERSOLL RAND TH-60
CARIBOU WATER WELLS Lock/Key No: _____

SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 112 °mag _____ °true _____

A). STICK-UP CASING PROTECTOR

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

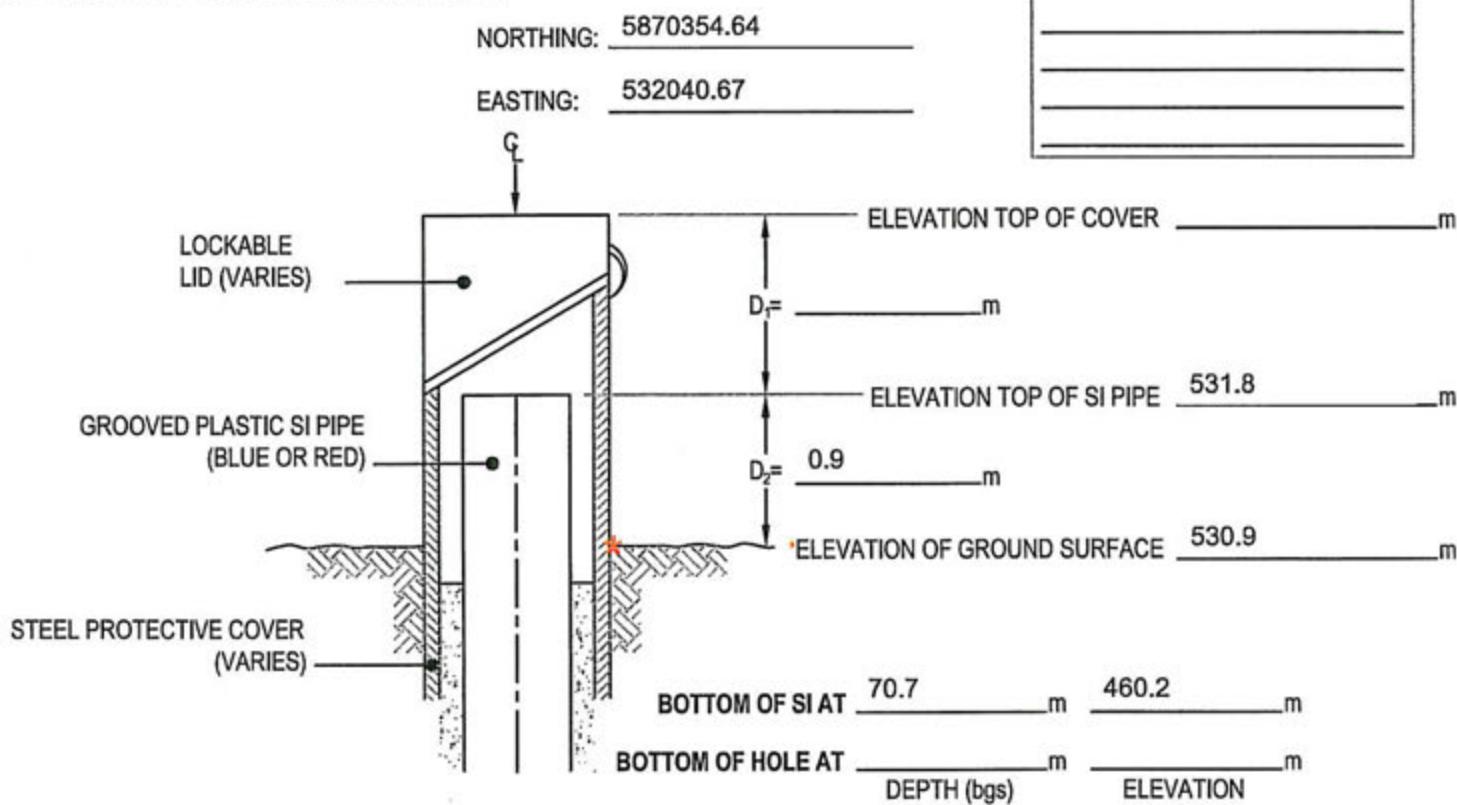
Date (s) Installed: 4 APRIL 2005 Project No: KX04397

Supervised by AMEC: S.GREEN Install'n No: SI11

Install Method / Driller: INGERSOLL RAND TH-60
CARIBOU WATER WELLS Lock/Key No: _____

SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 80 °mag _____ °true

A). STICK-UP CASING PROTECTOR



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

4 APRIL 2005

Project No:

KX04397

Supervised by AMEC:

S.GREEN

Install'n No:

SI12

Install Method / Driller:

INGERSOLL RAND TH-60
CARIBOU WATER WELLS

Lock/Key No:

SI Casing Make / Type:

Dia:

Length:

Az of A0:

110

°mag

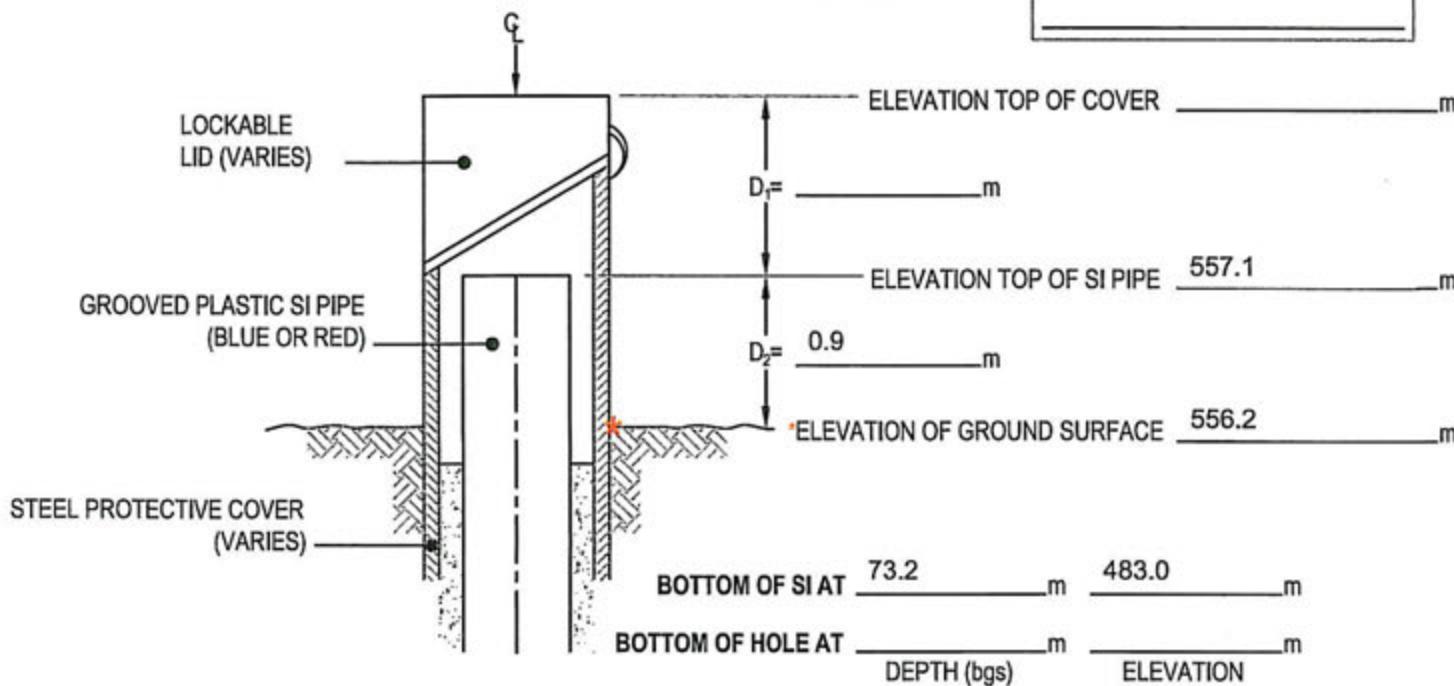
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A). STICK-UP CASING PROTECTOR

NORTHING: 5870278.21

EASTING: 531621.51

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name:

WEST QUESNEL SLOPE STABILITY

Date (s) Installed:

7 APRIL 2005

Project No:

KX04397

Supervised by AMEC:

S.GREEN

Install'n No:

SI13

Install Method / Driller:

INGERSOLL RAND TH-60
CARIBOU WATER WELLS

Lock/Key No:

SI Casing Make / Type:

Dia:

Length:

Az of A0:

54

°mag

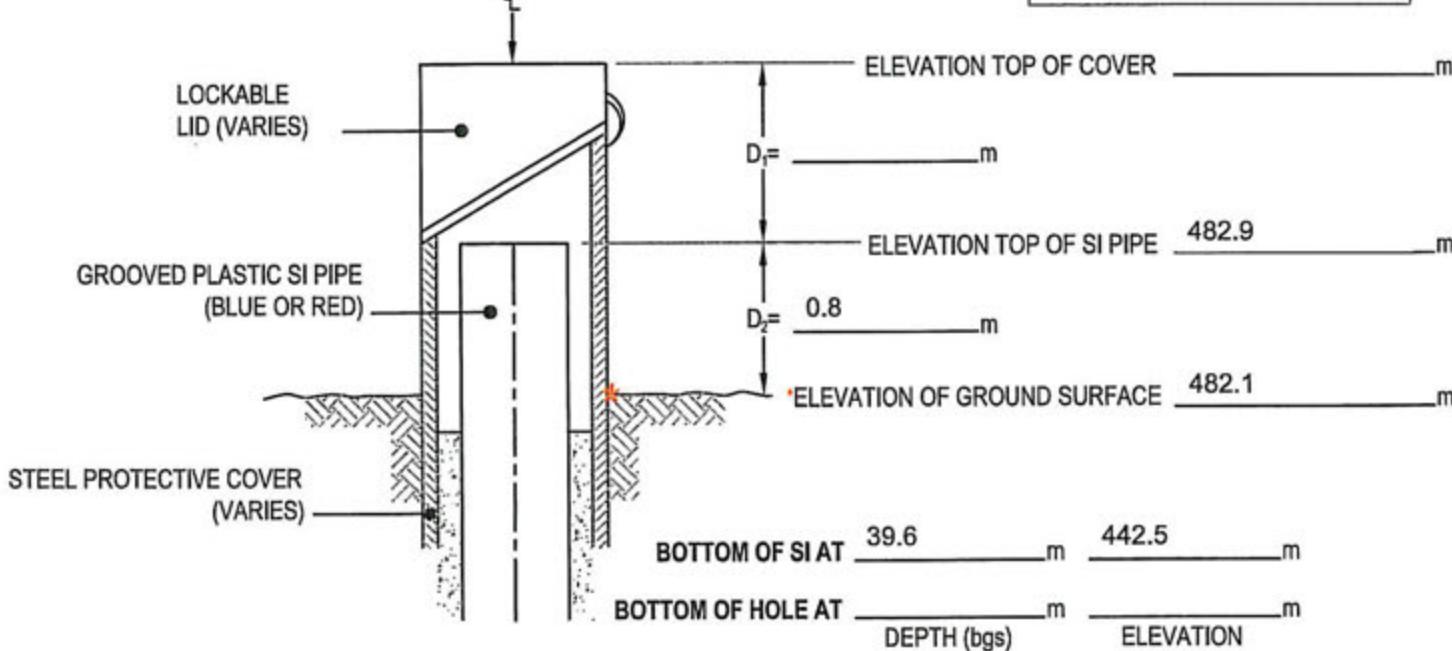
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A). STICK-UP CASING PROTECTOR

NORTHING: 5869937.72

EASTING: 532468.20

Anchor/Grout/backfill details:



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 20 JUNE 2005 Project No: KX04397

Supervised by AMEC: R.GUSTAFSON Install'n No: SI14

Install Method / Driller: B-80 GEOTECH DRILLING Lock/Key No: _____

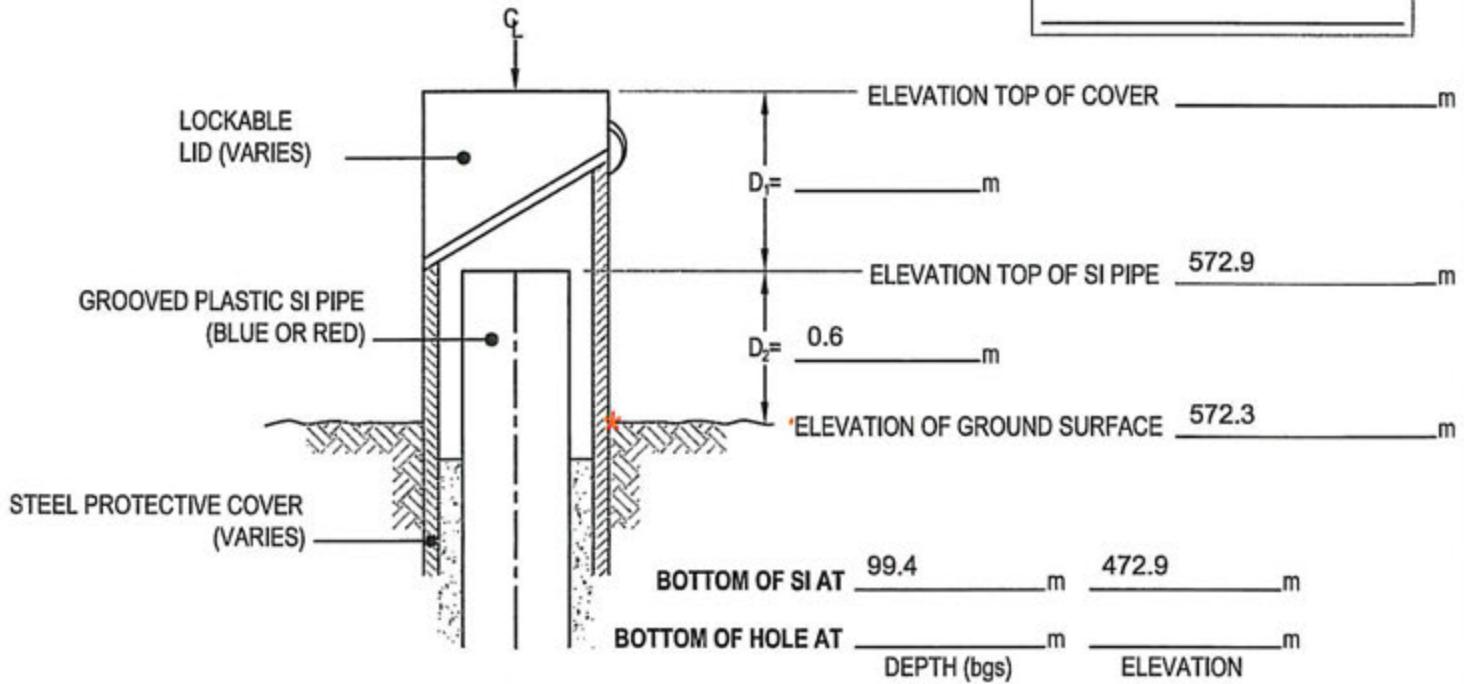
SI Casing Make / Type: _____ Dia: _____ Length: _____ Az of A0: 148 °mag _____ °true

A). STICK-UP CASING PROTECTOR

Anchor/Grout/backfill details:

NORTHING: 5869683.61

EASTING: 531276.55



GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

SLOPE INCLINOMETER (SI) INSTALLATION DETAIL

amec

Project Name: WEST QUESNEL SLOPE STABILITY

Date (s) Installed: 25 JULY 2005 Project No: KX04397

Supervised by AMEC: R.GUSTAFSON Install'n No: SI15

Install Method / Driller: GEOPROBE GEOTECH DRILLING Lock/Key No: _____

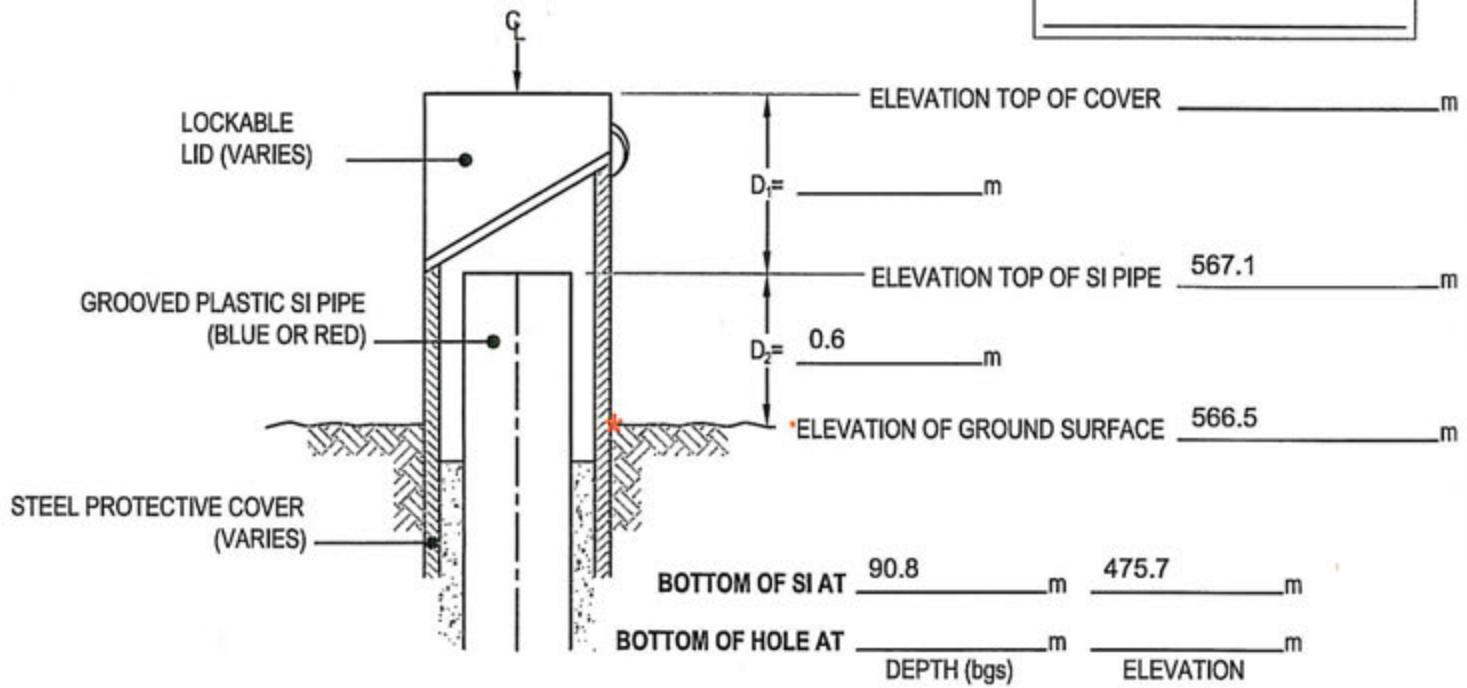
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A). STICK-UP CASING PROTECTOR

Anchor/Grout/backfill details:

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EASTING: 532163.15

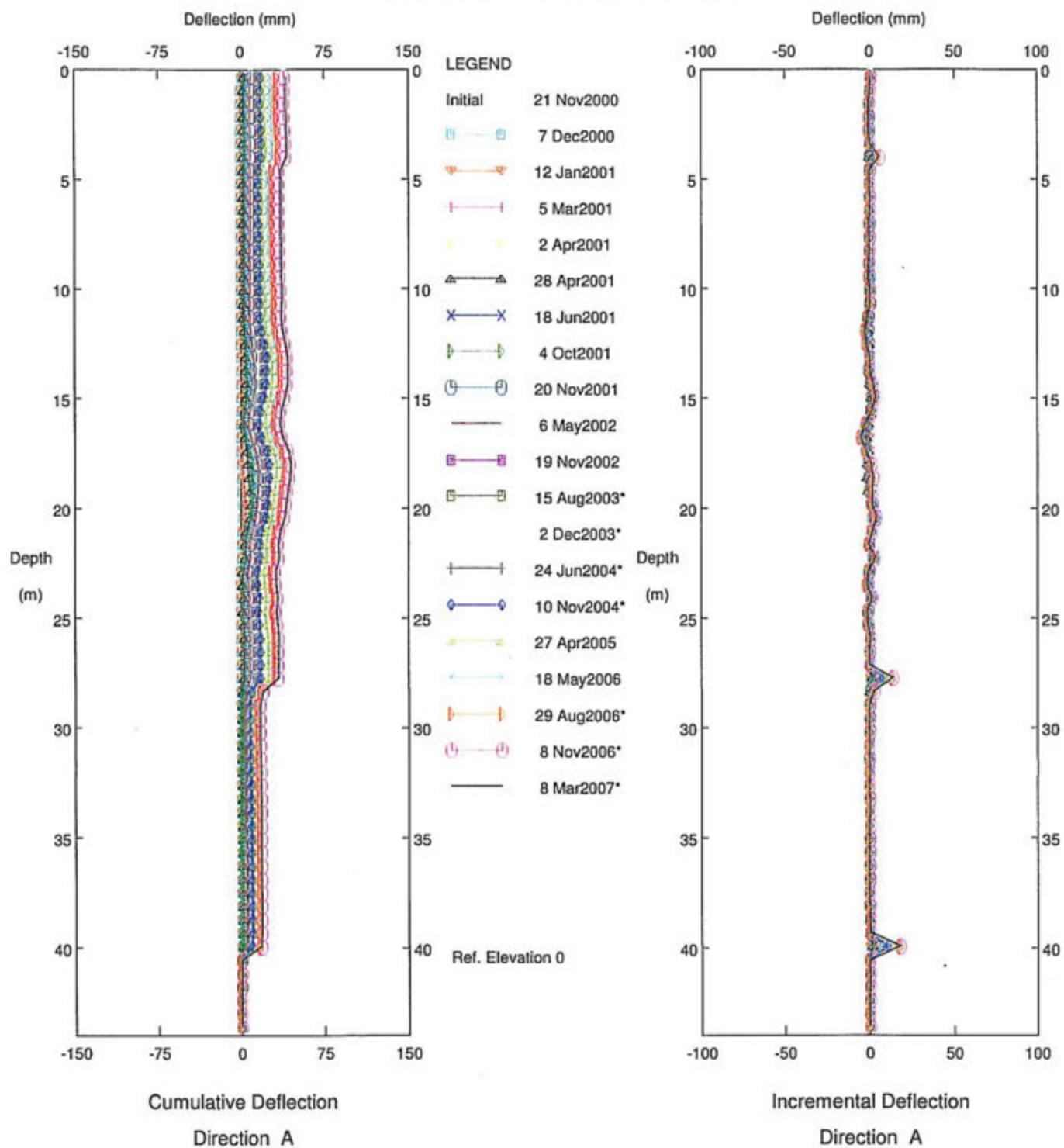


GROUND SURFACE ELEVATION SHOULD BE TAKEN DIRECTLY OUTSIDE THE INSTALLATION

Table G1: Slope Inclinometer Instrumentation Installation Details

Installation No.	Location	Ground Elevation (m)	Top of SI Elevation (m)	Install Depth (m) (from top of SI)	Ao Azimuth (°)
SI-1	Avery Lane	479.5	480.2	43.3	128
SI-2	Avery Lane	485.1	485.9	74.4	138
SI-3	Abbott & Bettcher	502.6	503.5	101.8	108
SI-4	Voyageur School	536.9	537.6	153.6	128
SI-5	Abbott Drive	516.9	517.6	144.5	-
SI-6	Dixon Street	556.9	557.7	153.0	150
SI-7	Lewis Drive & Pierce Street	541.6	542.3	124.9	84
SI-8	Pinchbeck St	525.9	526.7	71.3	82
SI-9	Patchett Ave	534.9	535.9	79.2	146
SI-10	Dawson Street	545.1	546.1	79.8	112
SI-11	Picard Ave	530.9	531.8	70.7	78
SI-12	East end of Panagrot Ave	556.2	557.1	73.2	110
SI-13	Lewis Drive	482.1	482.9	39.6	54
SI-14	Findlay Road	572.3	572.9	99.4	148
SI-15	West end of Panagrot Ave	566.5	567.1	90.8	92

AMEC Earth & Environmental - Pr. George

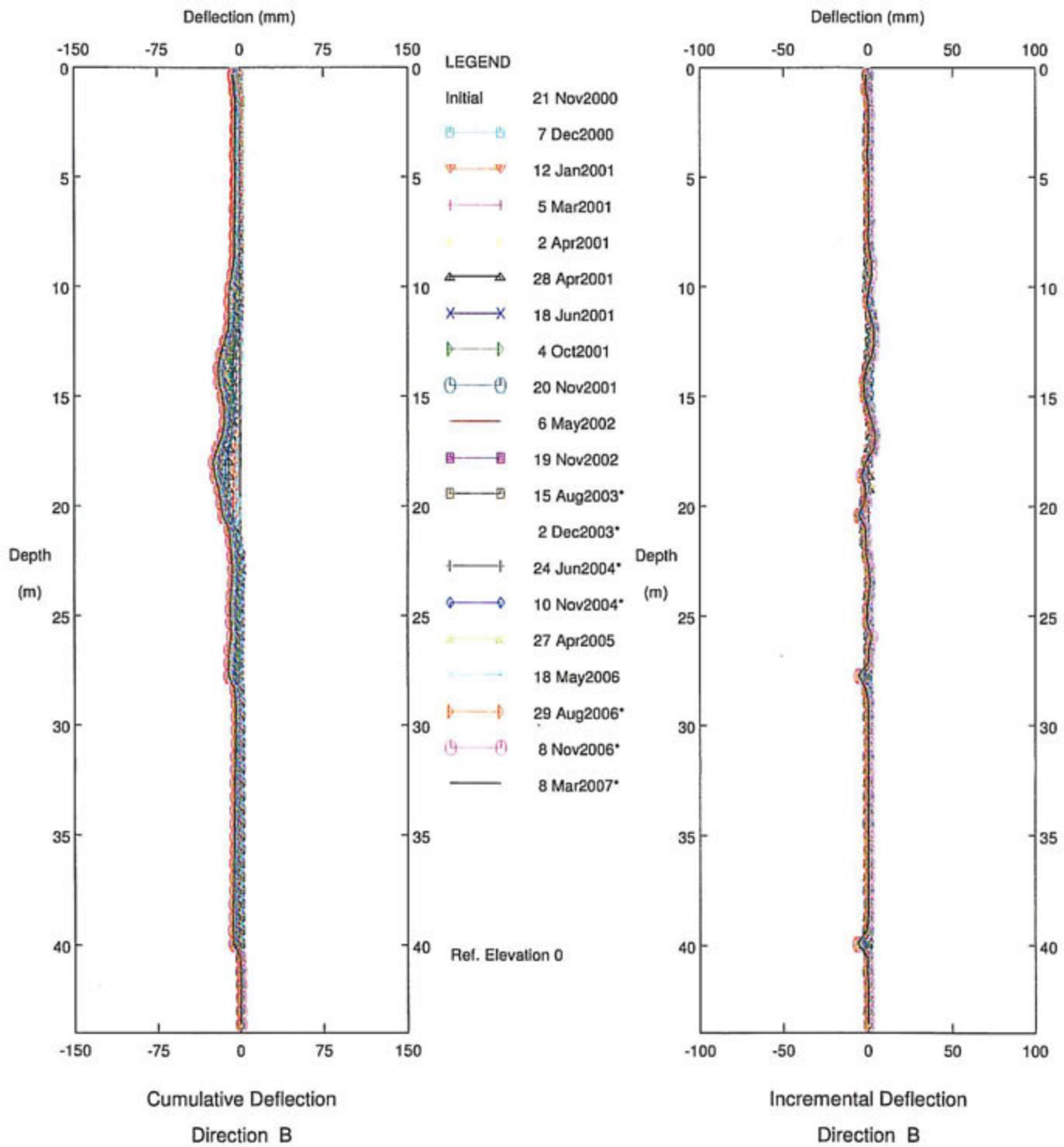


KX04397 W. Quesnel Stability Study, Inclinometer SI1

Lower Avery Lane

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

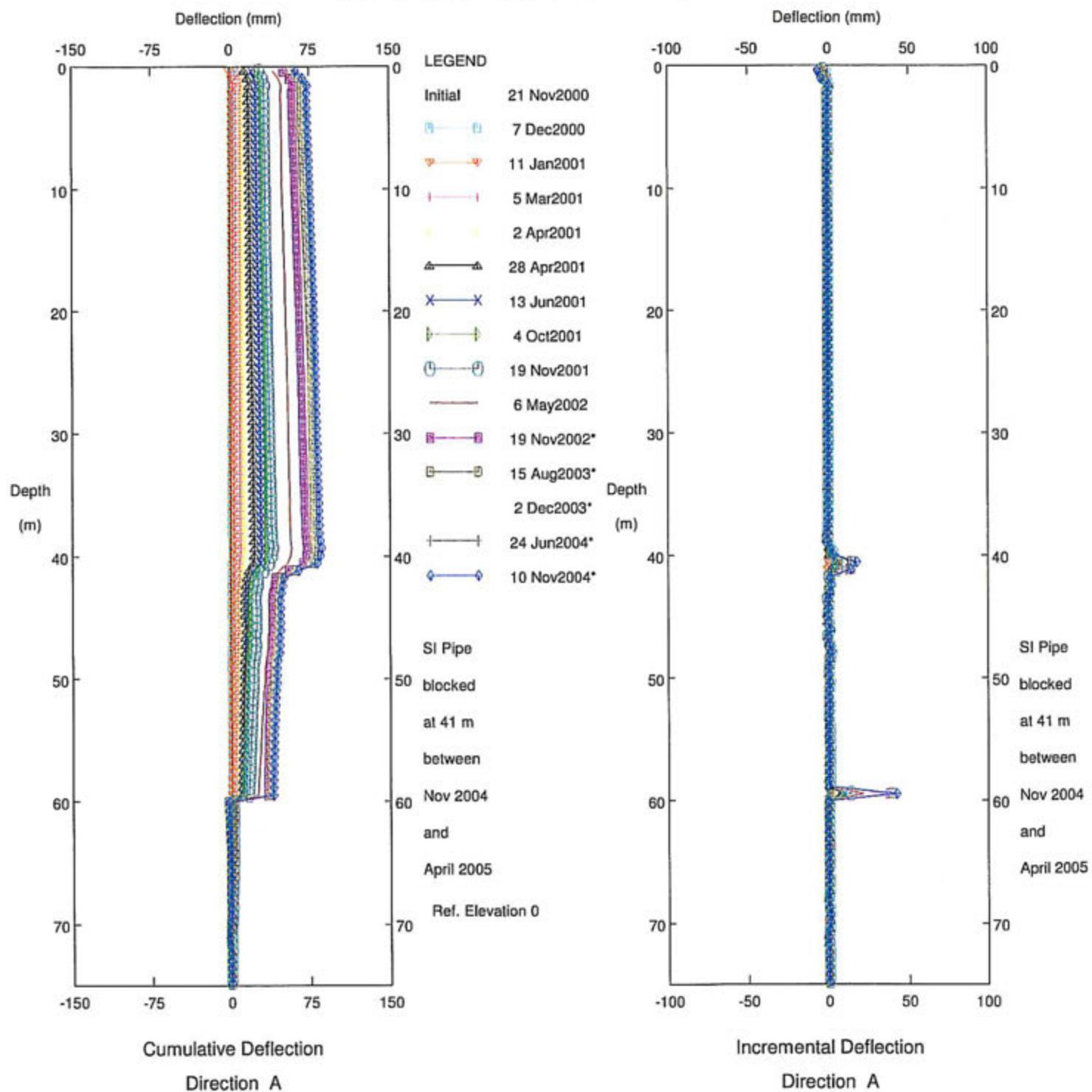


KX04397 W. Quesnel Stability Study, Inclinometer SI1

Lower Avery Lane

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

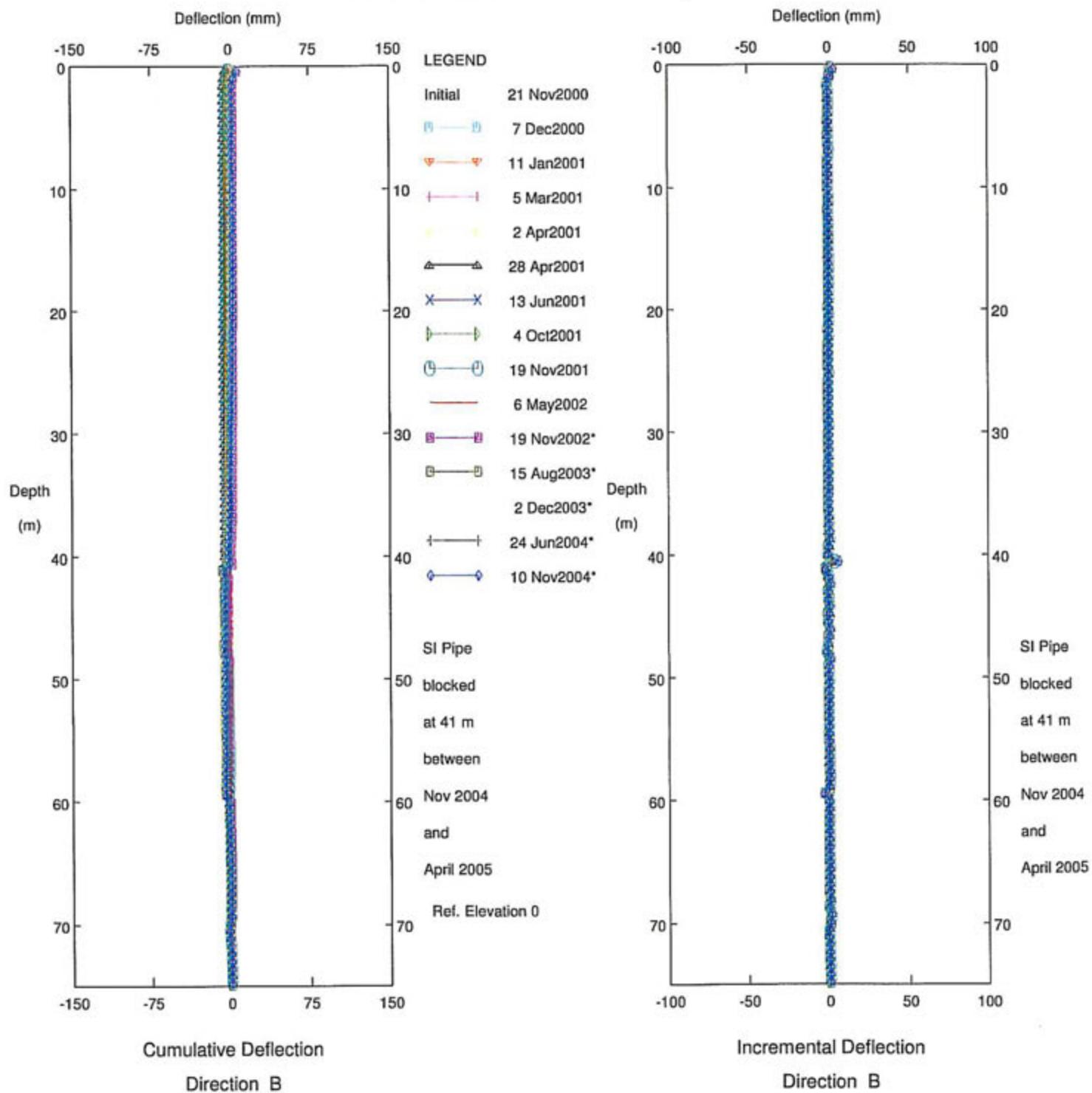


KX04397 W. Quesnel Stability Study, Inclinometer SI2

Upper Avery Lane

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

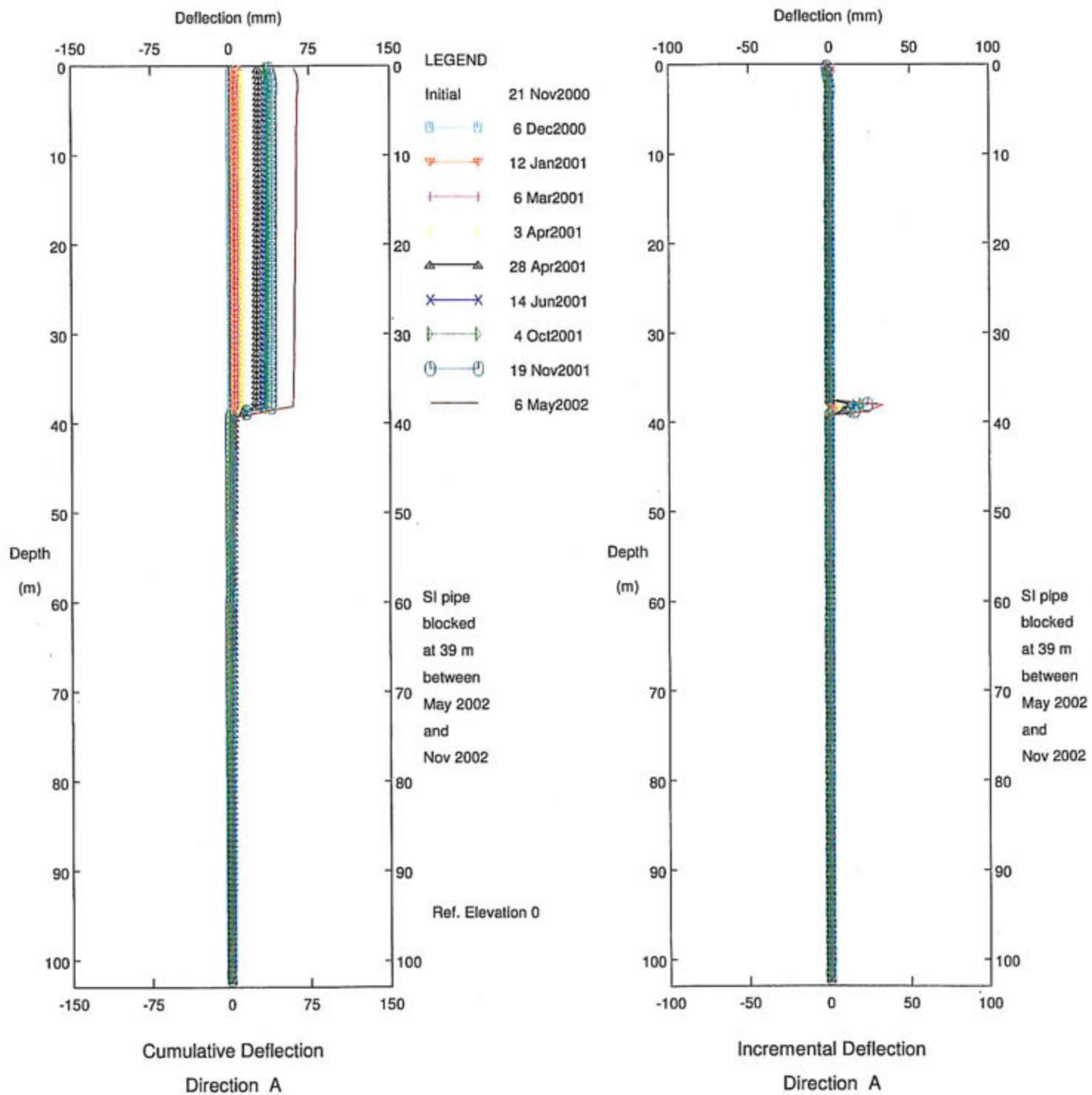


KX04397 W. Quesnel Stability Study, Inclinometer SI2

Upper Avery Lane

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

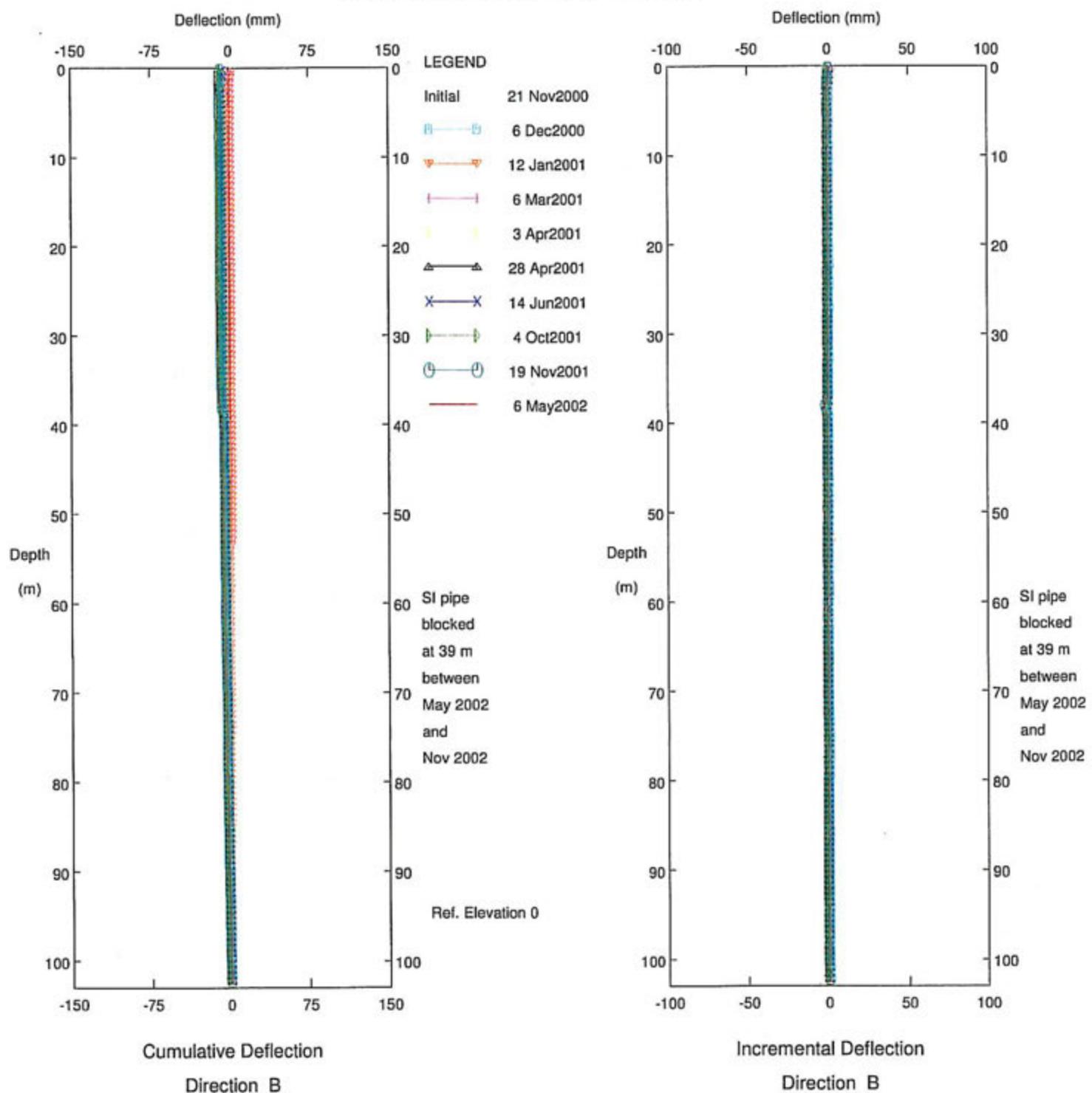


KX04397 W. Quesnel Stability Study, Inclinometer SI3

Abbott Drive near Bettcher

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

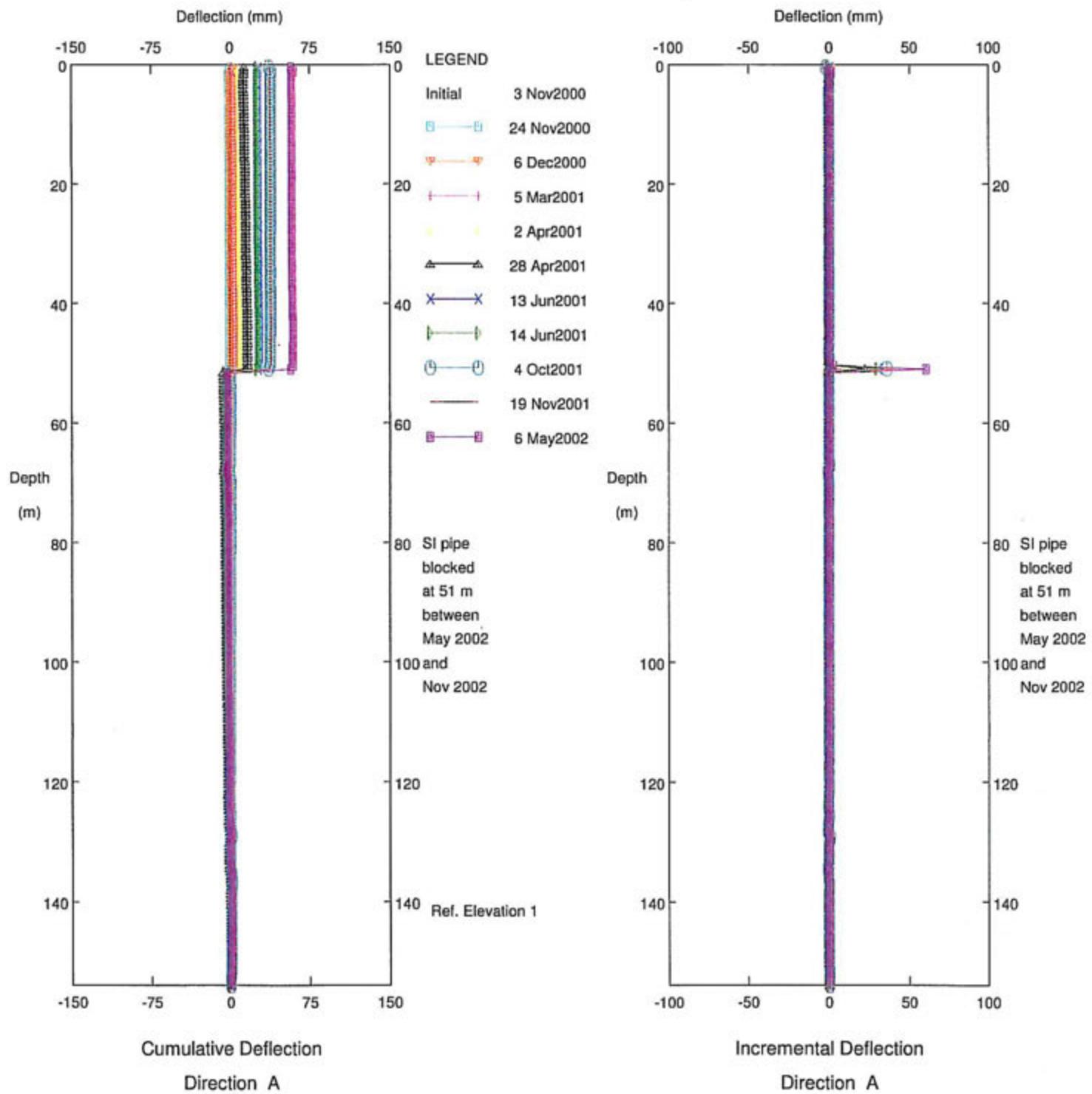


KX04397 W. Quesnel Stability Study, Inclinometer SI3

Abbott Drive near Bettcher

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

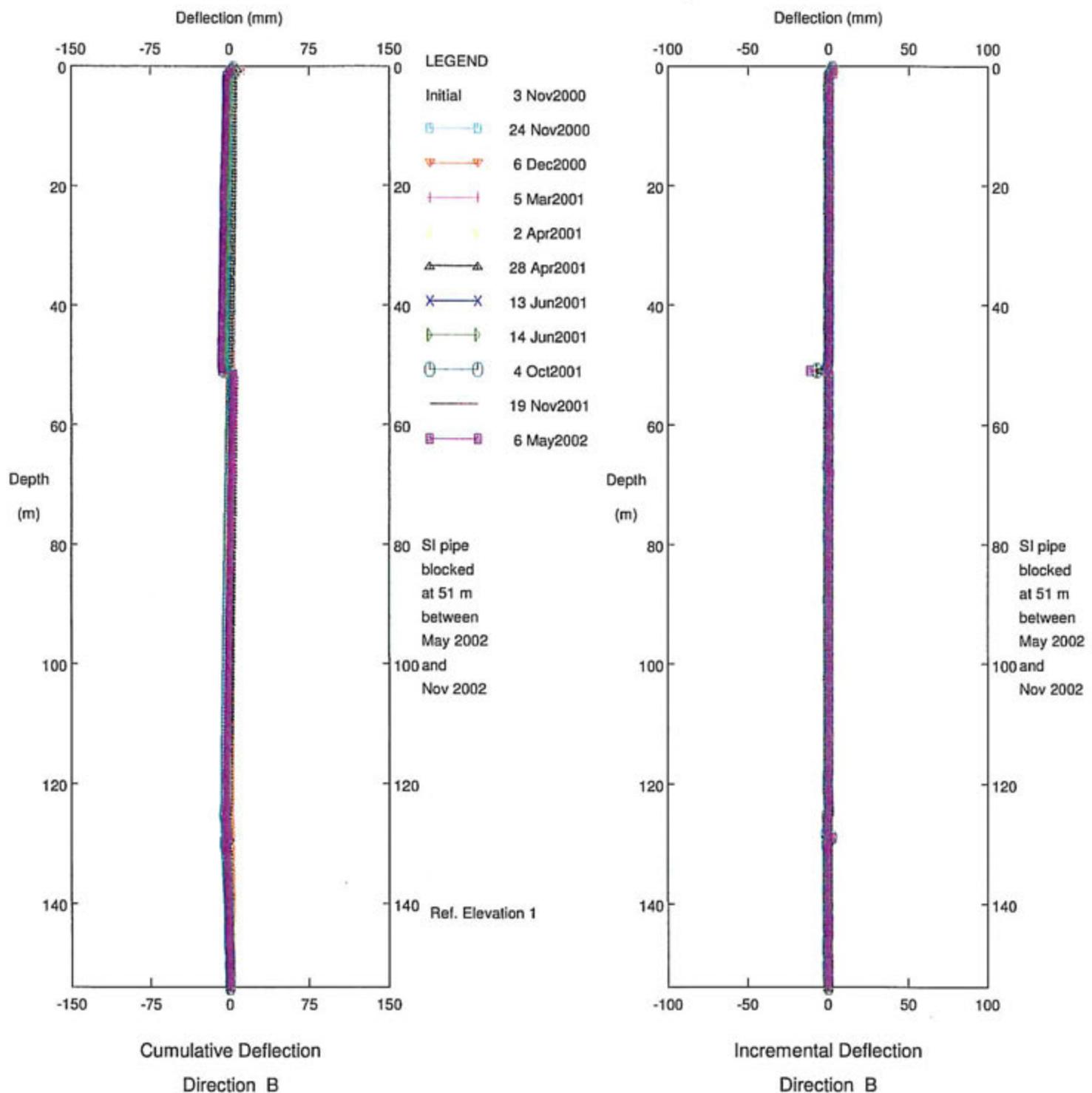


KX04397 W.Quesnel Stability Study, Inclinometer SI4

Voyageur School

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

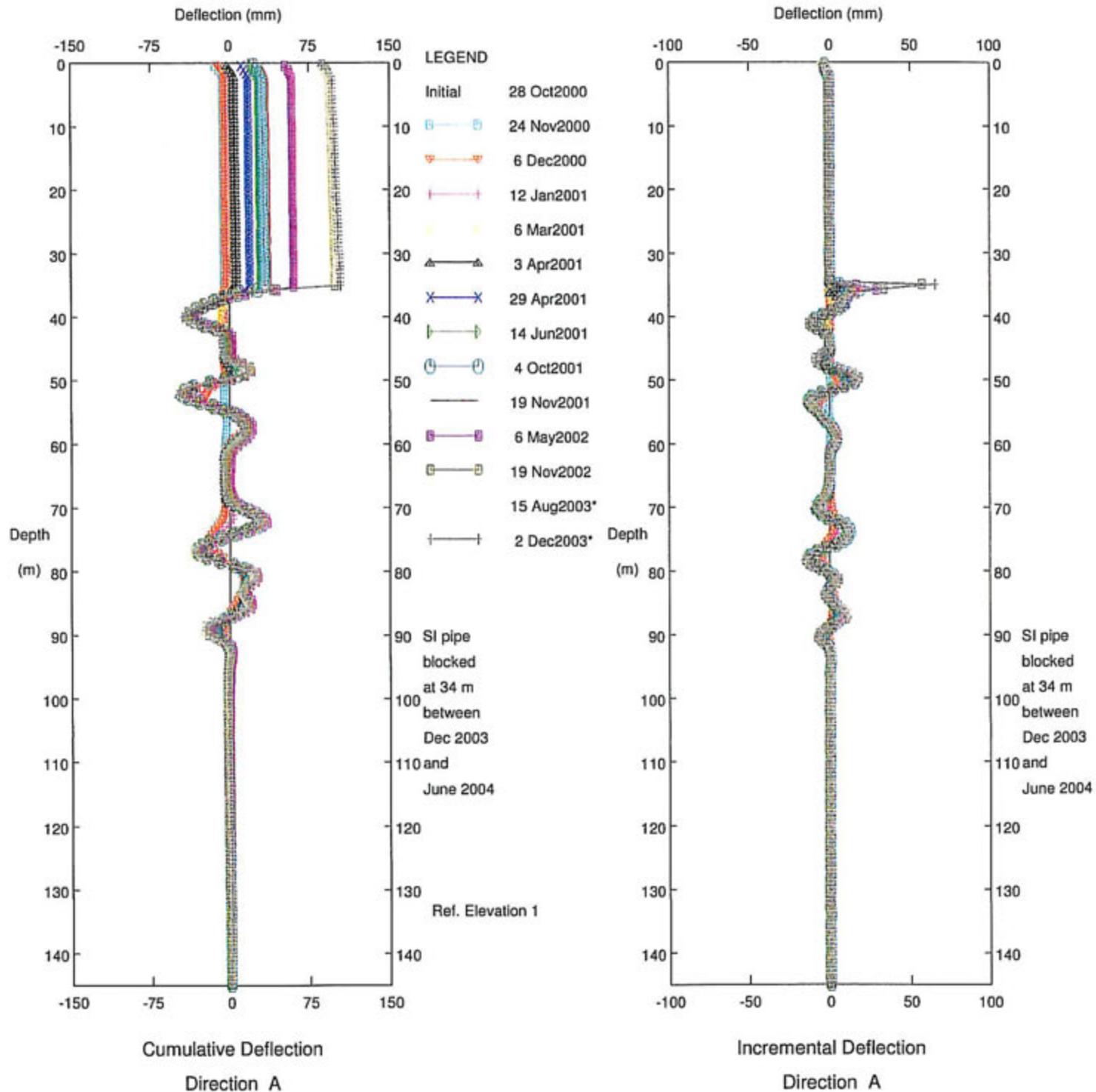


KX04397 W.Quesnel Stability Study, Inclinometer SI4

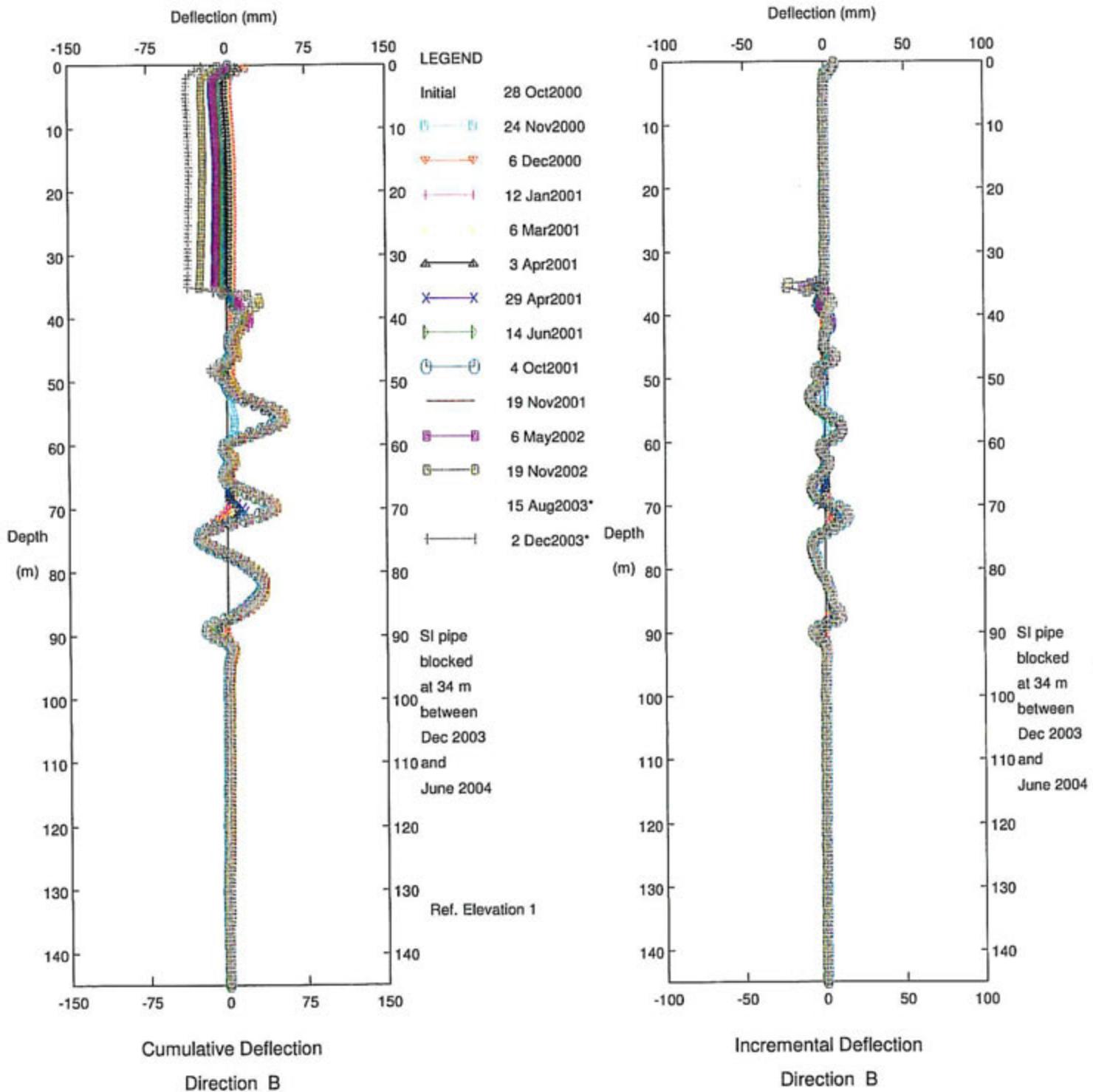
Voyageur School

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George



AMEC Earth & Environmental - Pr. George

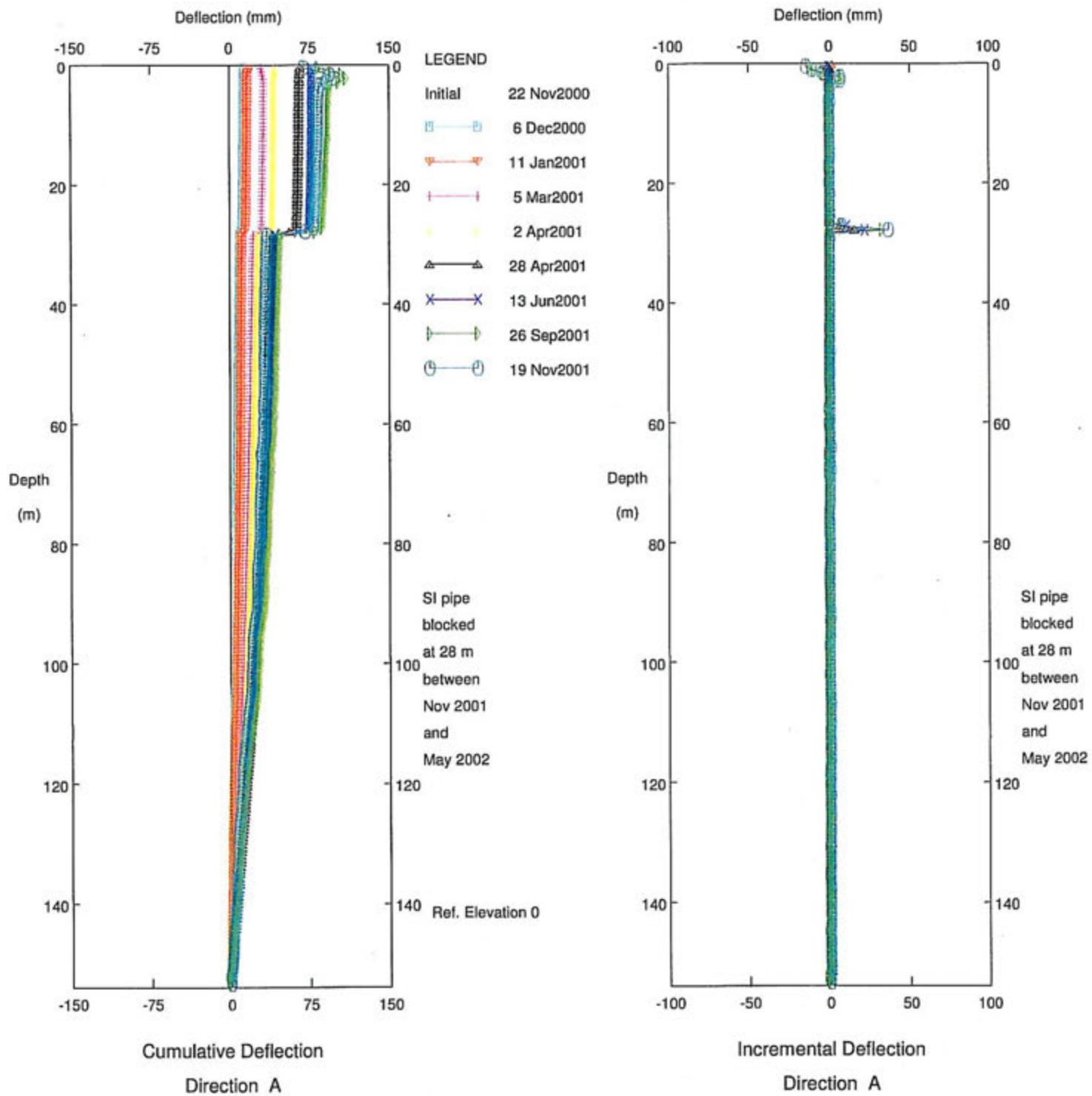


KX04397 W. Quesnel Stability Study, Inclinometer SI5

Abbott Drive

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

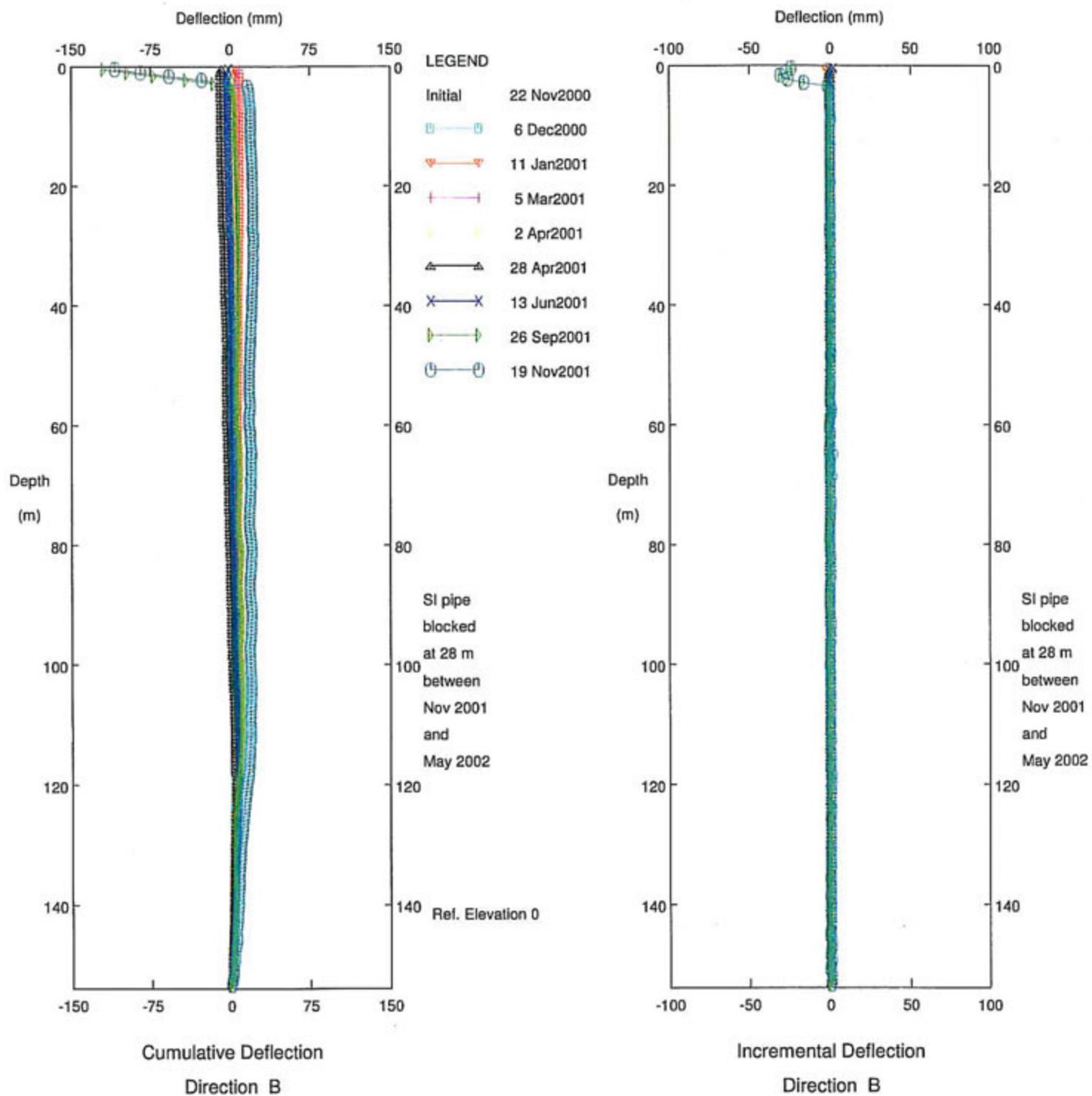


KX04397 W.Quesnel Stability Study, Inclinometer SI6

End of Dixon Street

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

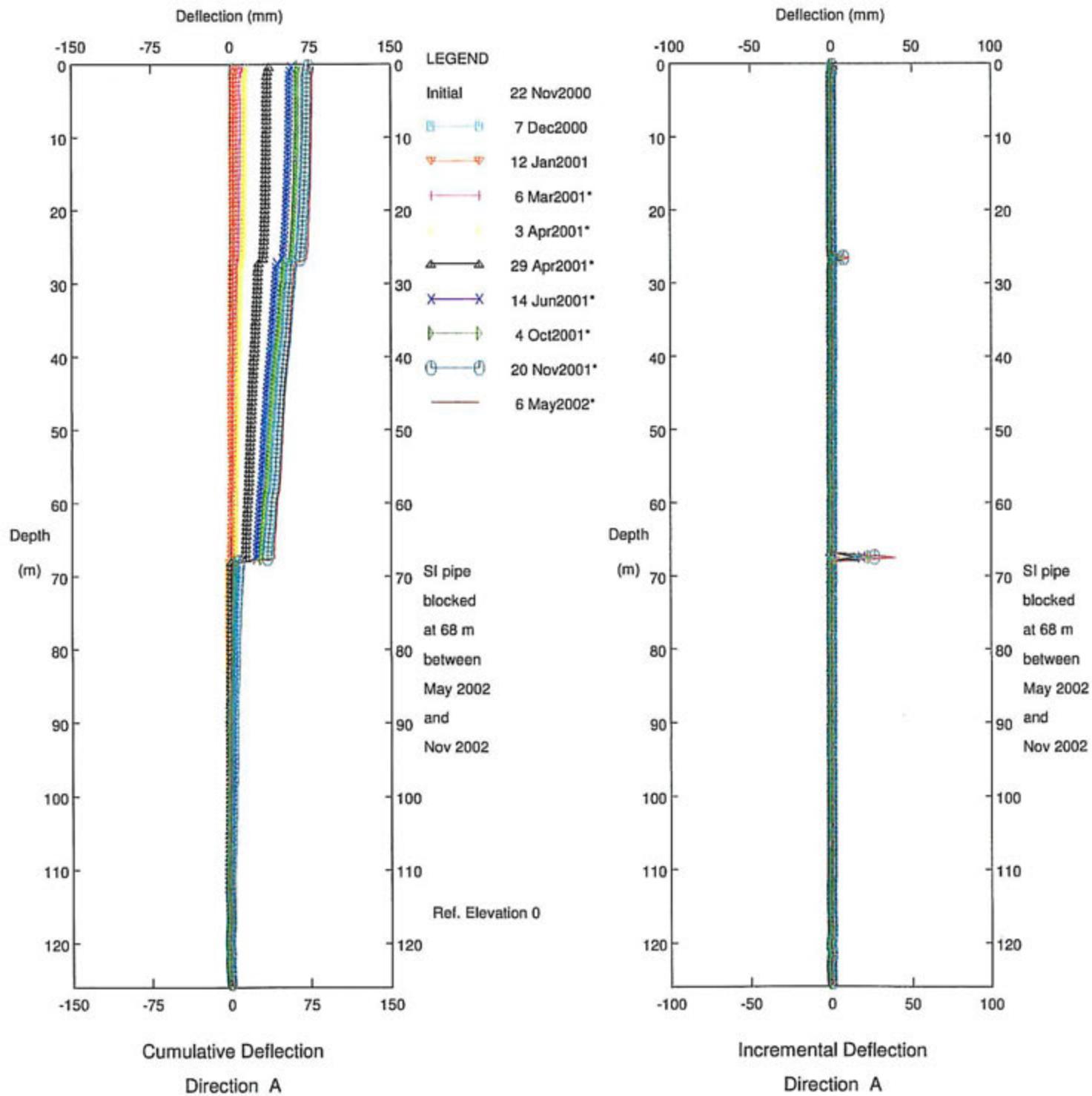


KX04397 W.Quesnel Stability Study, Inclinometer SI6

End of Dixon Street

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

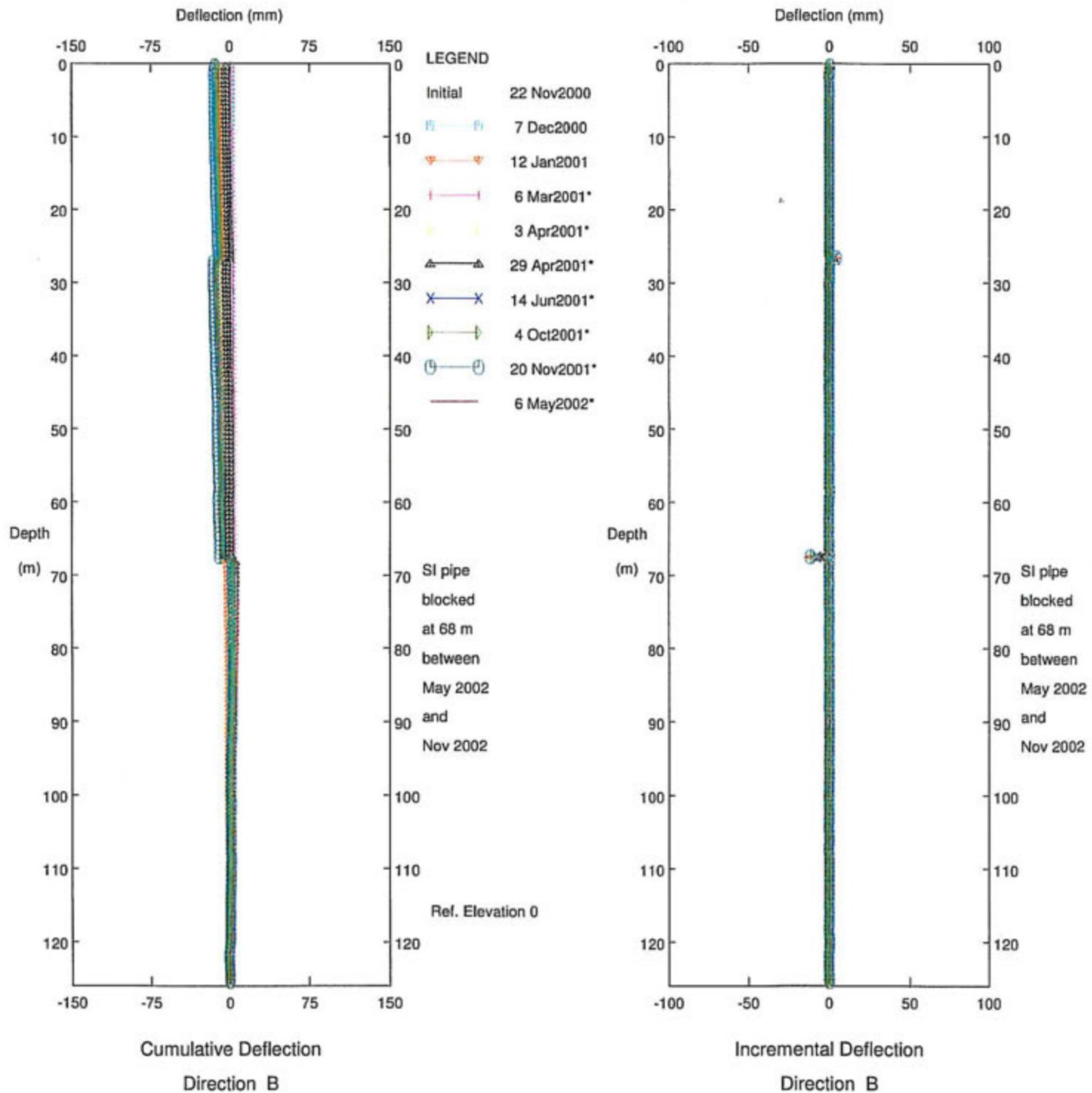


KX04397 W. Quesnel Stability Study, Inclinometer SI7

Pierce Crescent & Lewis Drive

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

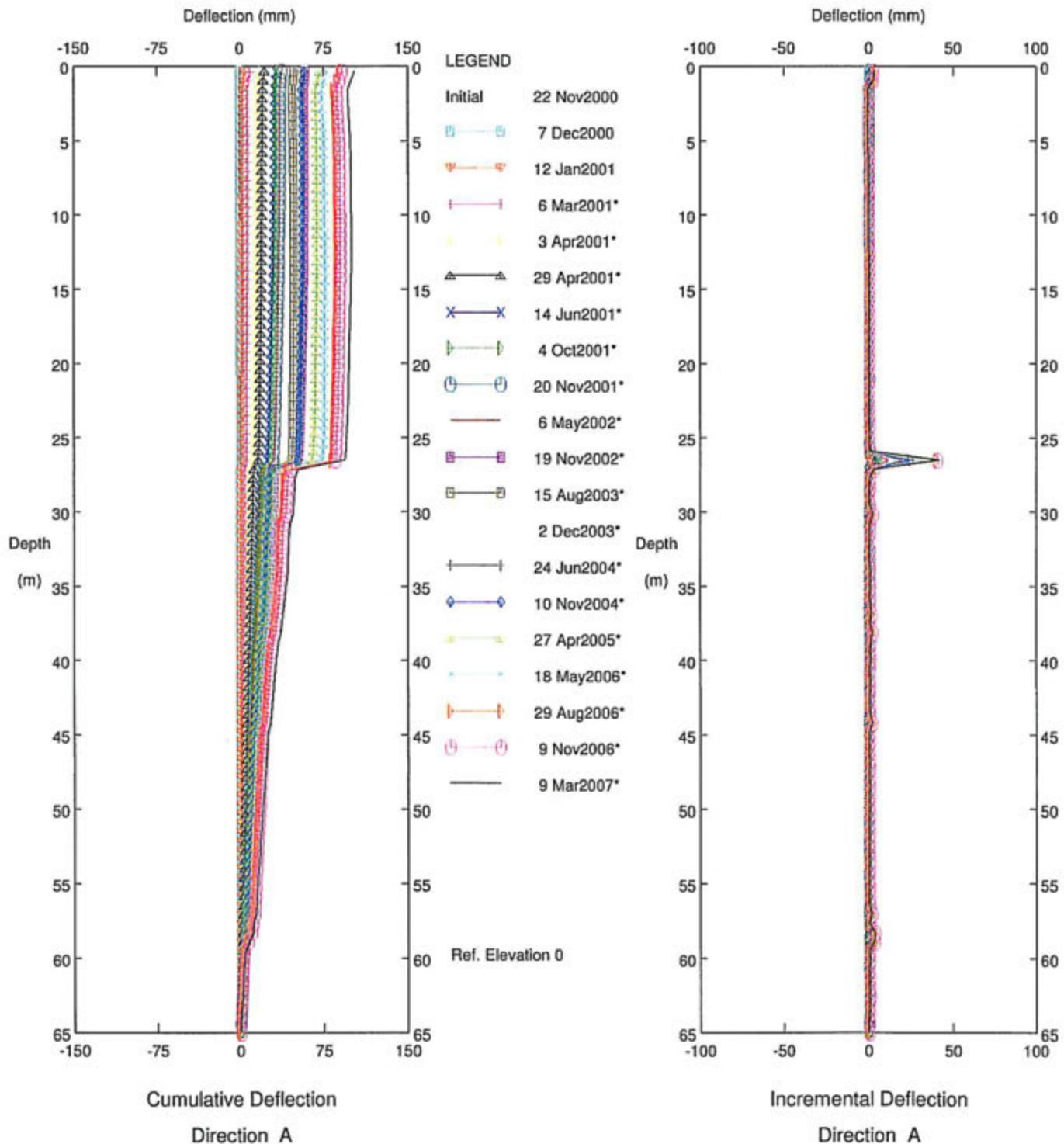


KX04397 W. Quesnel Stability Study, Inclinometer SI7

Pierce Crescent & Lewis Drive

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George

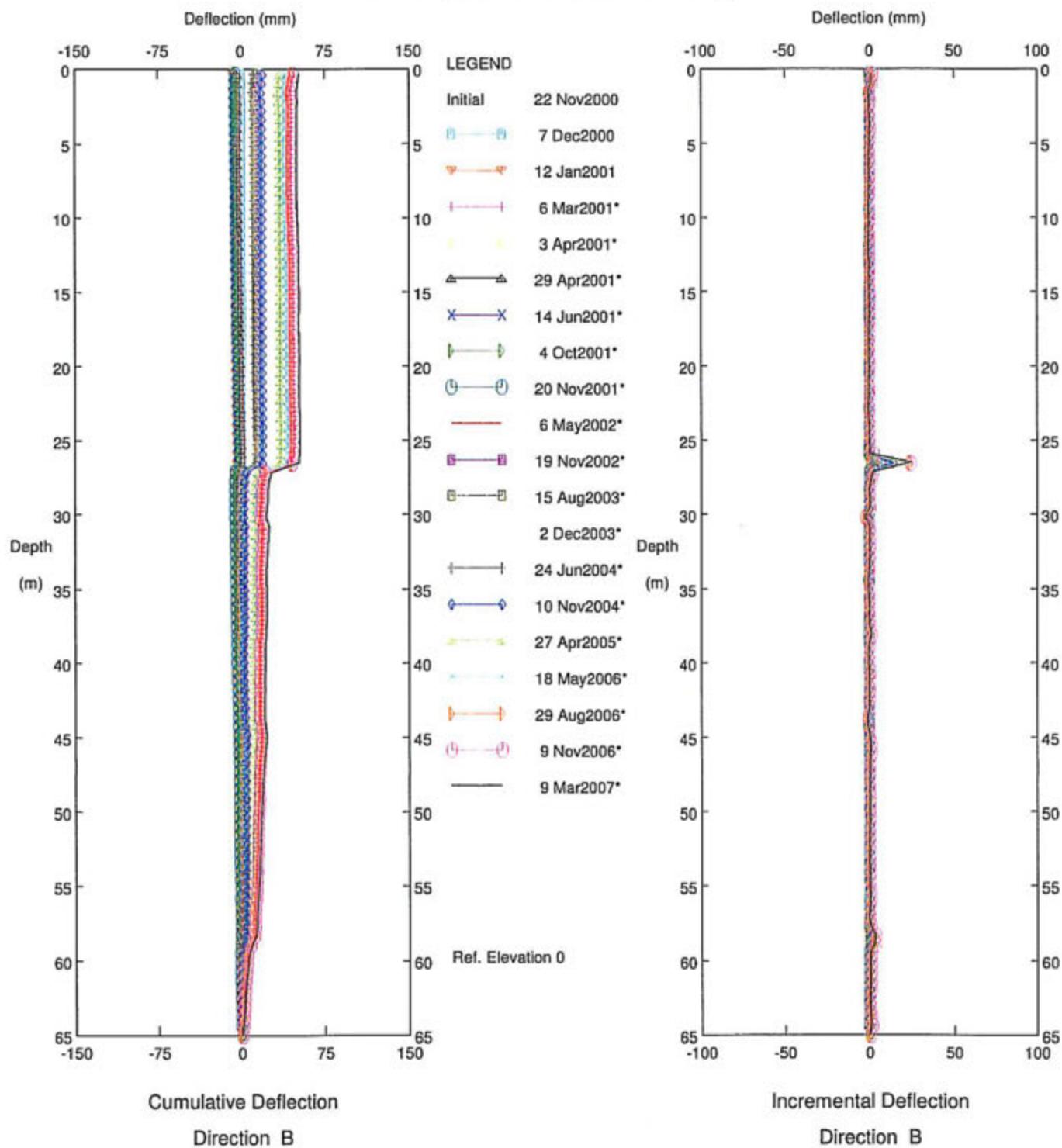


KX04397 W. Quesnel Stability Study, Inclinometer SI7

Pierce Crescent & Lewis Drive

Sets marked * include zero shift and/or rotation corrections.

AMEC Earth & Environmental - Pr. George



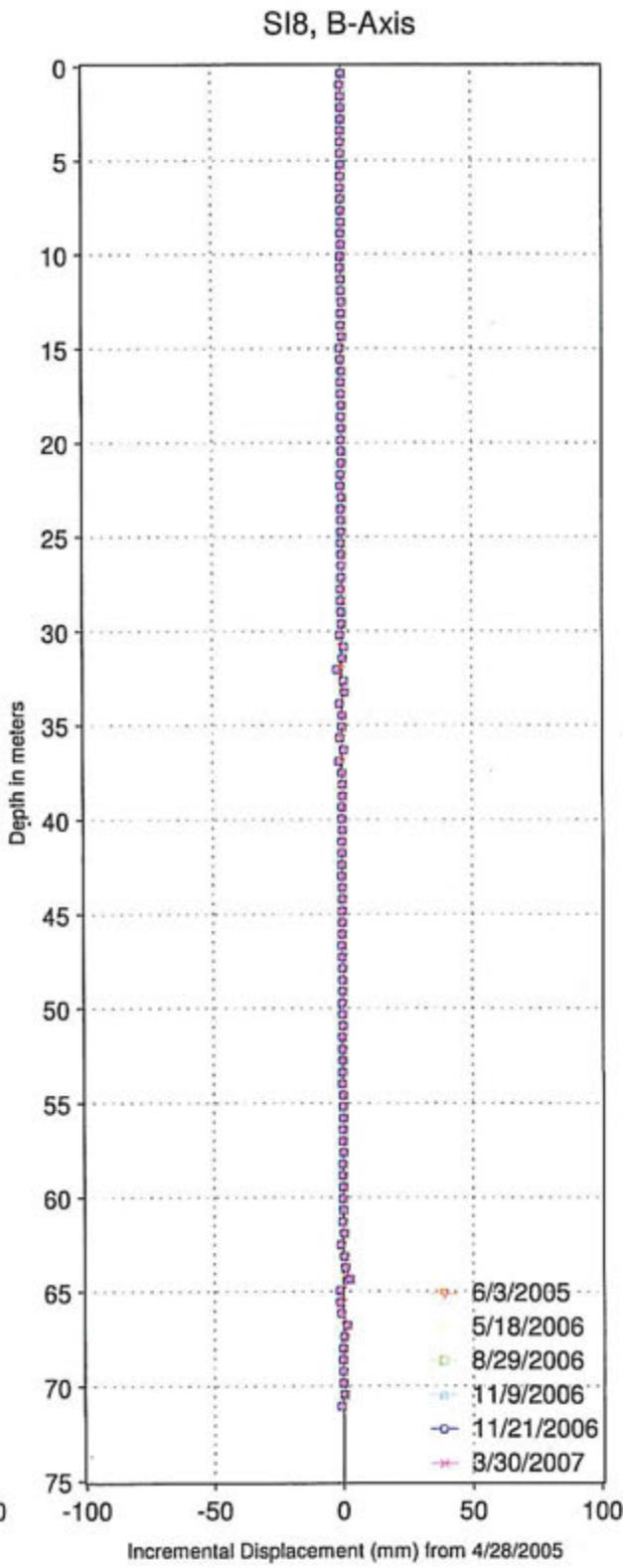
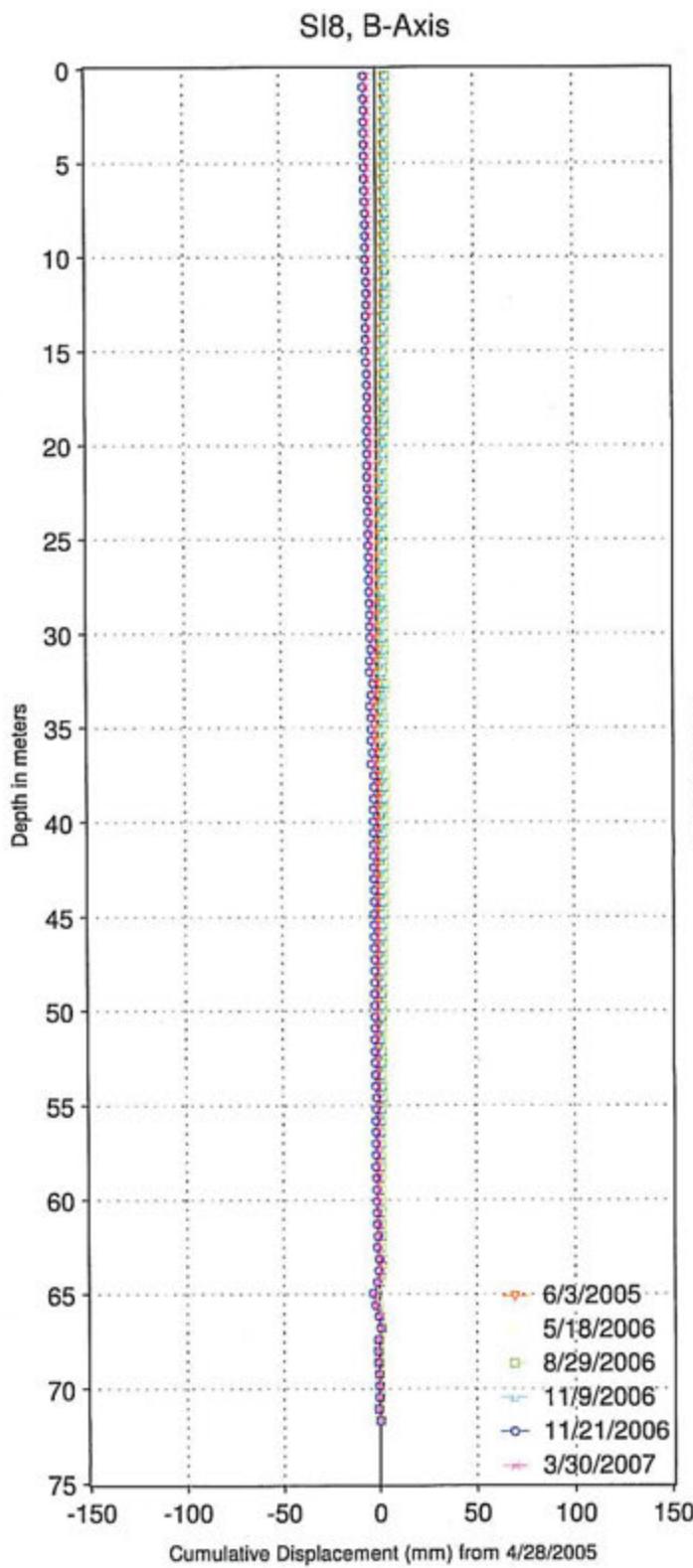
KX04397 W. Quesnel Stability Study, Inclinometer SI7

Pierce Crescent & Lewis Drive

Sets marked * include zero shift and/or rotation corrections.

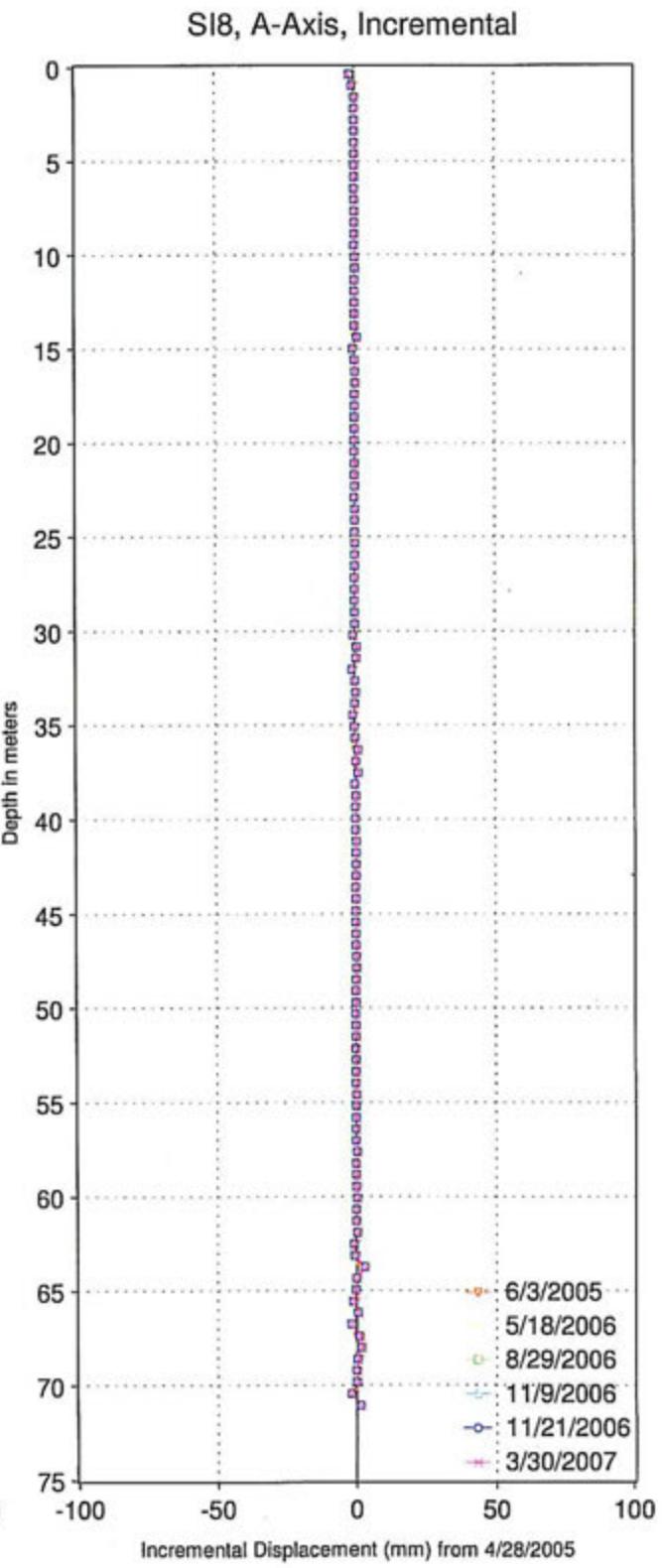
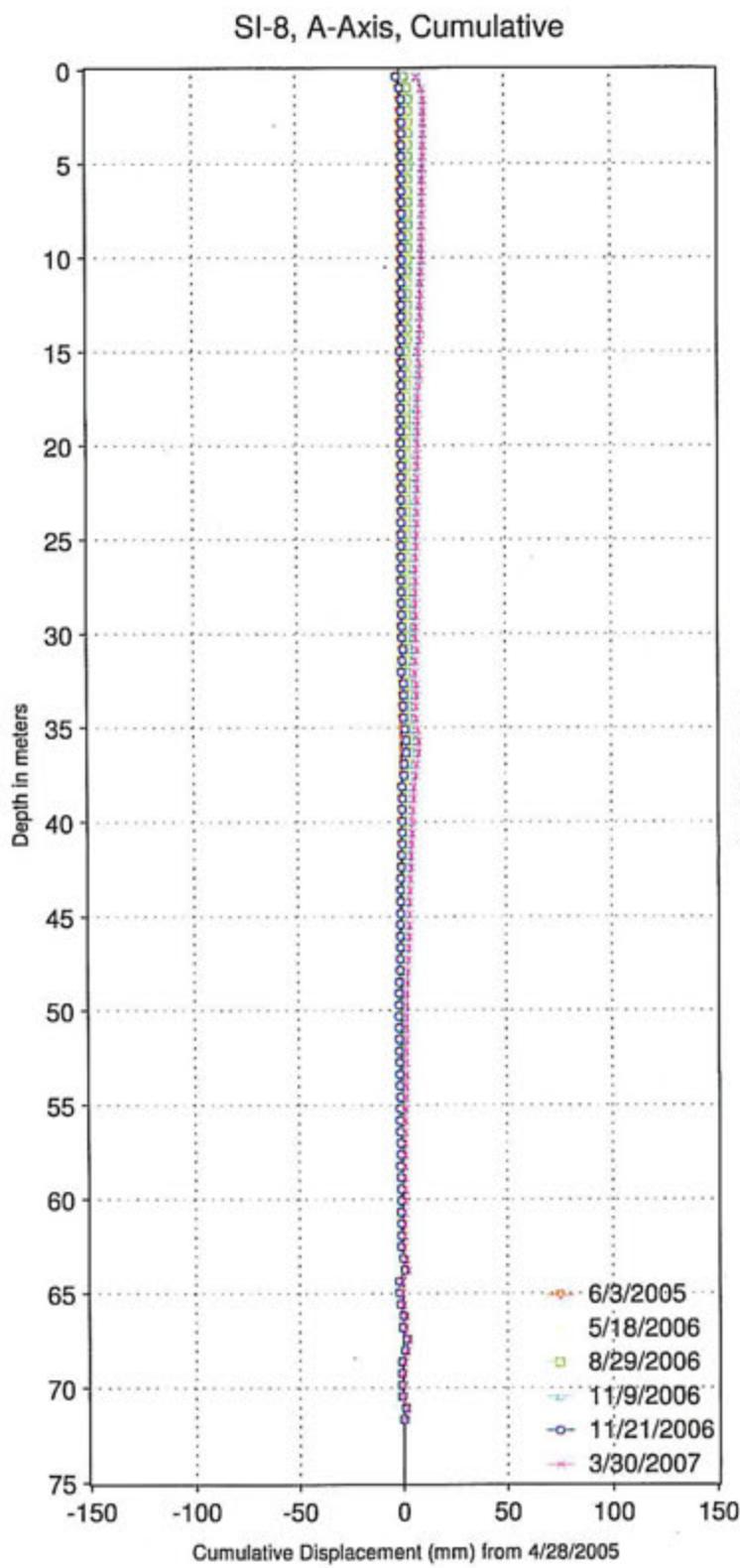
KX04397

WEST QUESNEL LAND STABILITY Study
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI8



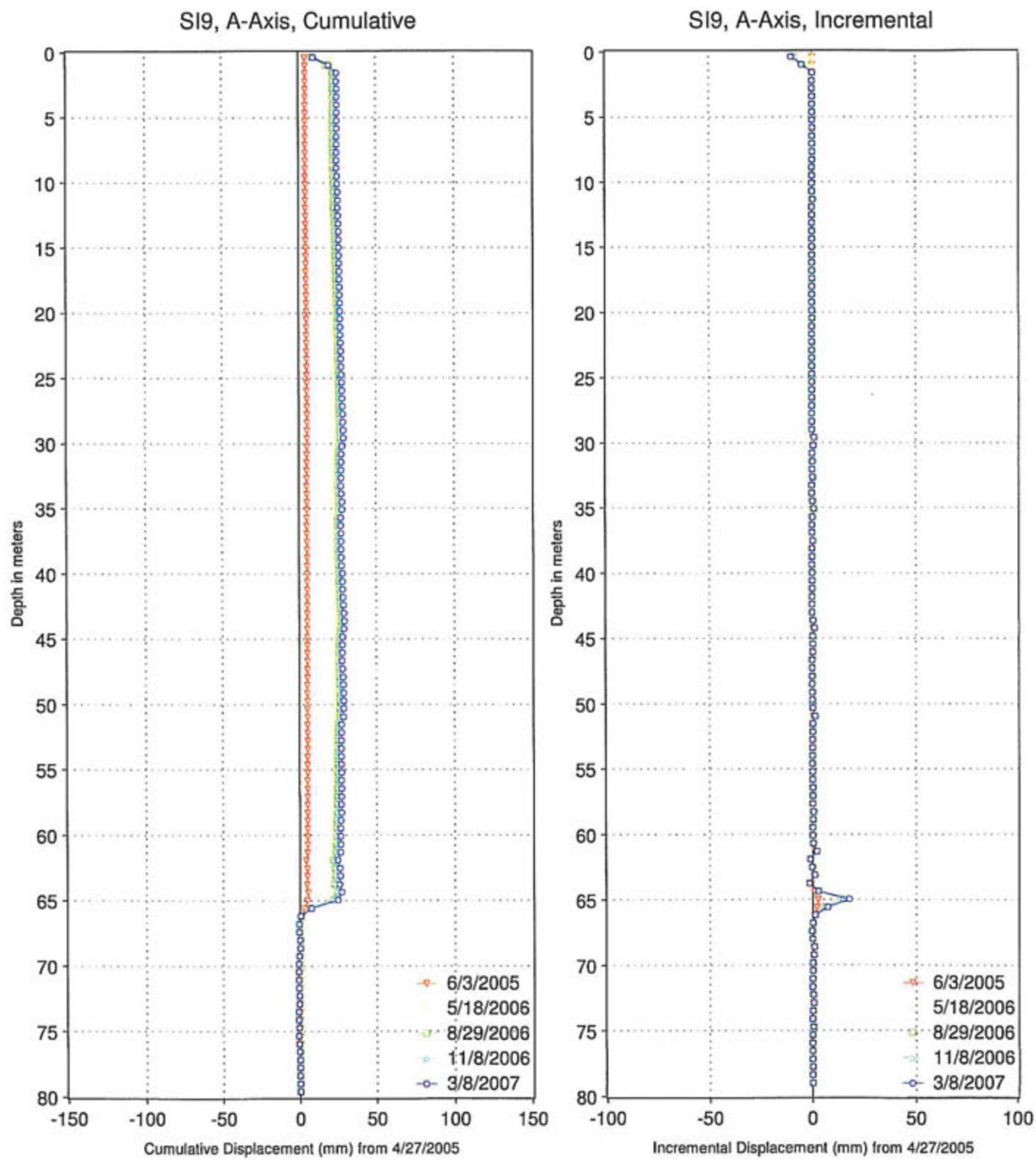
KX04397

WEST QUESNEL LAND STABILITY MONITORING
CUMMULATIVE & INCREMENTAL DISPLACEMENTS
SI-8



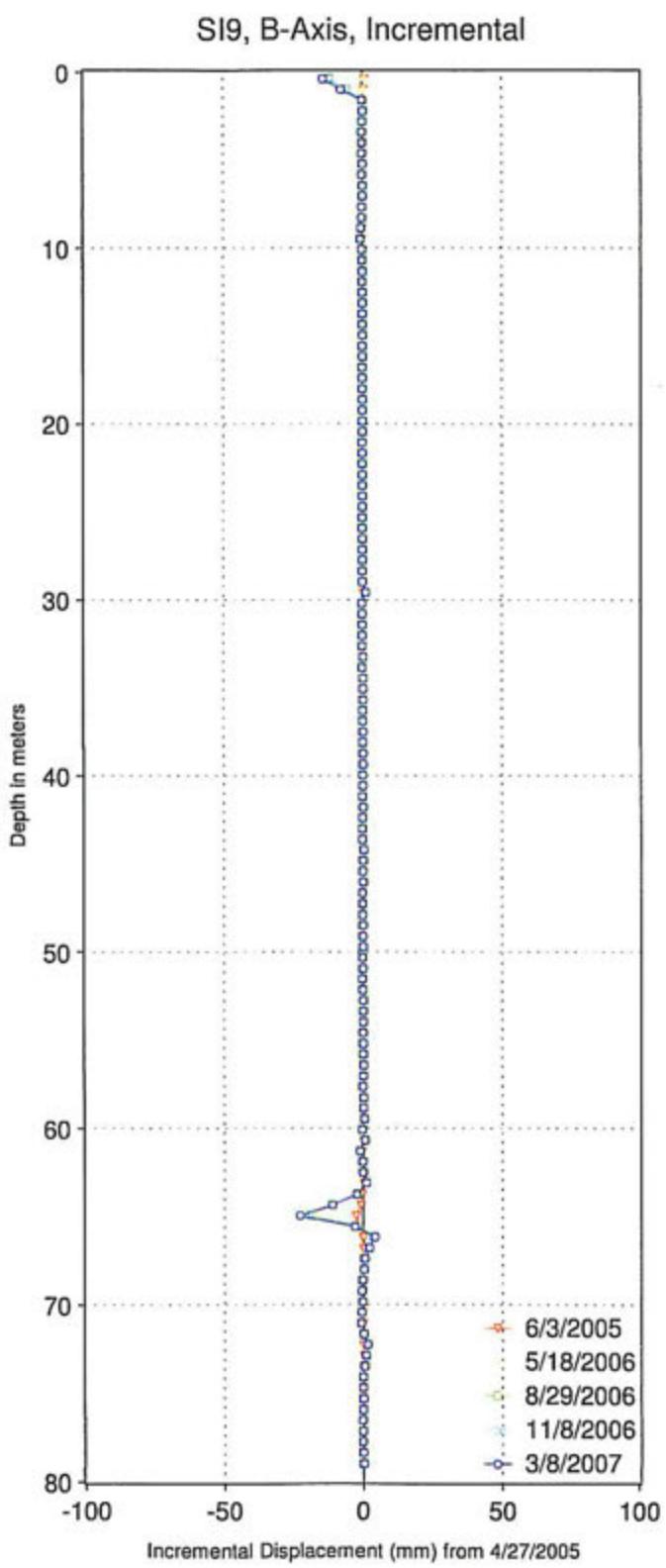
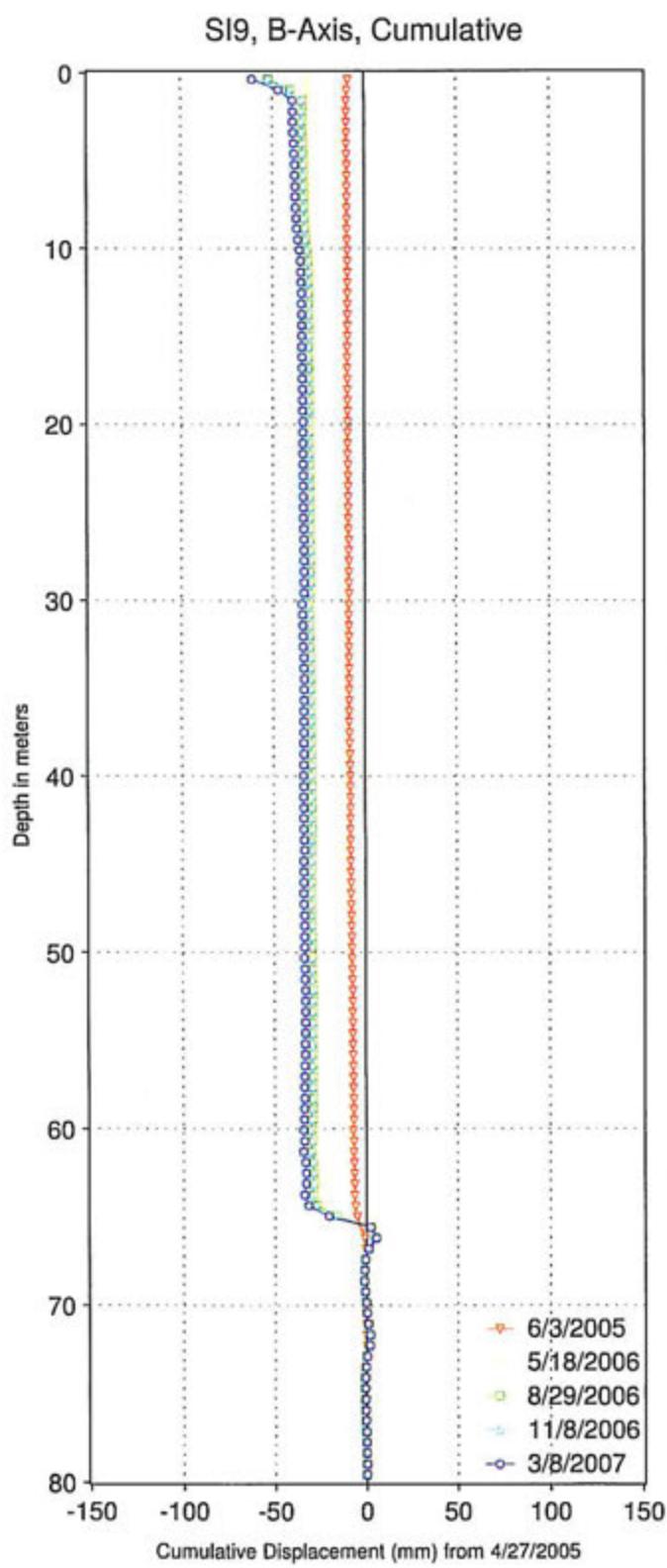
KX09397

WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI9



KX09397
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI9

amec



KX04397

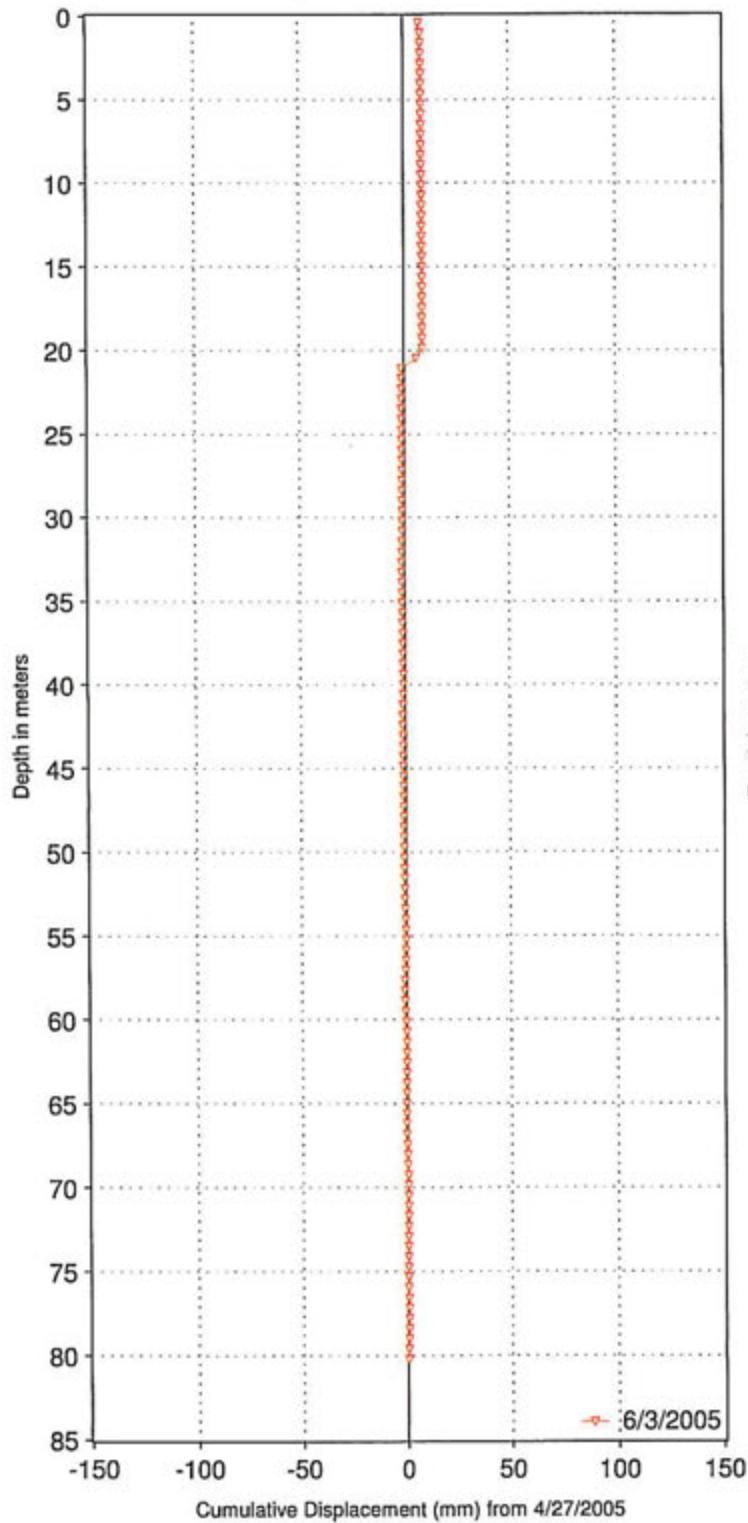
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI10

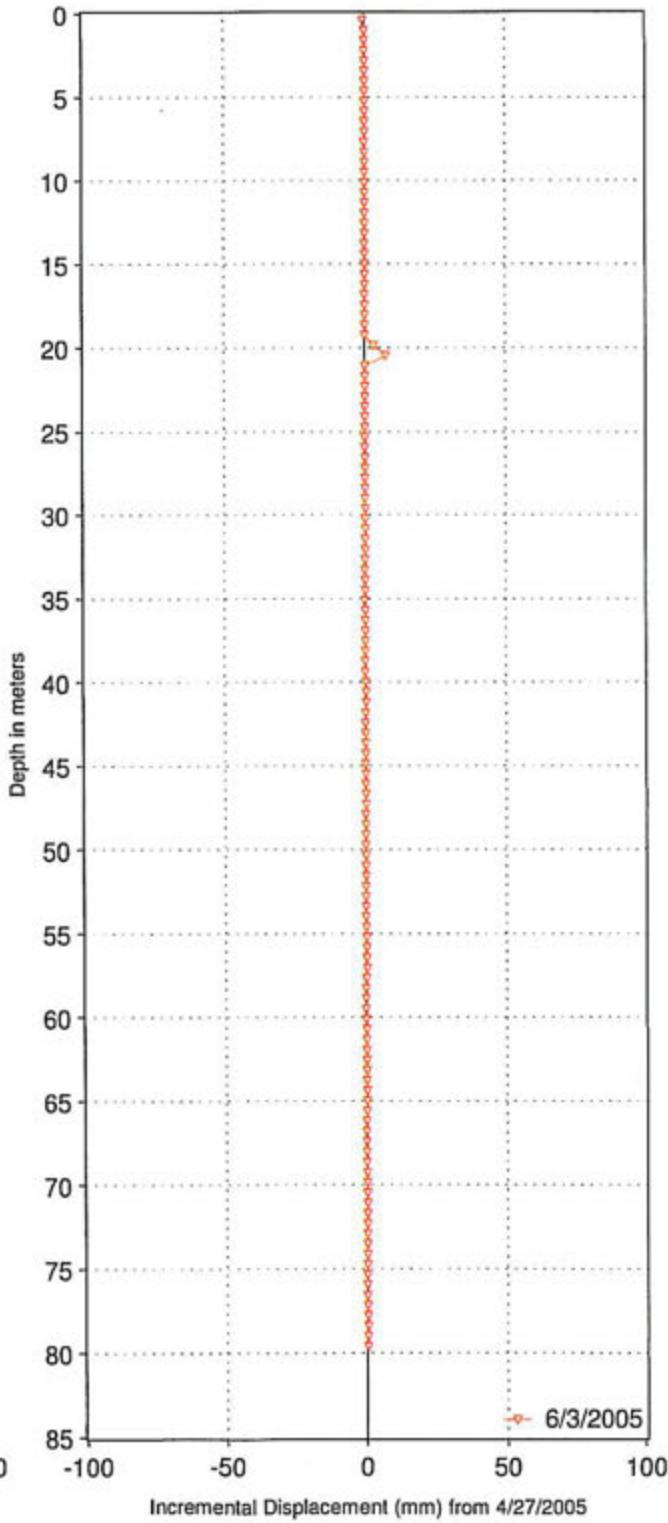
SI sheared at 69° between 06/03/2005 and 05/19/2006



SI10, A-Axis, Cumulative



SI10, A-Axis, Incremental



KX04397

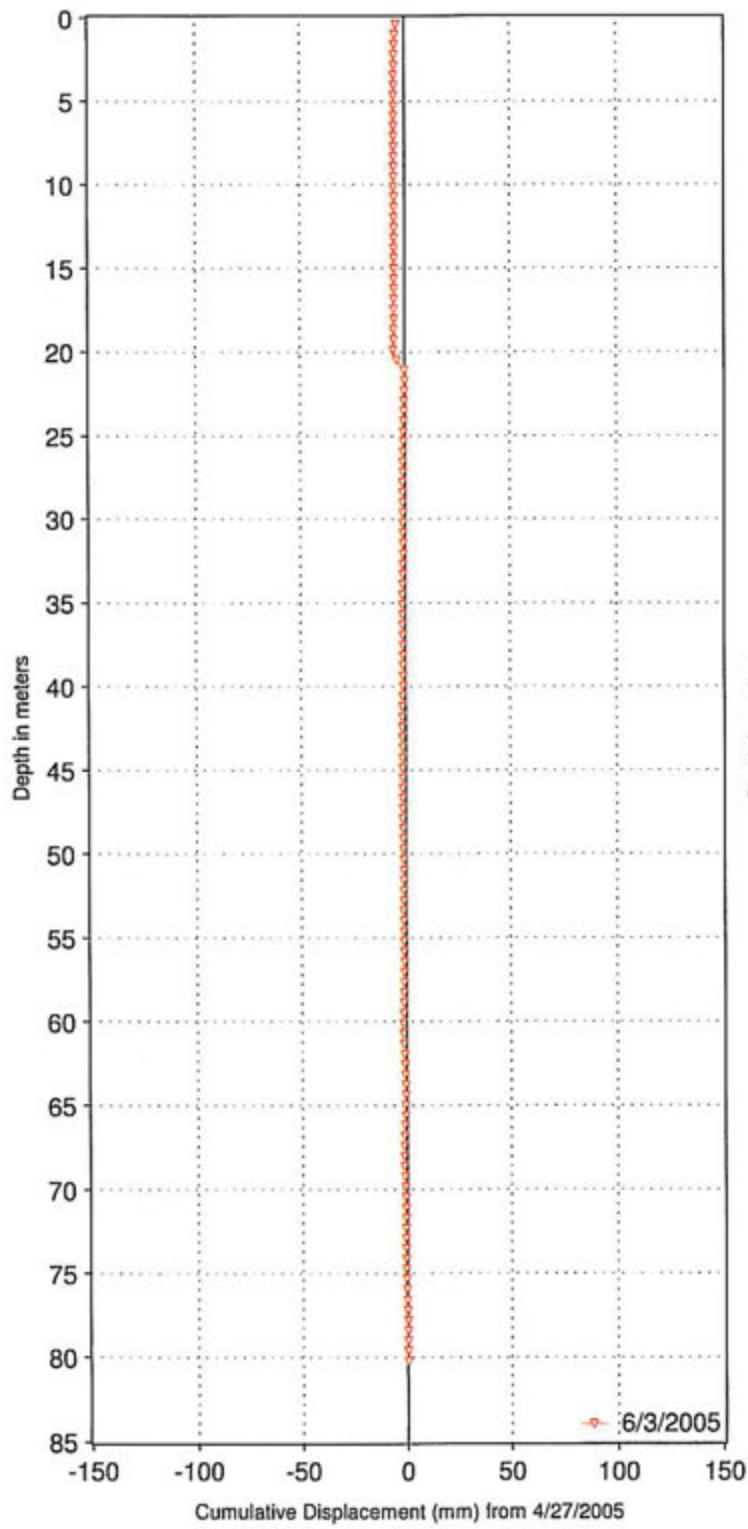
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI10

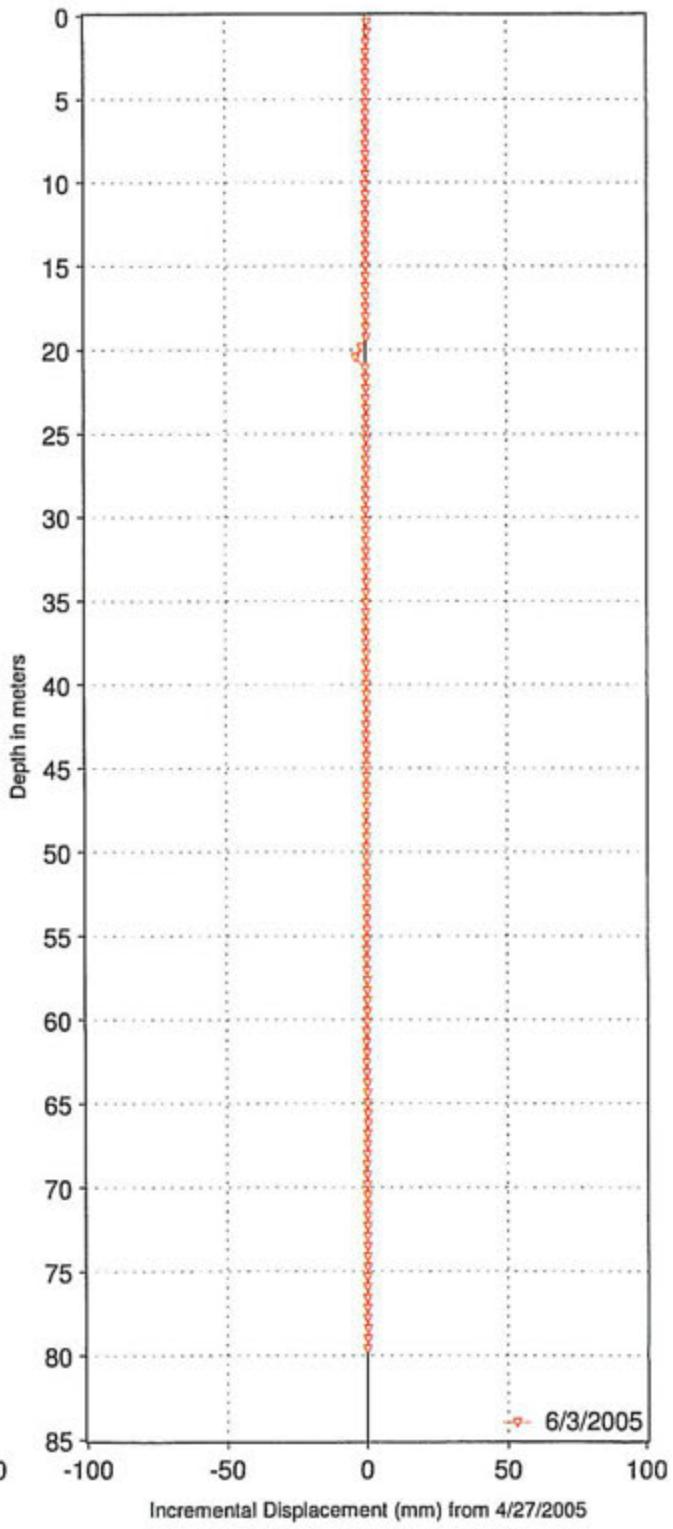
SI blocked at 21.0 m between 06/03/2005 and 05/19/2006



SI10, B-Axis, Cumulative



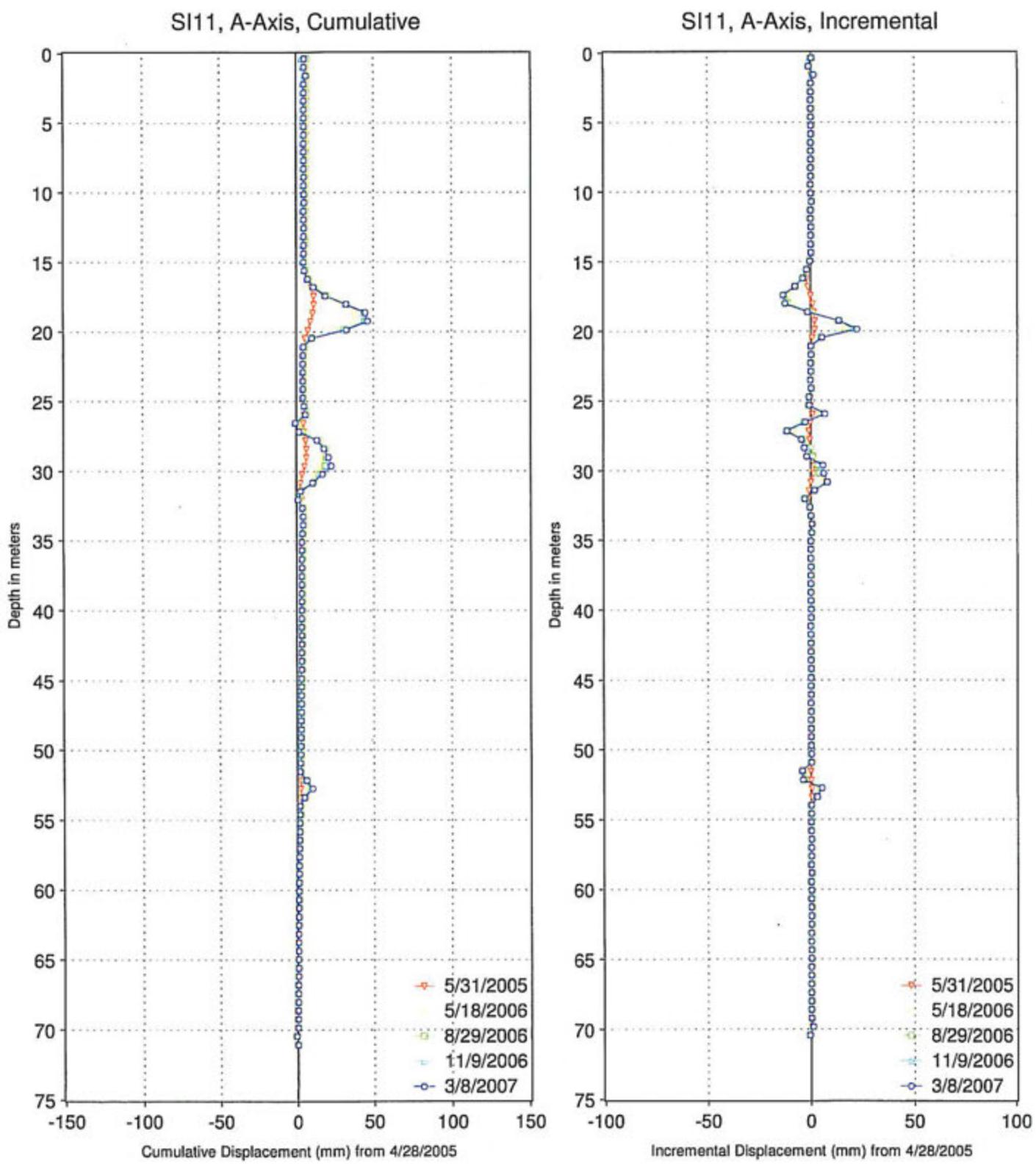
SI10, B-Axis, Incremental



KX04397

WEST QUESNEL LAND STABILITY STABILITY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI11

amec

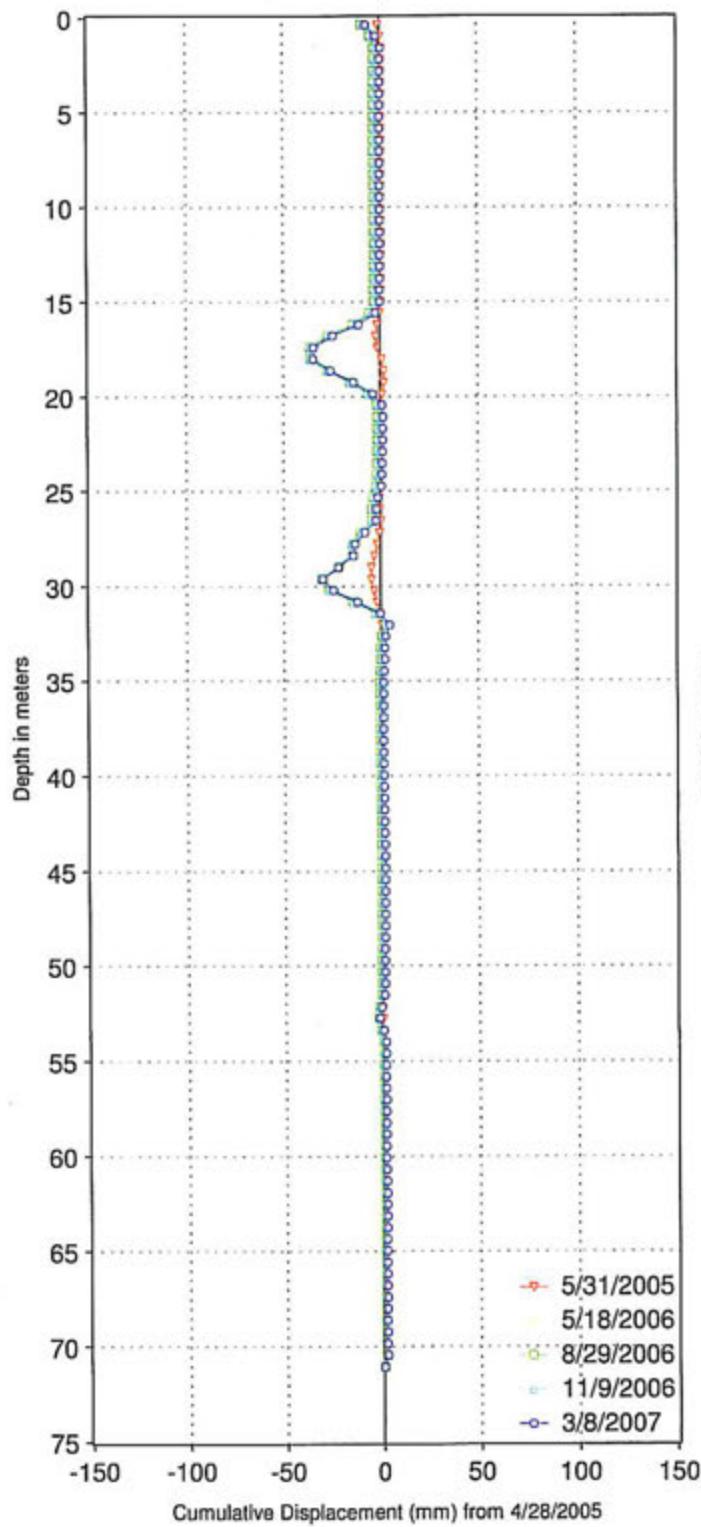


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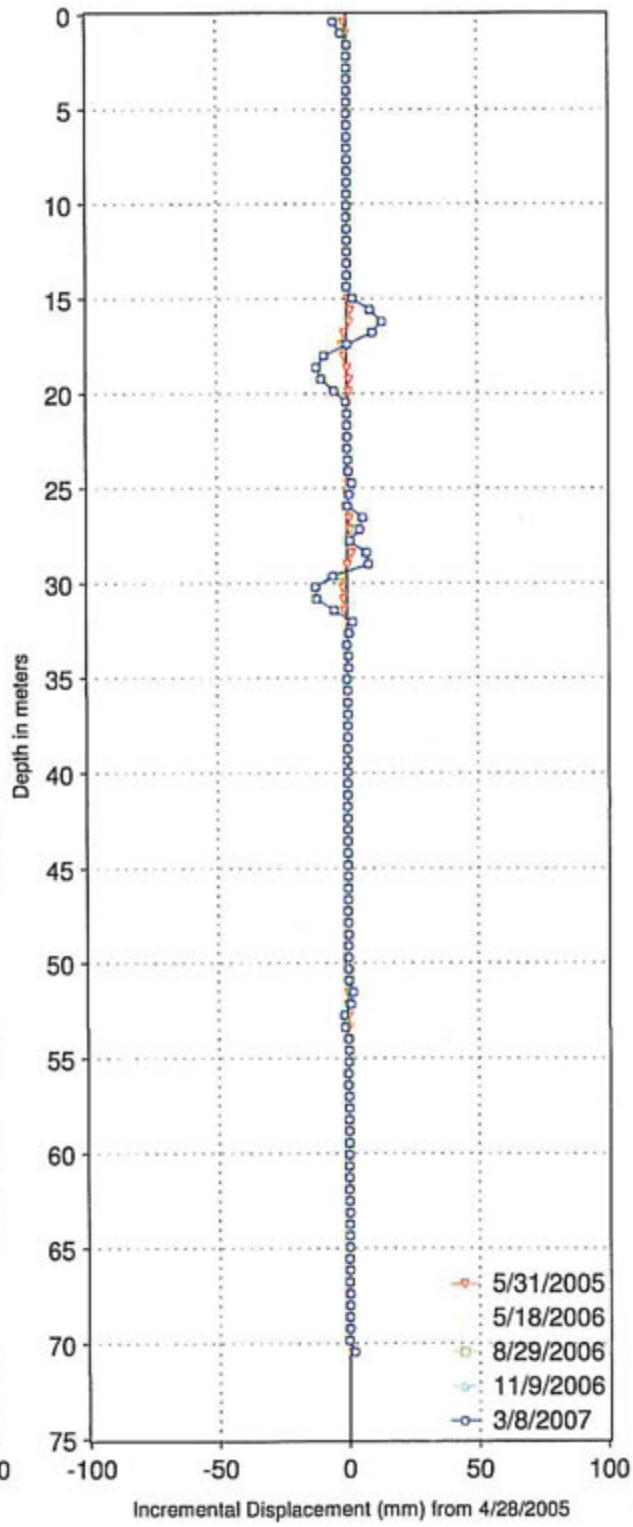
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI11



SI11, B-Axis, Cumulative



SI11, B-Axis, Incremental

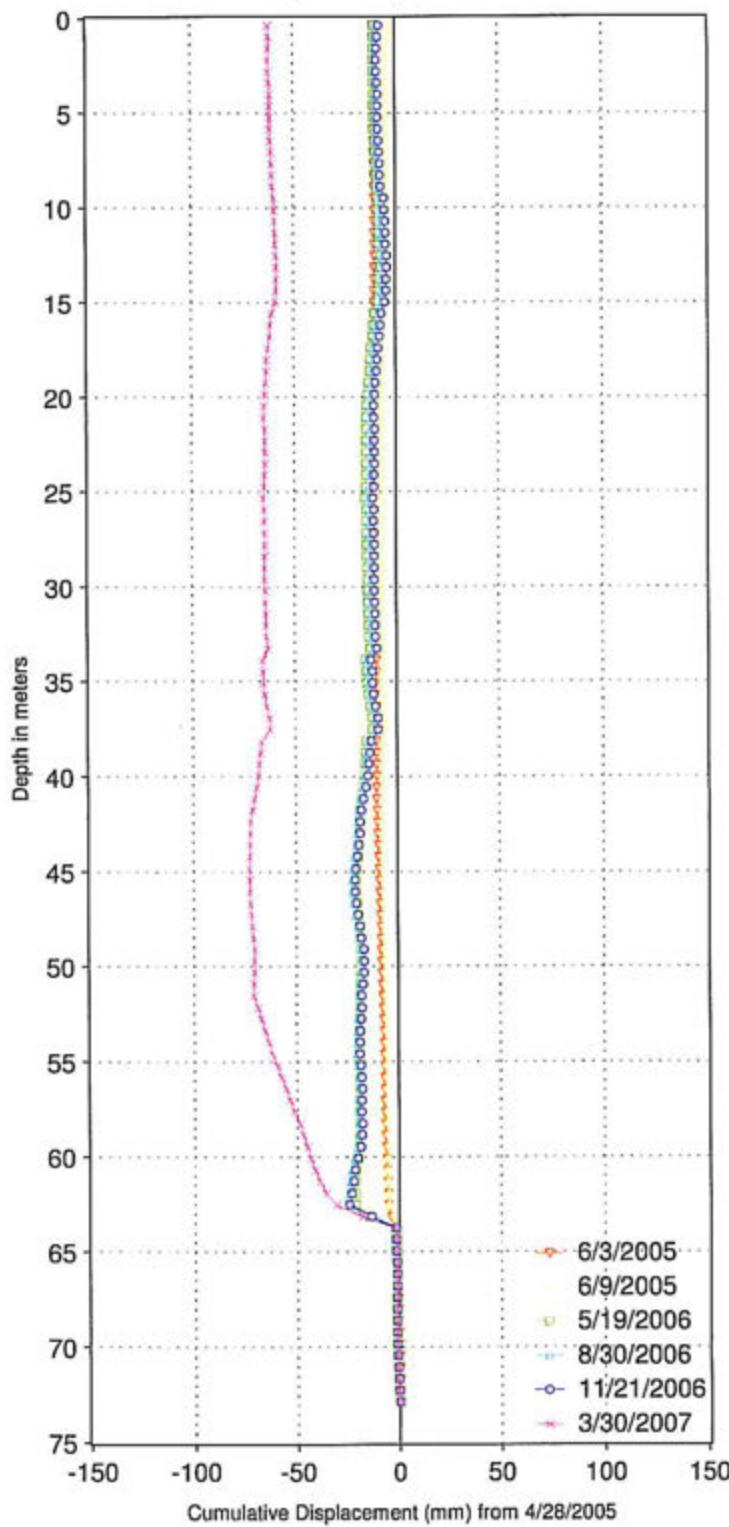


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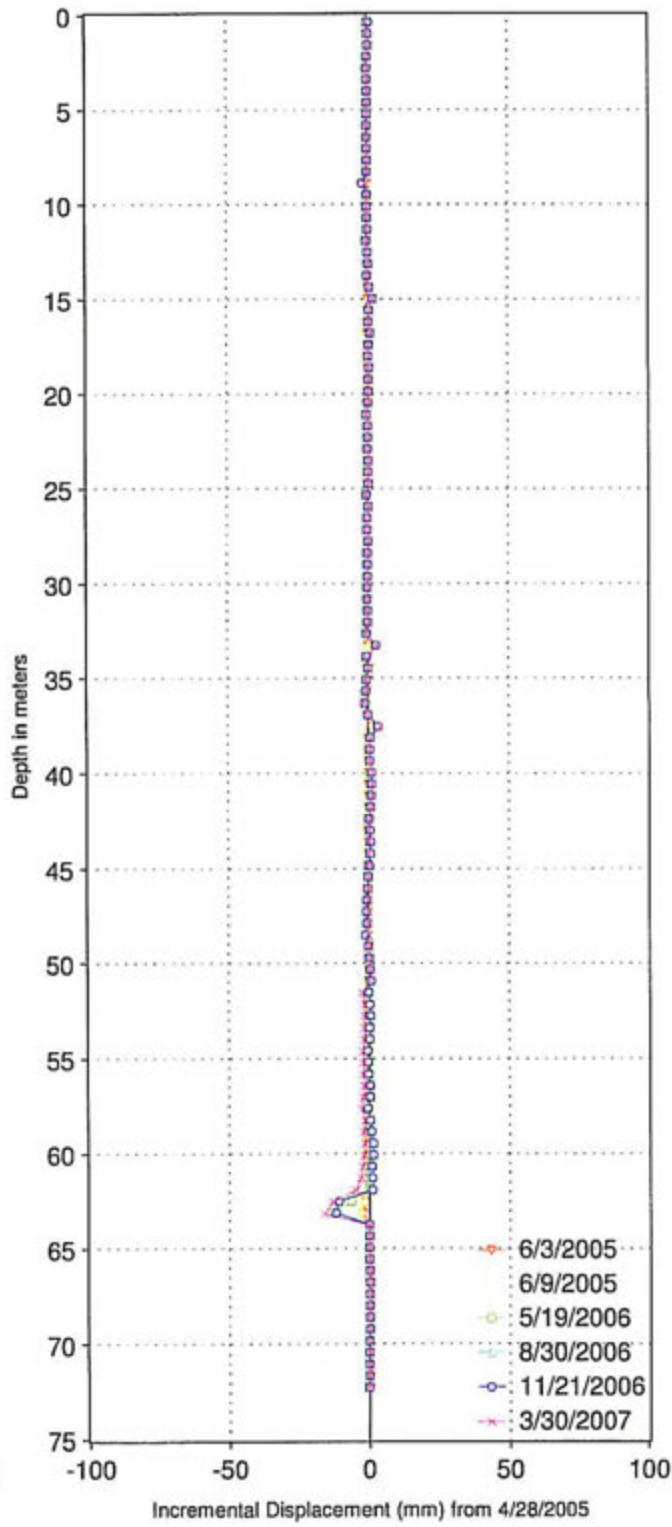
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI12



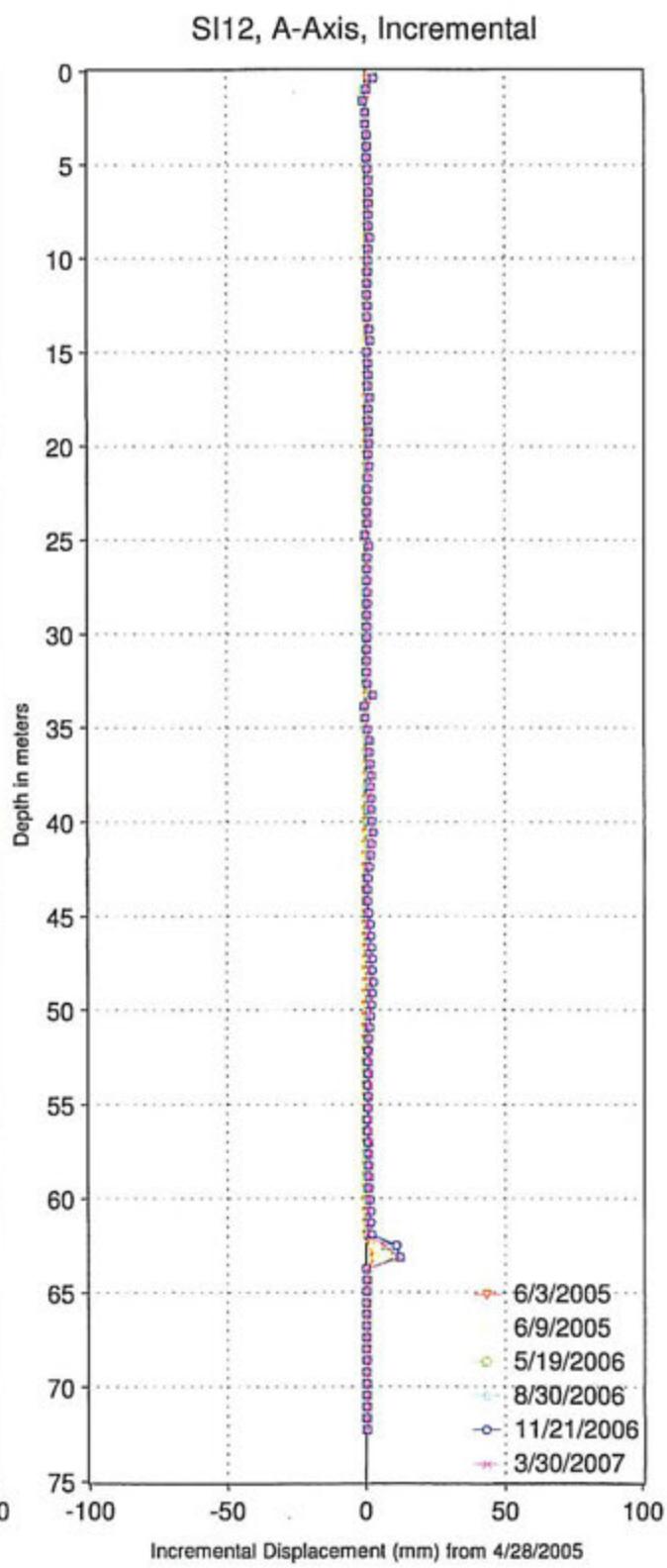
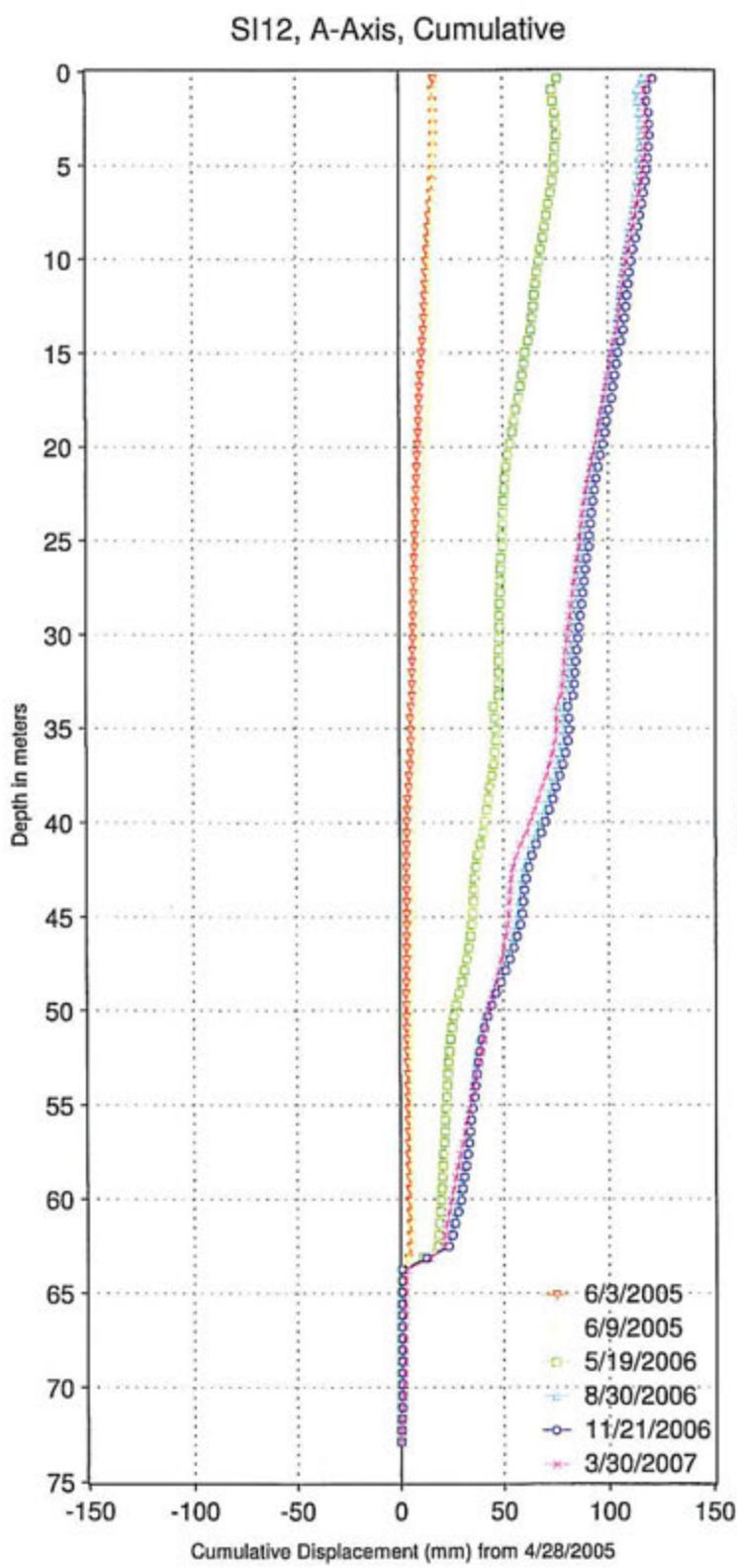
SI12, B-Axis, Cumulative



SI12, B-Axis, Incremental



KX04397
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI12

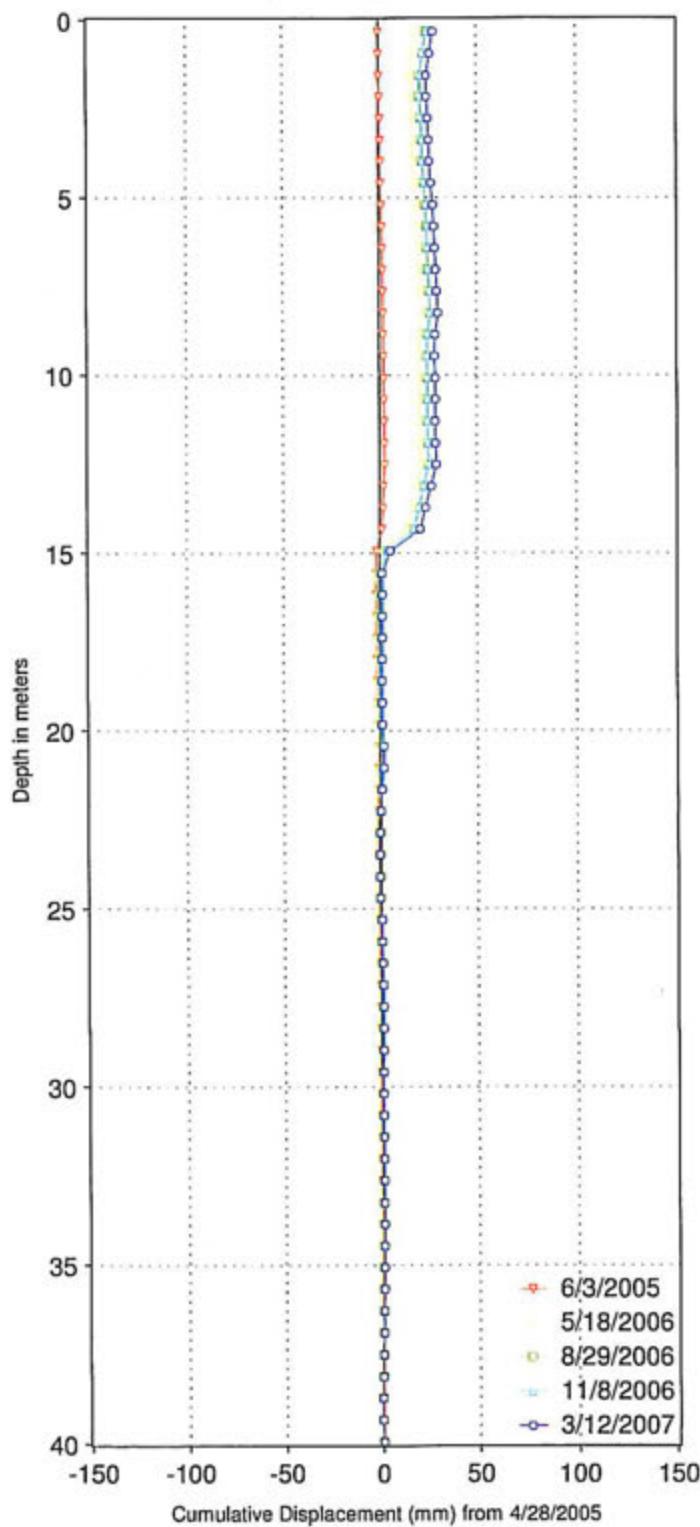


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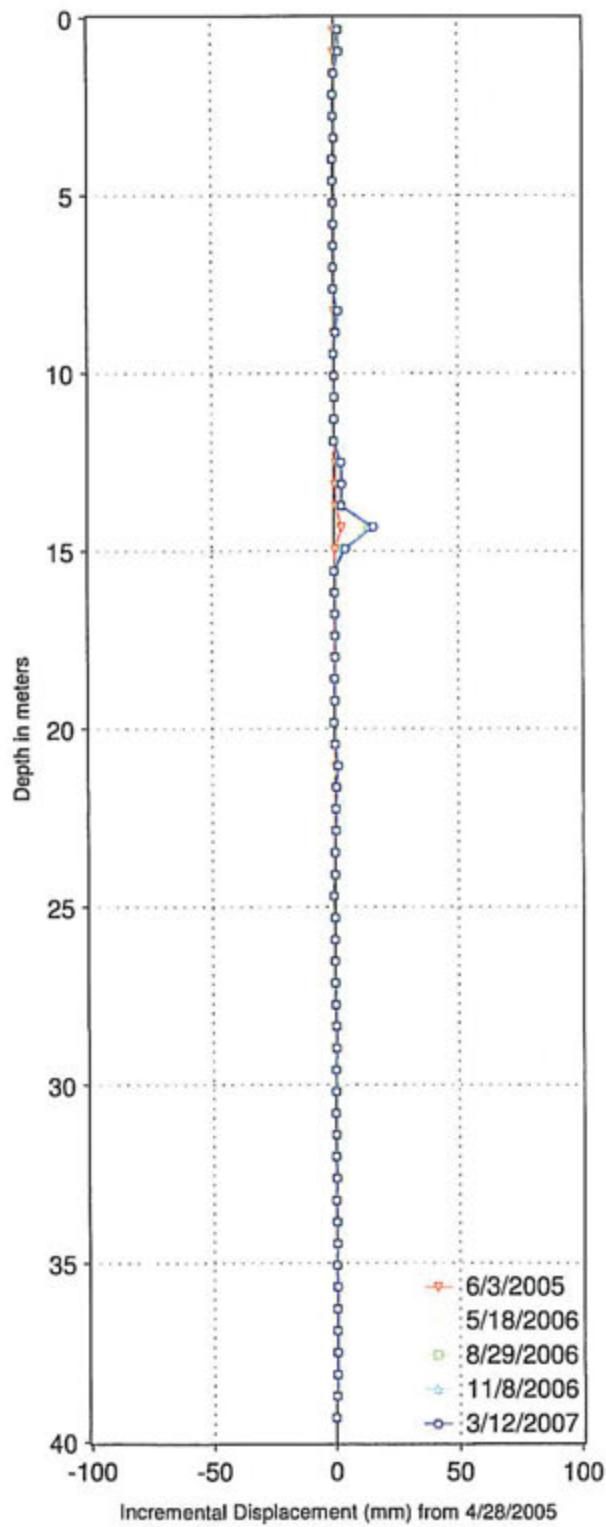
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI13



SI13, A-Axis, Cumulative



SI13, A-Axis, Incremental

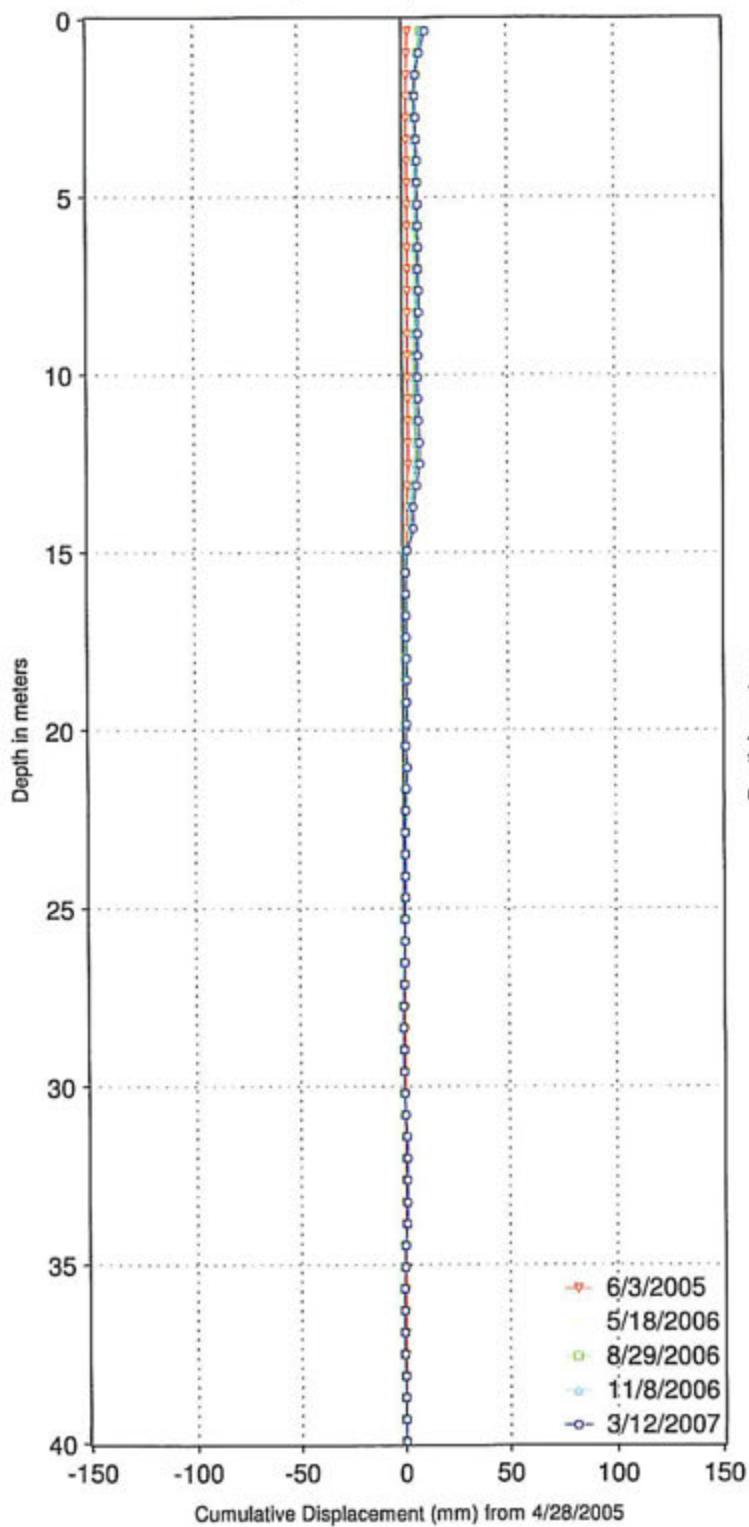


KX04397

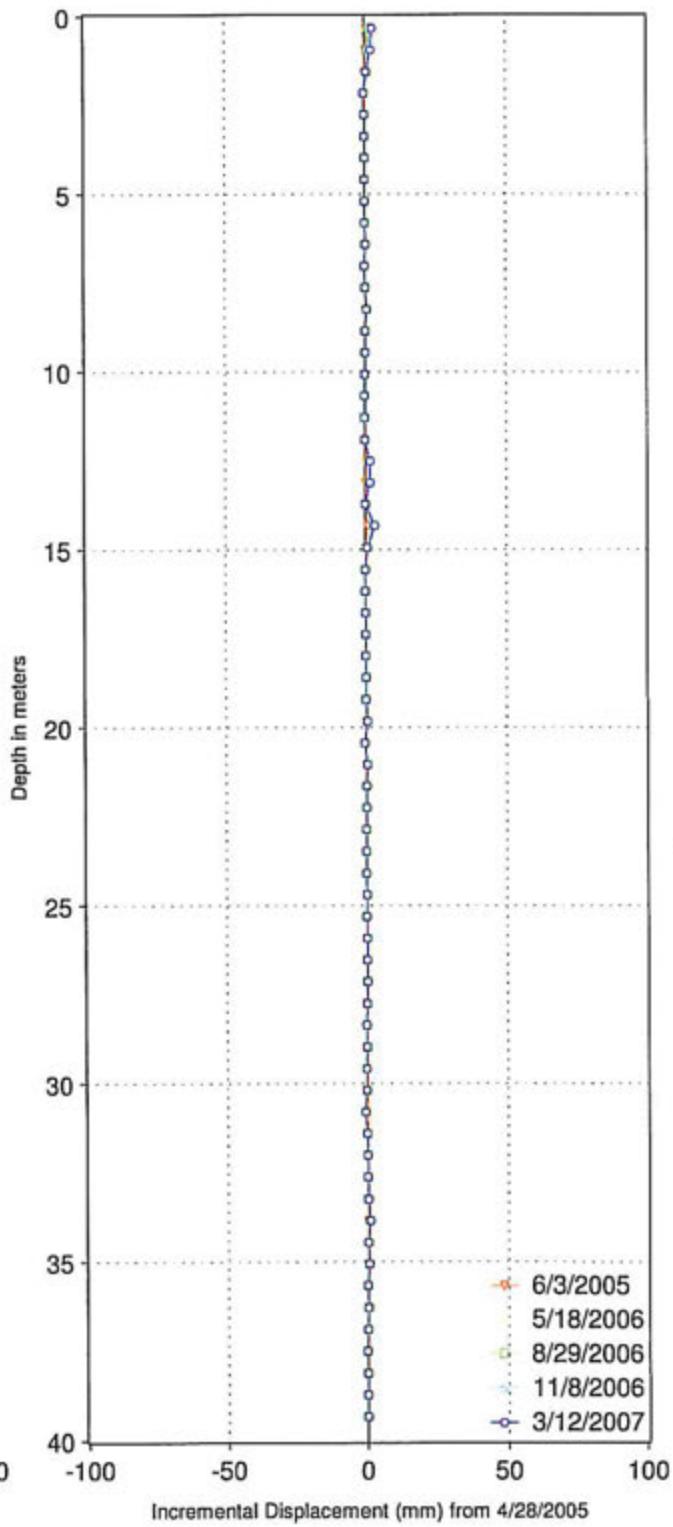
WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS
SI13



SI13, B-Axis, Cumulative



SI13, B-Axis, Incremental



KX04397

WEST QUESNEL STABILITY STUDY

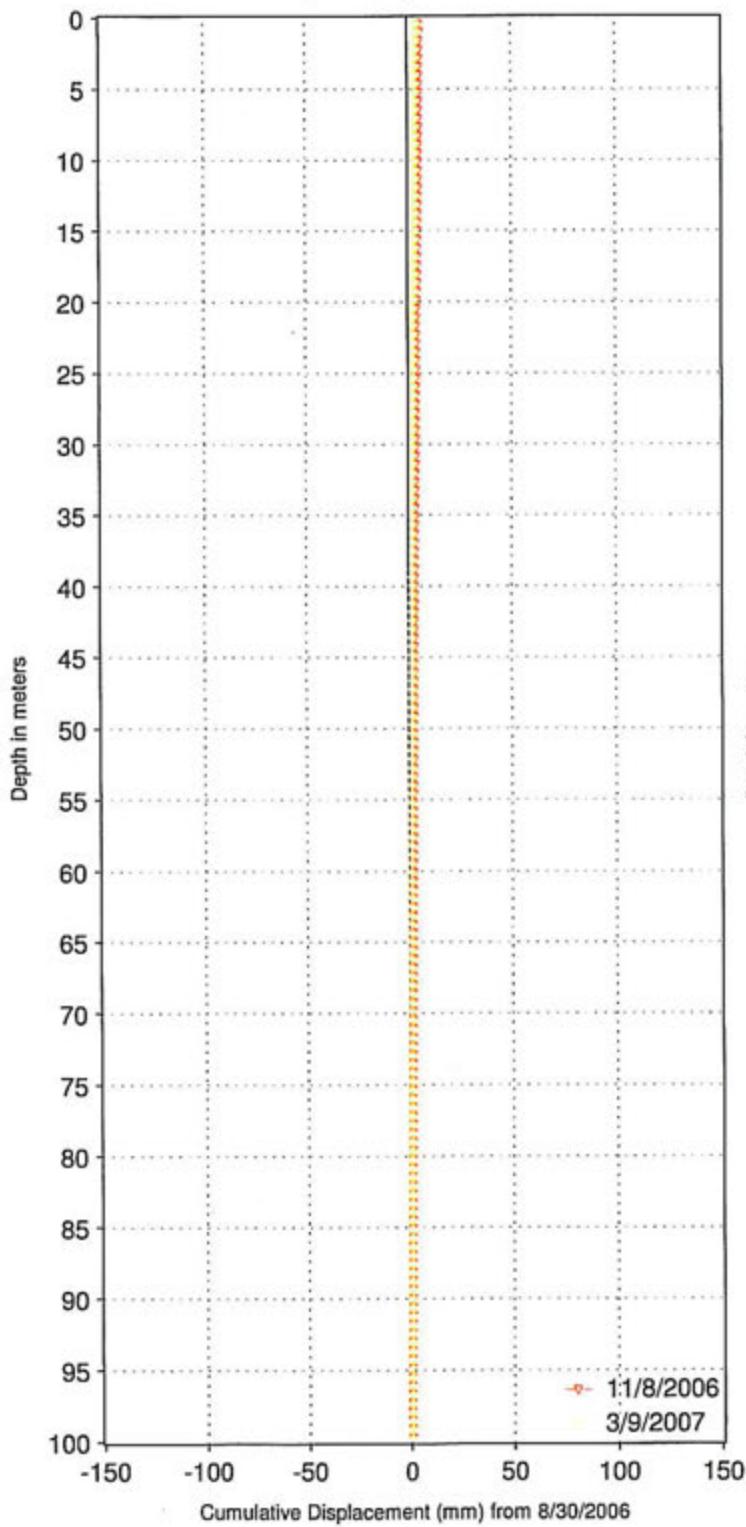
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI14

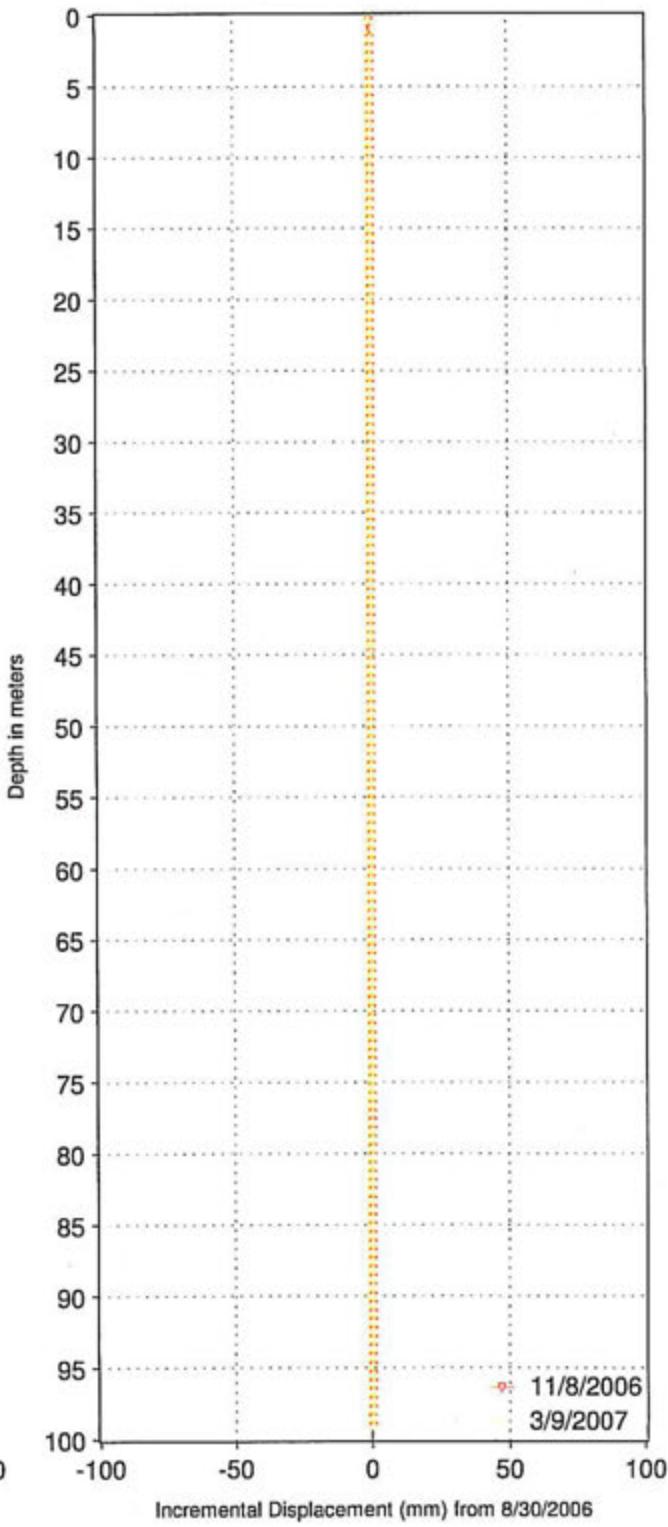


INITIAL READINGS

SI14, A-Axis, Cumulative



SI14, A-Axis, Incremental



KX04397

WEST QUESNEL STABILITY STUDY

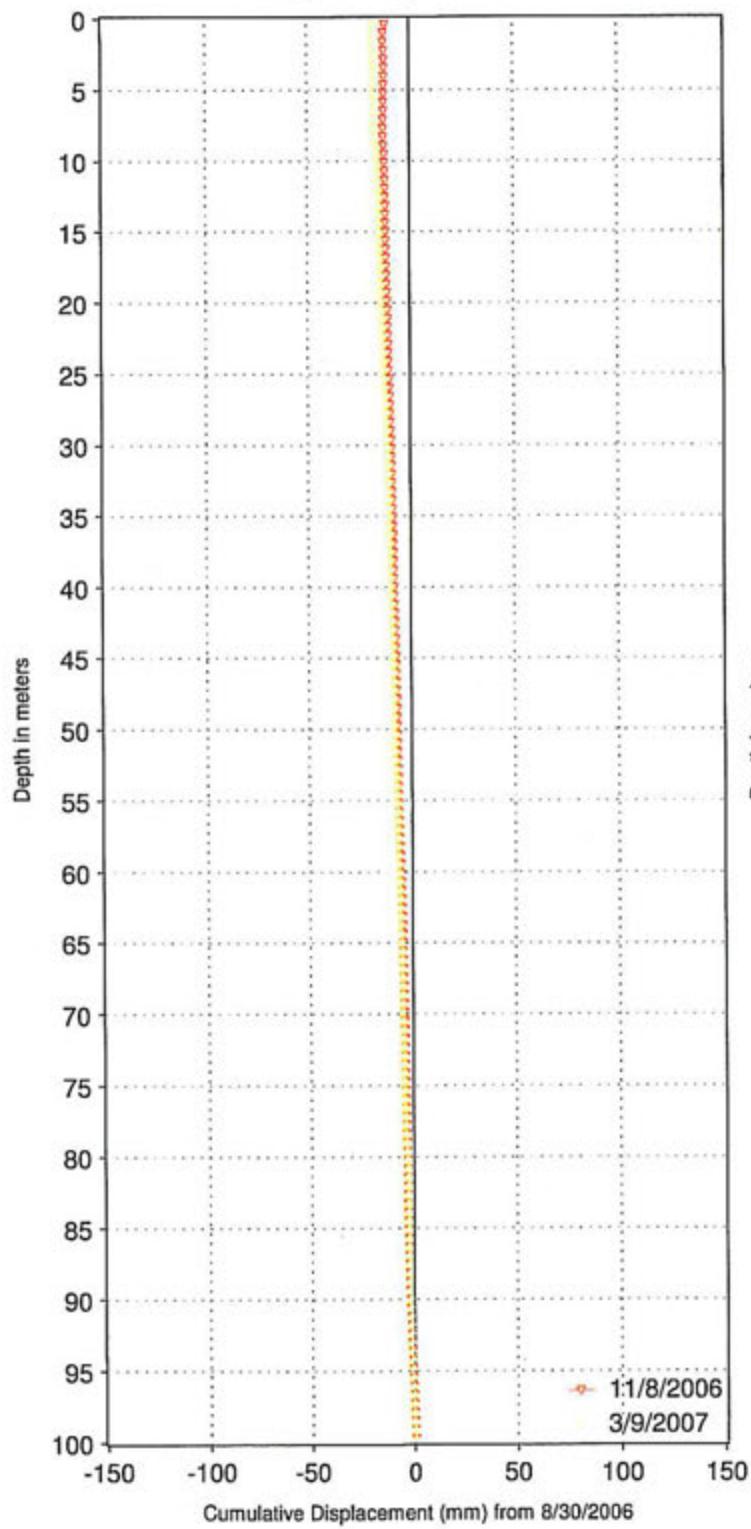
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI14

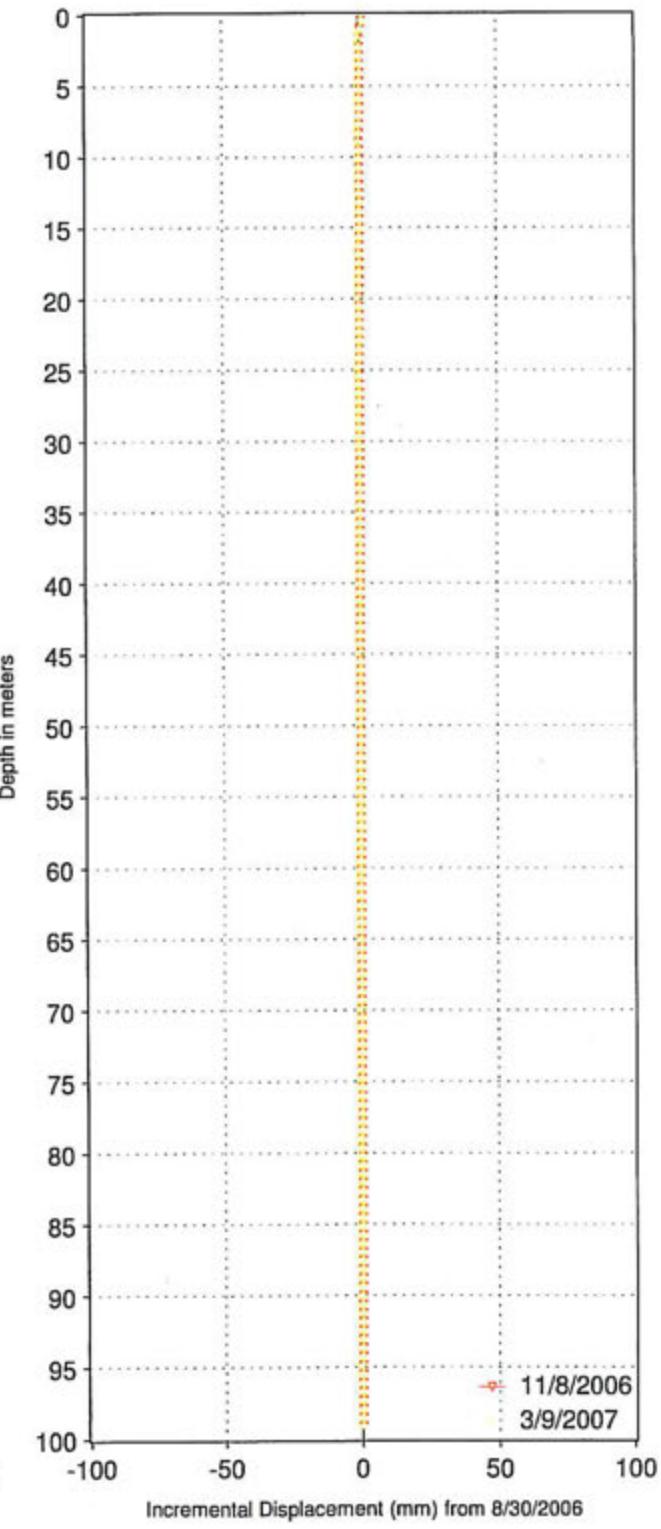


INITIAL READINGS

SI14, B-Axis, Cumulative



SI14, B-Axis, Incremental



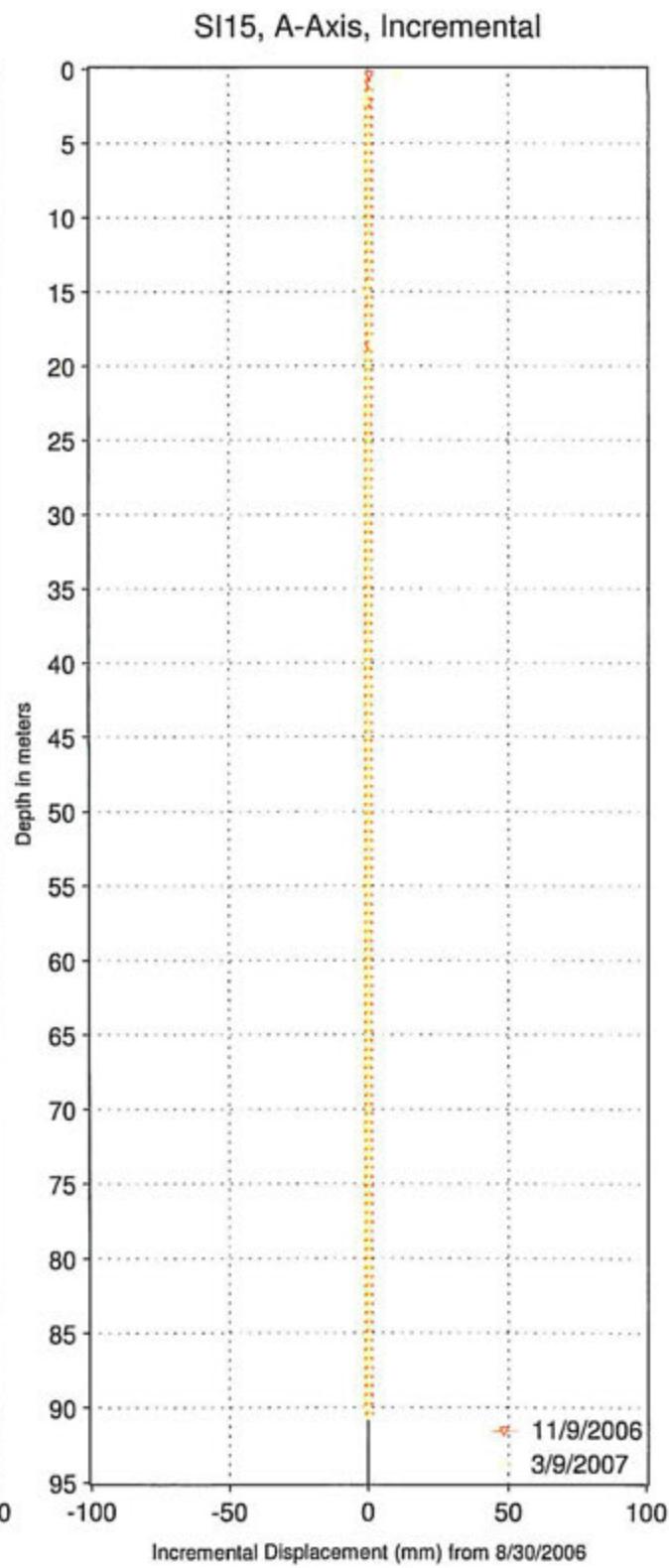
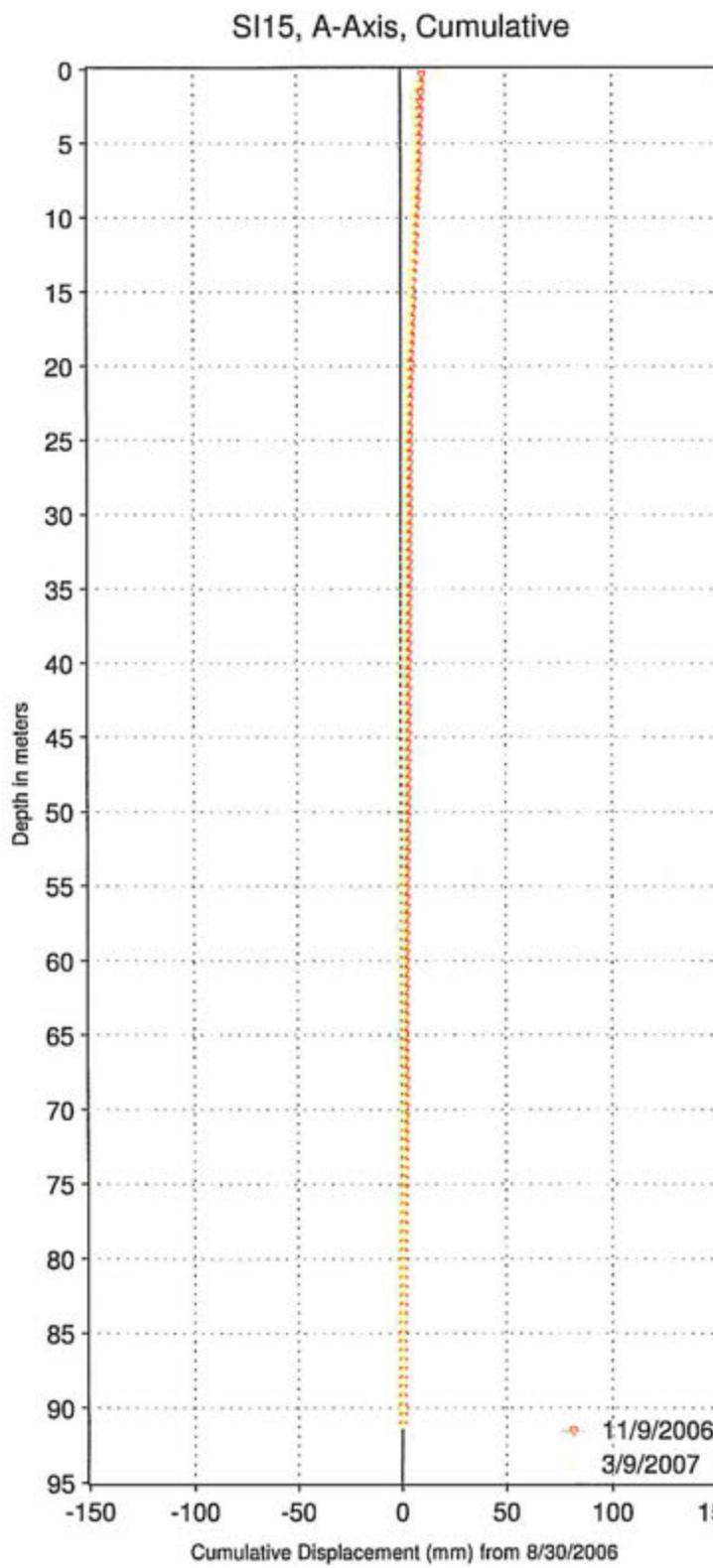
KX04397

WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI15



INITIAL READINGS



KX04397

WEST QUESNEL LAND STABILITY STUDY
CUMULATIVE & INCREMENTAL DISPLACEMENTS

SI15



INITIAL READINGS

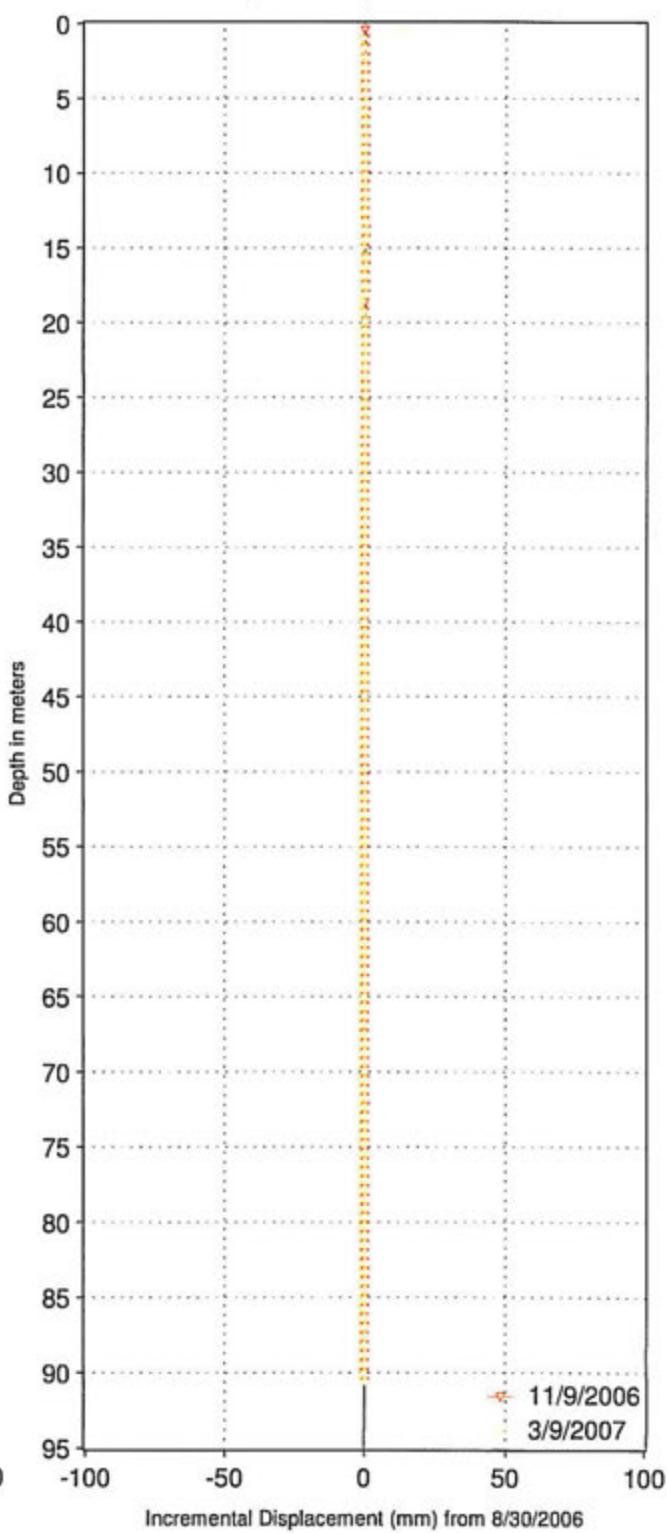
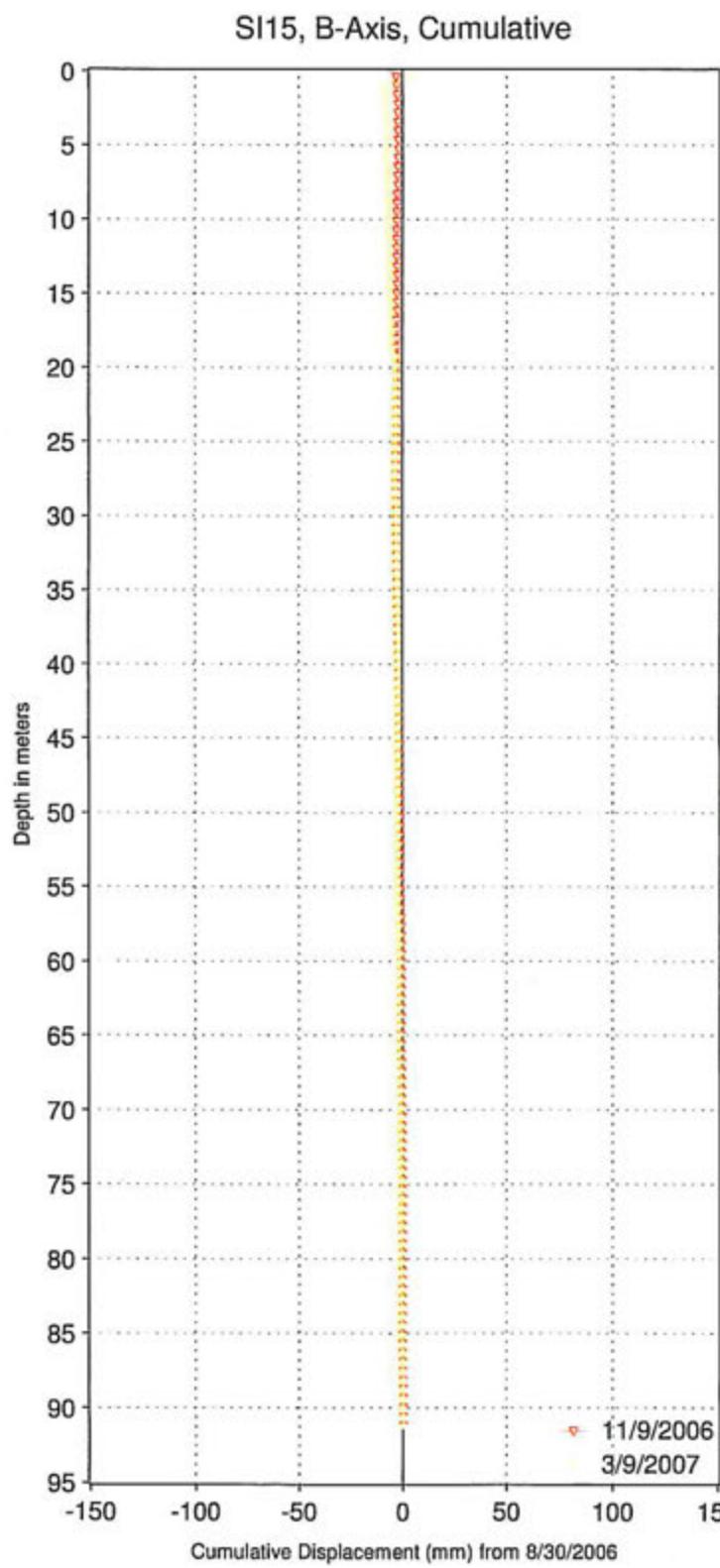


Table H1: Standpipe and Vibrating Wire Piezometer Installation Details

Installation No.	Location	Installation Type	Date Completed	Screen or Tip Depth (m) ¹	Type of Casing Protector
BH-2	Avery Lane	Standpipe	02 Nov. 2001	54.2-60.2	Stick-up
VWP-3	Abbott Drive	Vibrating Wire Piezometer (VWP)	15 Nov. 2001	38.0	Stick-up
BH-4	Voyager School	Standpipe	06 Dec. 2001	39.6-39.9	Stick-up
VWP-4	Voyager School	VWP	06 Dec. 2001	49.0	Stick-up
BH-6A1	Dixon Street	Standpipe	18 Dec. 2001	44.0-45.2	Flush-Mount
BH-6A2	Dixon Street	Standpipe	18 Dec. 2001	26.4-27.8	Flush-Mount
PW-01	Flamingo Street	Standpipe	16 Sep. 2003	29.9-48.2	Stick-up
PW-02	Abbott Drive & Bettcher Street	Standpipe	18 Sep. 2003	29.8-55.0	Stick-up
BH-2A	Bettcher Street	Standpipe	07 Aug. 2003	86.9-88.4	Flush-Mount
BH-2B	Bettcher Street	Standpipe	07 Aug. 2003	47.7-48.2	Flush-Mount
BH-2C	Bettcher Street	Standpipe	08 Aug. 2003	25.3-25.9	Flush-Mount
BH-3A	Flamingo Street	Standpipe	24 Jul. 2003	49.9-50.5	Flush-Mount
BH-3B	Flamingo Street	Standpipe	22 Jul. 2004	38.7-40.2	Flush-Mount
BH-3C	Flamingo Street	Standpipe	22 Jul. 2004	32.0-33.5	Flush-Mount
BH-4A	Flamingo Street	Standpipe	17 Jul. 2003	48.8-49.4	Flush-Mount
BH-4B	Flamingo Street	Standpipe	17 Jul. 2003	38.7-40.2	Flush-Mount
BH-4C	Flamingo Street	Standpipe	18 Jul. 2003	22.8-23.4	Flush-Mount
VWP-7A	Lewis Drive & Pierce Street	VWP	21 Jul. 2005	27.0	Stick-up
VWP-7B	Lewis Drive & Pierce Street	VWP	21 Jul. 2005	64.9	Stick-up
VWP-8A	Pinchbeck Street	VWP	15 Jun. 2005	13.1	Stick-up
VWP-8B	Pinchbeck Street	VWP	15 Jun. 2005	13.0	Stick-up
VWP-8C	Pinchbeck Street	VWP	15 Jun. 2005	28.0	Stick-up
VWP-8D	Pinchbeck Street	VWP	29 May 2006	56.2	Stick-up
VWP-9A	Patchett Ave	VWP	27 Jun. 2005	15.1	Stick-up
VWP-9B	Patchett Ave	VWP	31 May 2006	30.6	Stick-up
VWP-9C	Patchett Ave	VWP	31 May 2006	50.1	Stick-up
VWP-10A	Dawson Street	VWP	05 Jul. 2005	11.6	Stick-up
VWP-10B	Dawson Street	VWP	05 Jul. 2005	20.4	Stick-up
VWP-10C	Dawson Street	VWP	05 Jul. 2005	48.6	Stick-up
VWP-11A	Picard Ave	VWP	10 Jun. 2005	14.3	Stick-up
VWP-11B	Picard Ave	VWP	10 Jun. 2005	32.0	Stick-up
VWP-11C	Picard Ave	VWP	10 Jun. 2005	50.3	Stick-up
VWP-12A	East end of Panagrot Ave	VWP	22 Jun. 2005	18.0	Stick-up
VWP-12B	East end of Panagrot Ave	VWP	22 Jun. 2005	22.9	Stick-up
VWP-12C	East end of Panagrot Ave	VWP	22 Jun. 2005	26.5	Stick-up

Table H1: Standpipe and Vibrating Wire Piezometer Installation Details

Installation No.	Location	Installation Type	Date Completed	Screen or Tip Depth (m) ¹	Type of Casing Protector
VWP-12D	East end of Panagrot Ave	VWP	22 Jun. 2005	54.9	Stick-up
VWP-13A	Lewis Drive	VWP	08 Jul. 2005	5.1	Stick-up
VWP-13B	Lewis Drive	VWP	08 Jul. 2005	14.4	Stick-up
VWP-14A	Findlay Road	VWP	06 Jun. 2006	20.1	Stick-up
VWP-14B	Findlay Road	VWP	06 Jun. 2006	65.8	Stick-up
VWP-14C	Findlay Road	VWP	06 Jun. 2006	98.8	Stick-up
VWP-15A	West end of Panagrot Ave	VWP	27 Jun. 2006	8.8	Stick-up
VWP-15B	West end of Panagrot Ave	VWP	27 Jun. 2006	63.4	Stick-up
VWP-15C	West end of Panagrot Ave	VWP	27 Jun. 2006	83.2	Stick-up
VWP-16A	North of Diatomite Mine	VWP	23 Oct. 2006	42.7	Stick-up
VWP-16B	North of Diatomite Mine	VWP	23 Oct. 2006	85.6	Stick-up
VWP-16C	North of Diatomite Mine	VWP	23 Oct. 2006	139.0	Stick-up
VWP-16D	North of Diatomite Mine	VWP	23 Oct. 2006	214.9	Stick-up

Notes:

1 Depths are measured from ground surface. Screen depth is for standpipes, tip depth is for vibrating wire piezometers.

Appendix H: Legend for Stick Logs Shown in Figures H1 – H19

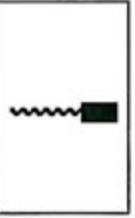
	F ^G	Glaciofluvial material. This unit may contain smaller discontinuous layers of silt, sand, gravel, till, and various near surface fill. Due to high variability of soils, the unit is modeled as isotropic.
	L ^G	Glaciolacustrine material. This unit generally contains layered silt, clay and fine sand.
	T _{AC}	T _{AC} —Australian Creek Tertiary sediments, comprised of sediments of volcanic origin.
	MT _{cc}	MT _{cc} – Mississippian to Triassic Cache Creek Sedimentary to Metamorphic bedrock. Highly fractured.
	Vibrating Wire Piezometer	Denotes the location of a vibrating wire piezometer in the stick log (tip and wire).
	Standpipe Piezometer	This symbol is used to denote the location of a standpipe piezometer in a stick log. The dashed section is the screened portion of the standpipe and is relative to the actual length of screen used.

Figure H1

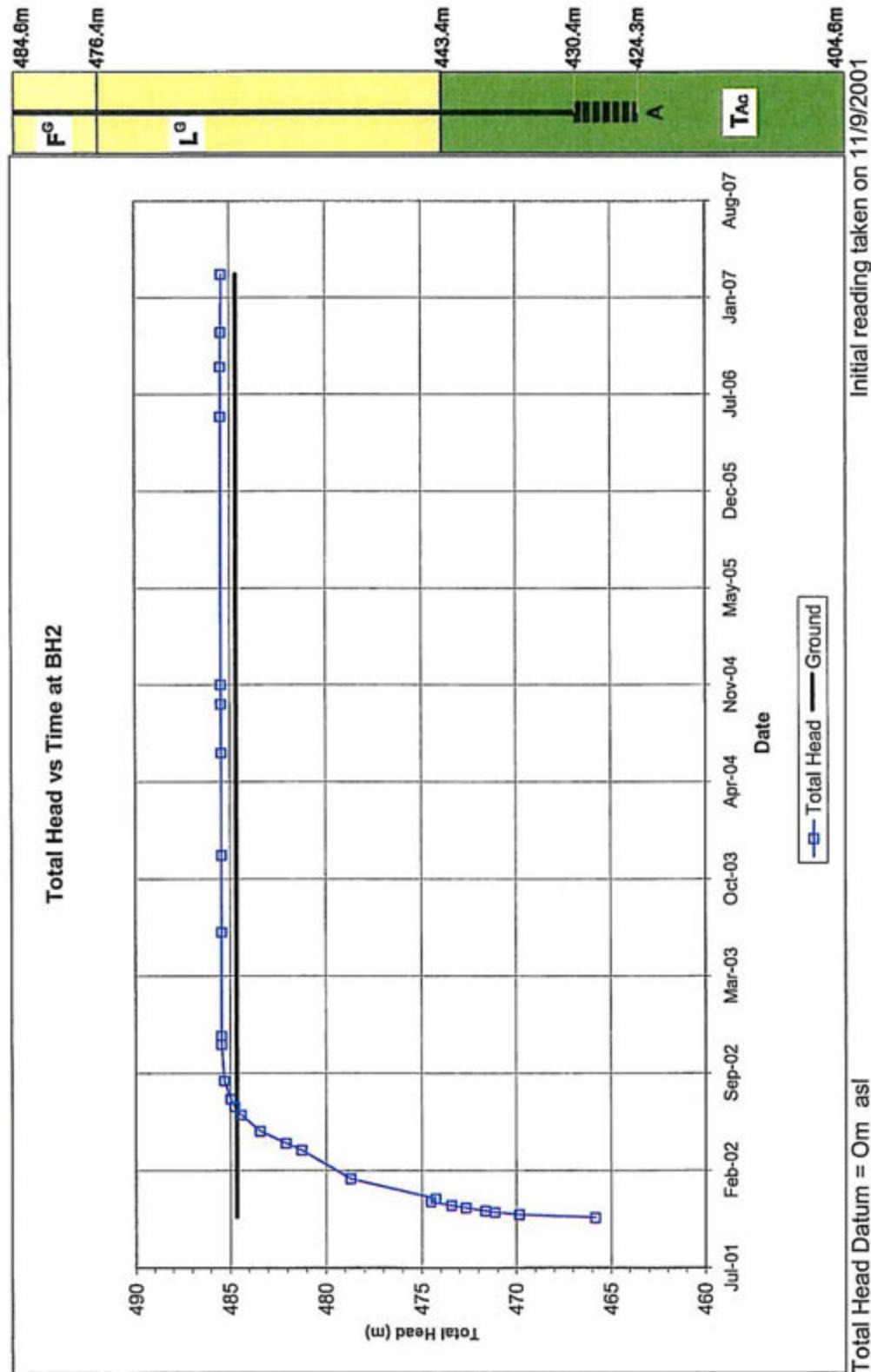


Figure H2

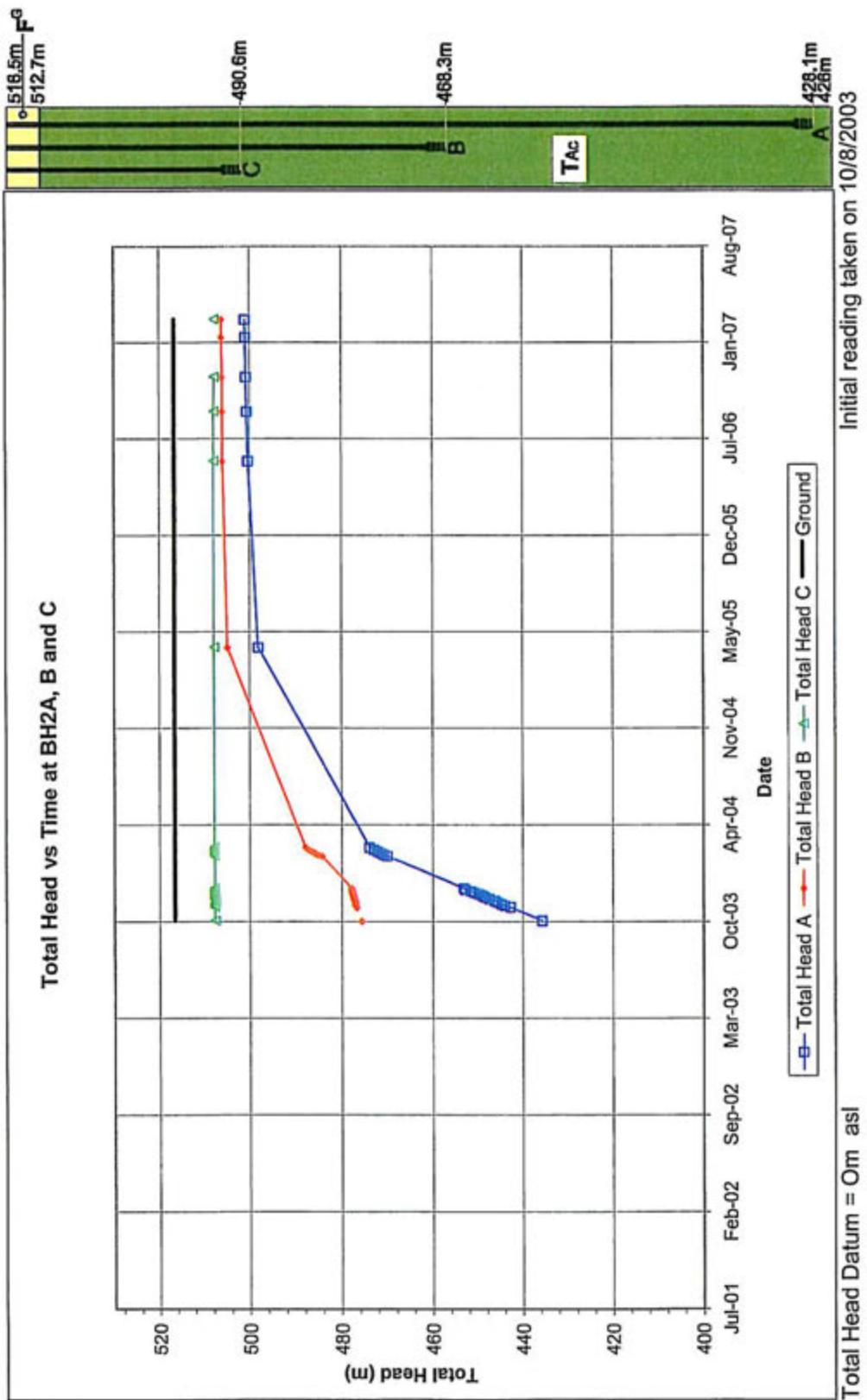


Figure H3

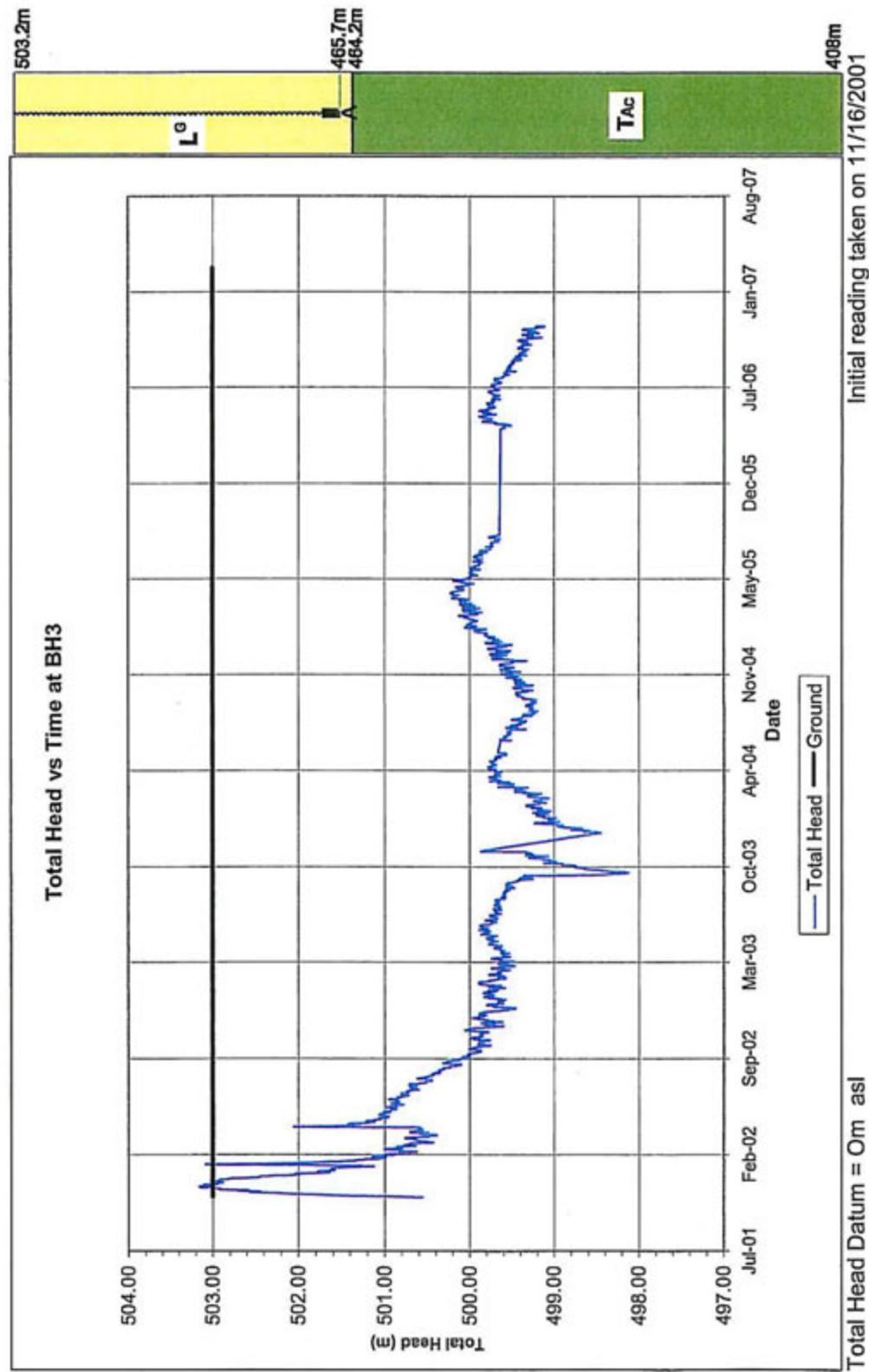


Figure H4

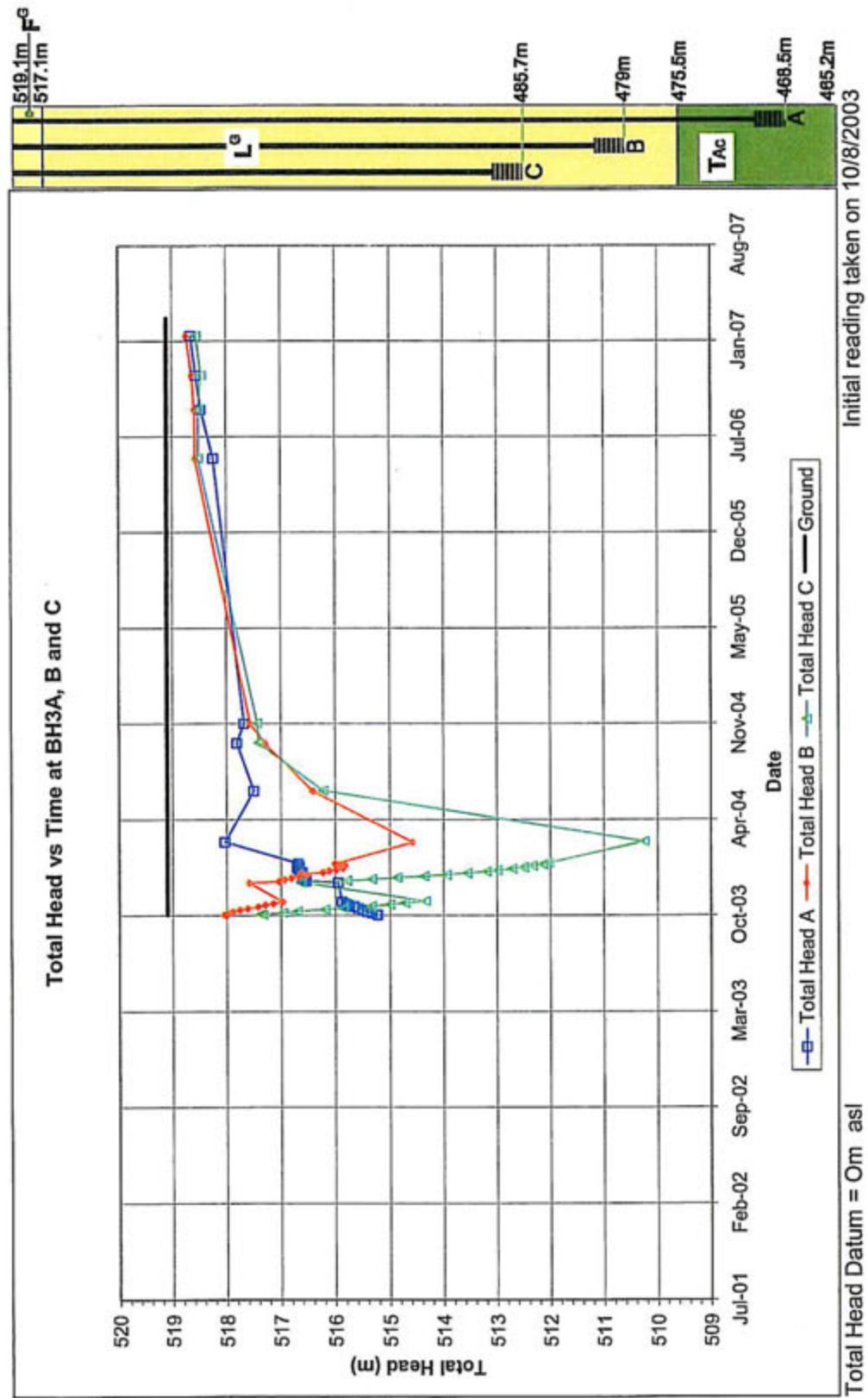


Figure H5

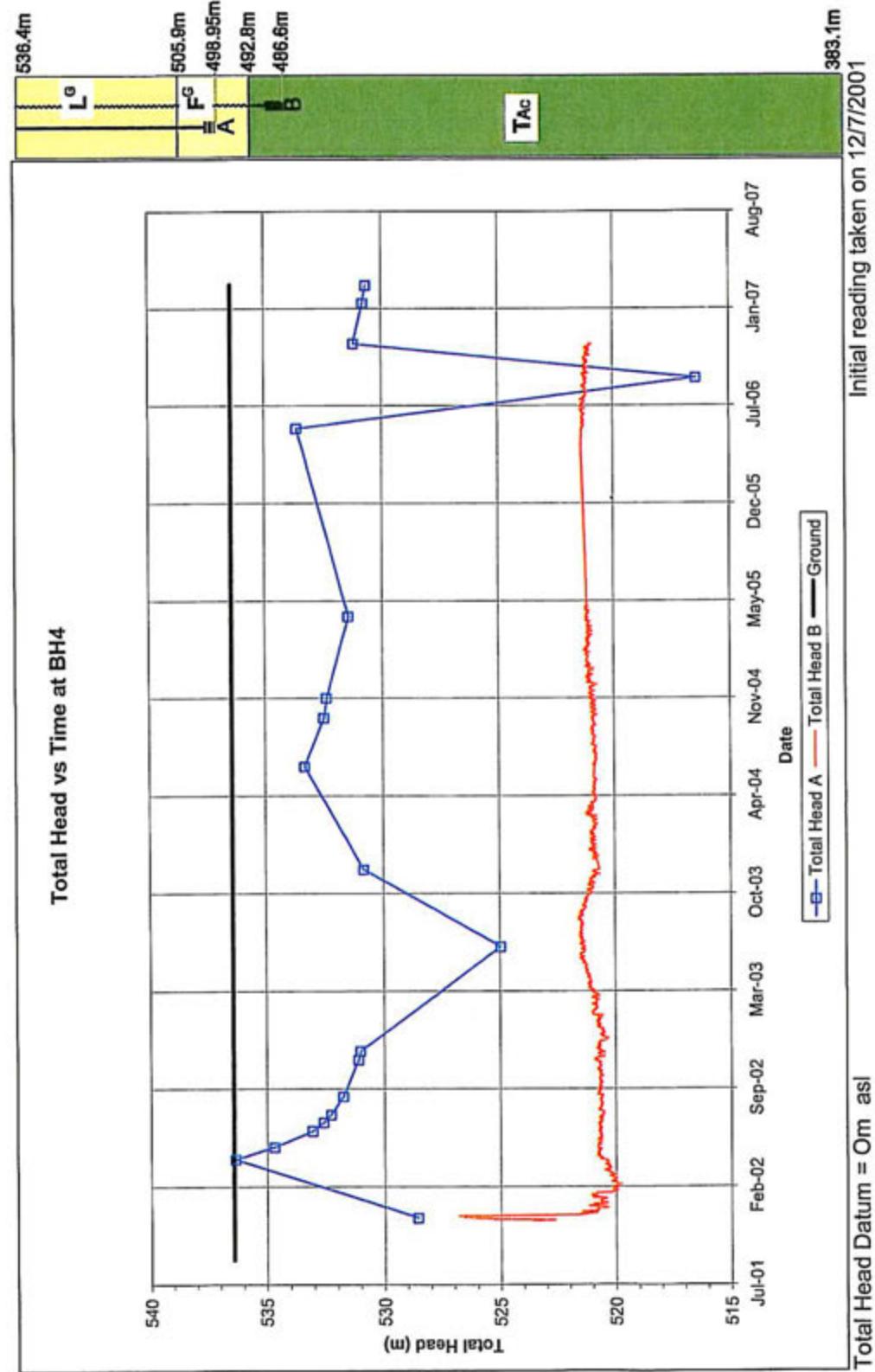


Figure H6

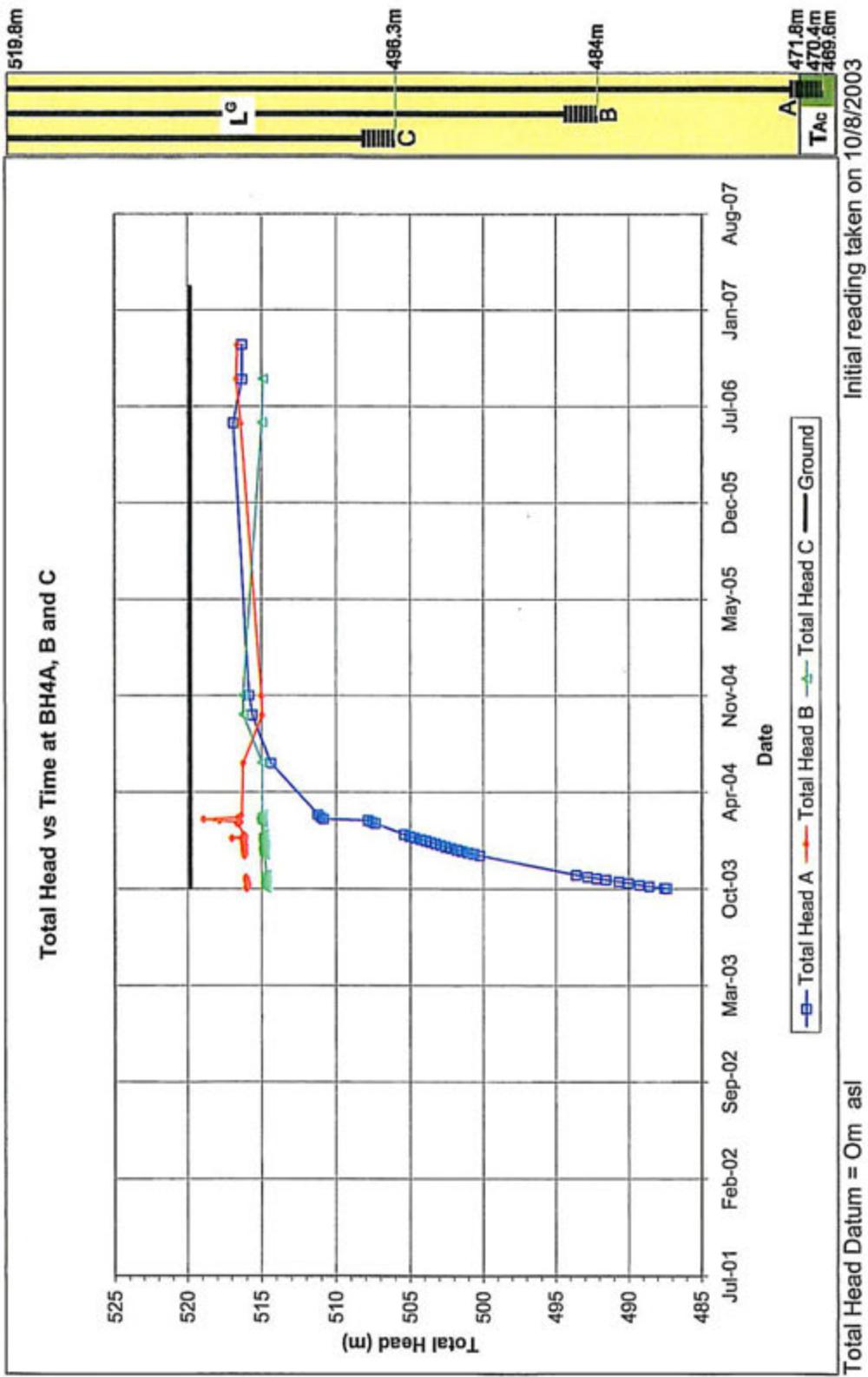


Figure H7

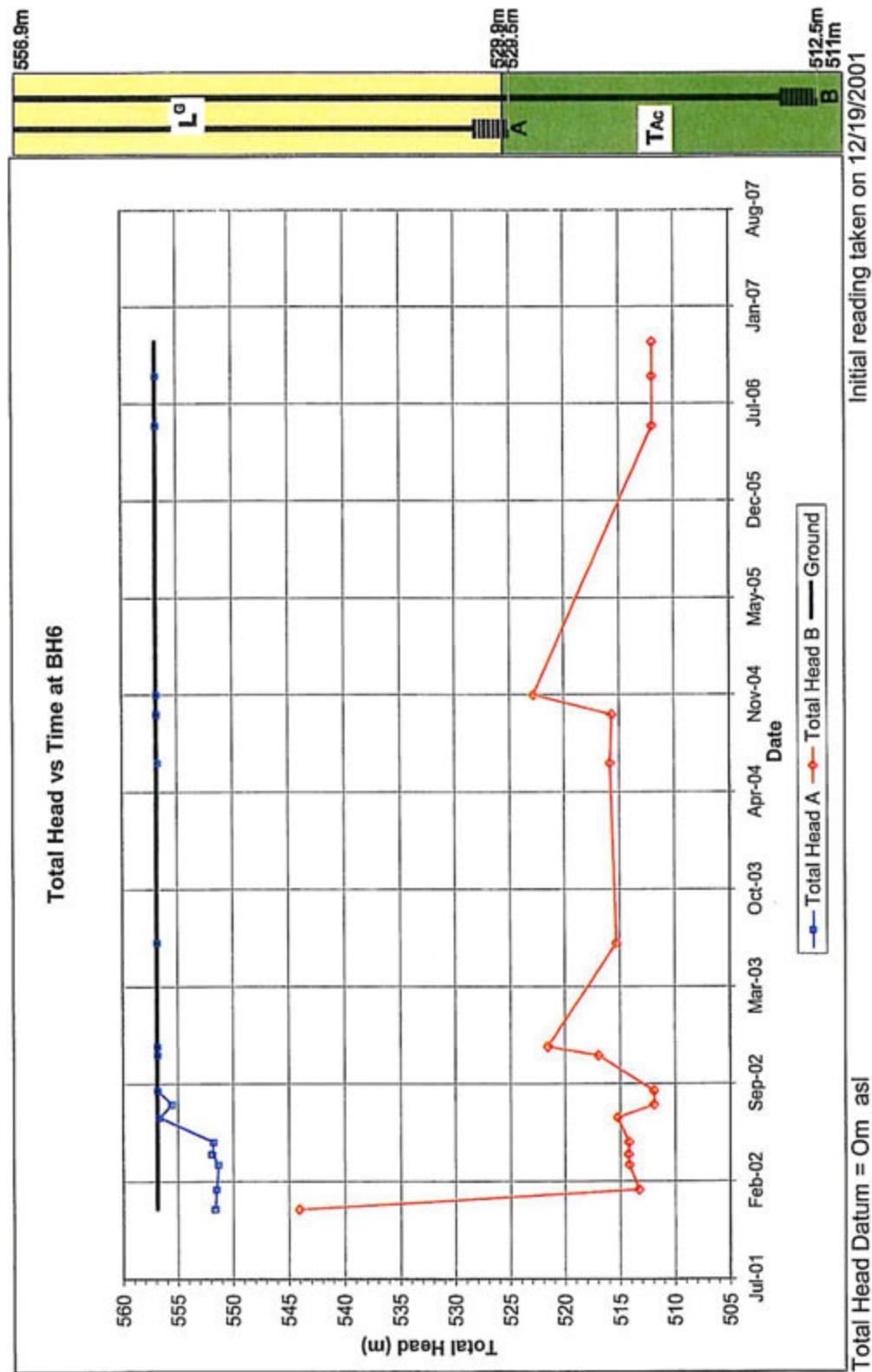


Figure H8

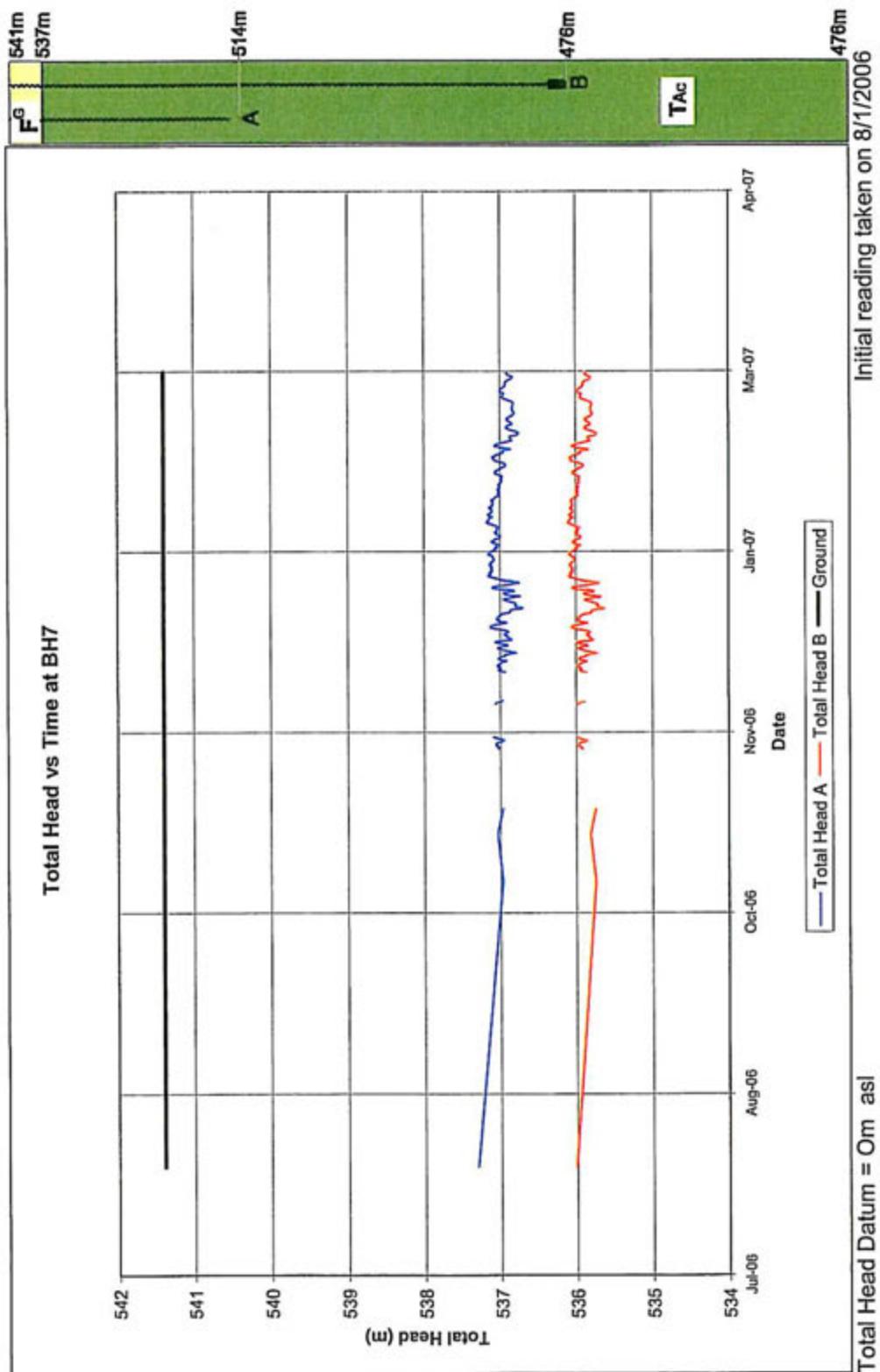


Figure H9

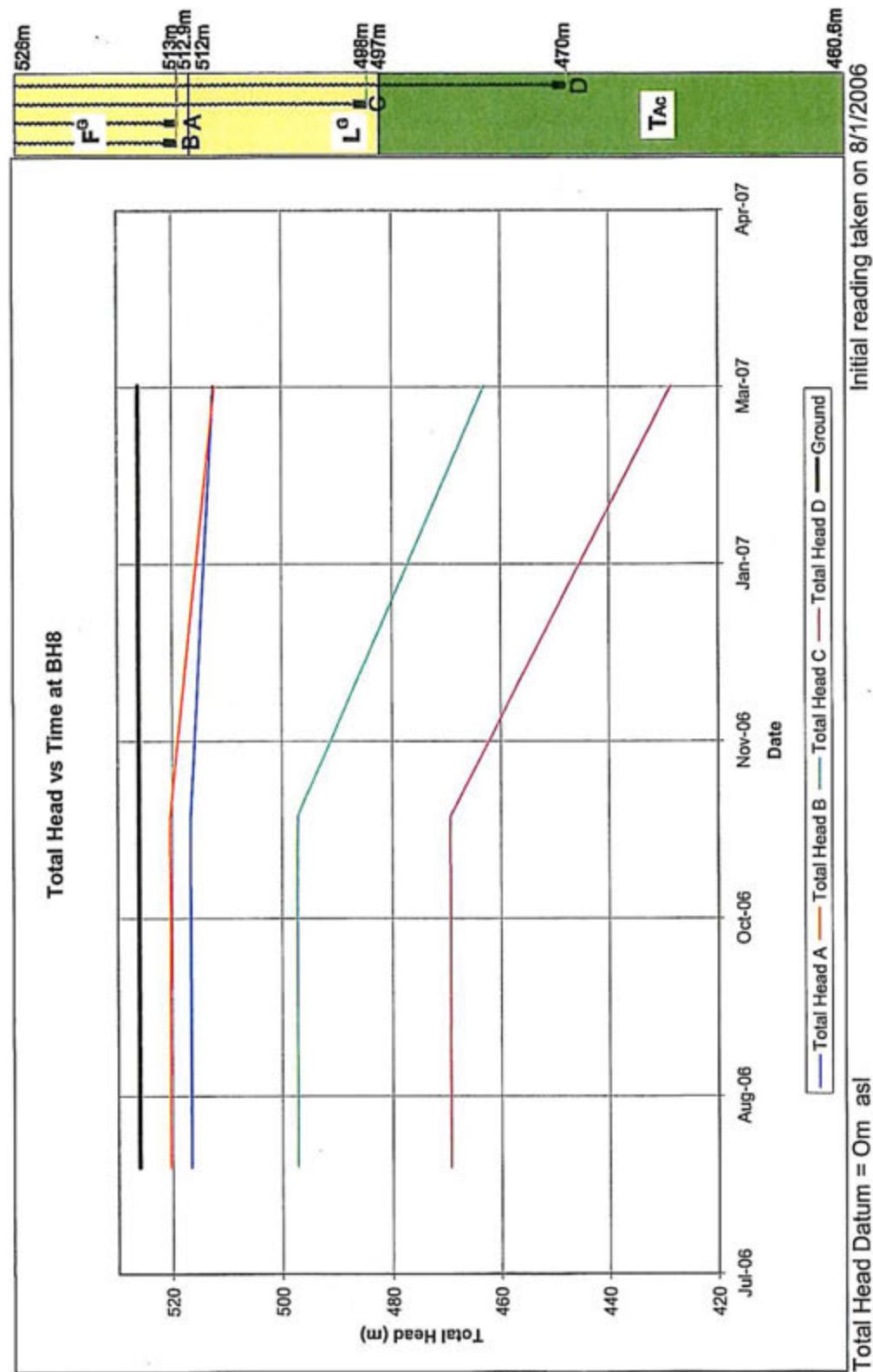


Figure 10

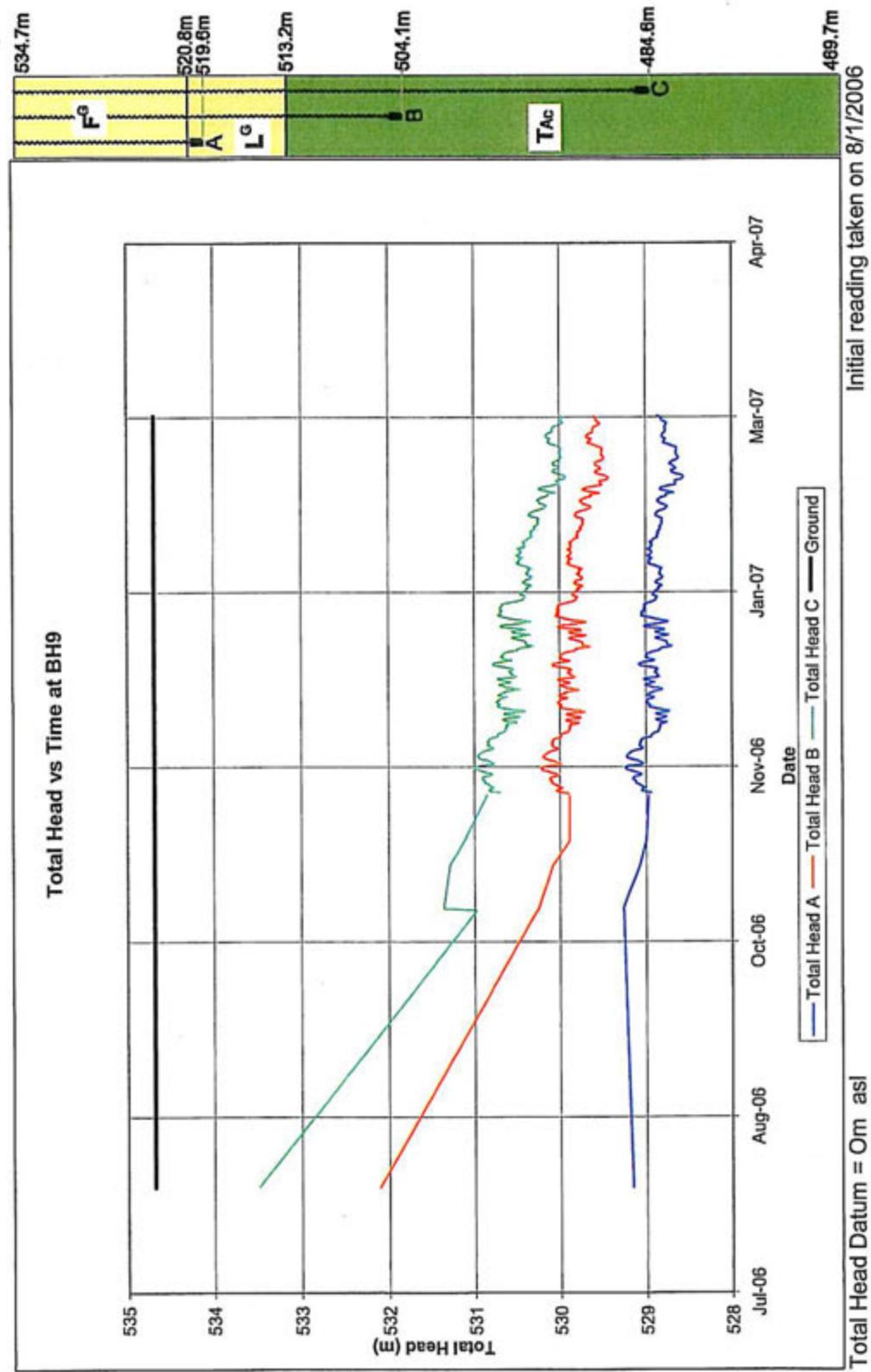


Figure H11

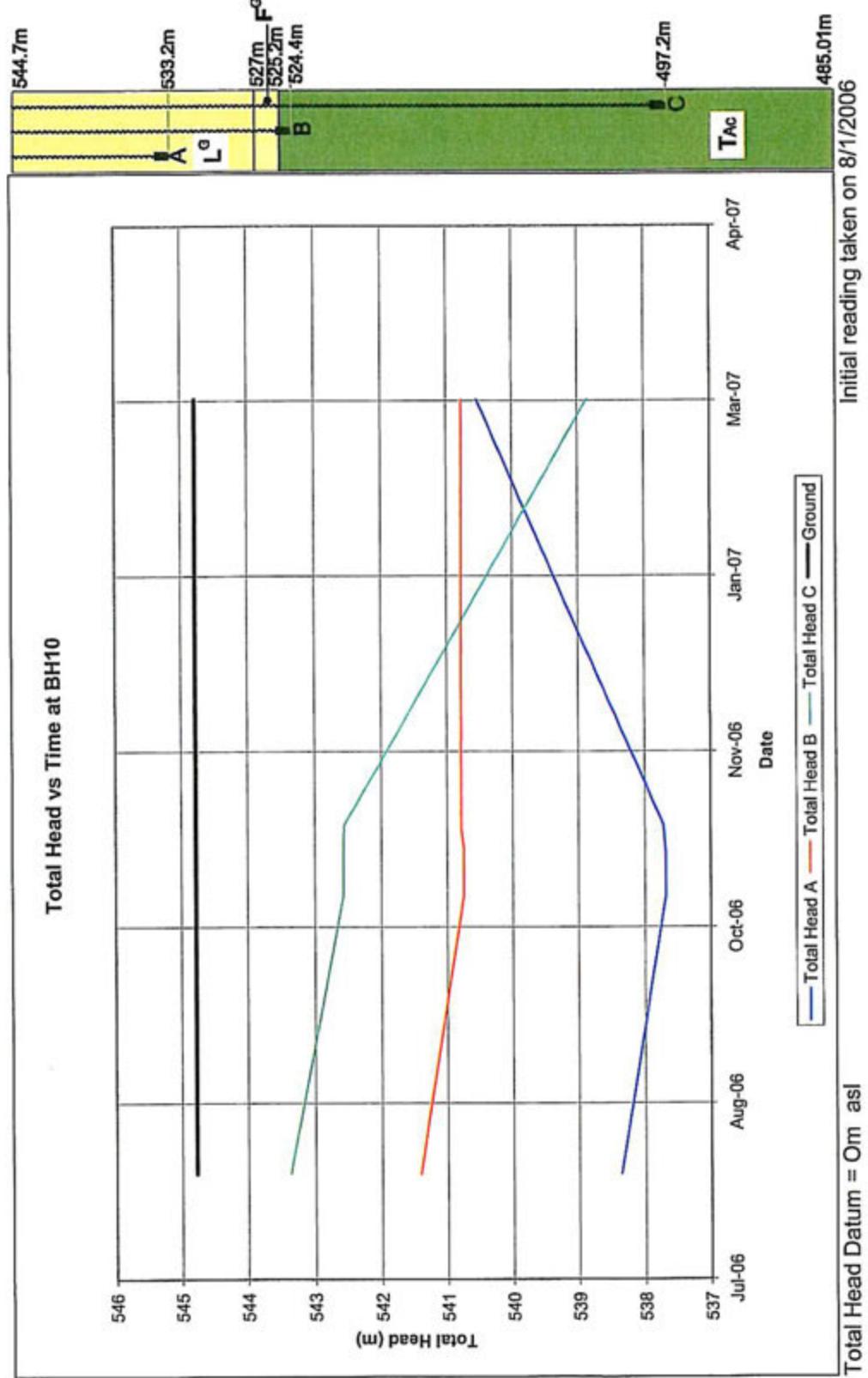


Figure H12

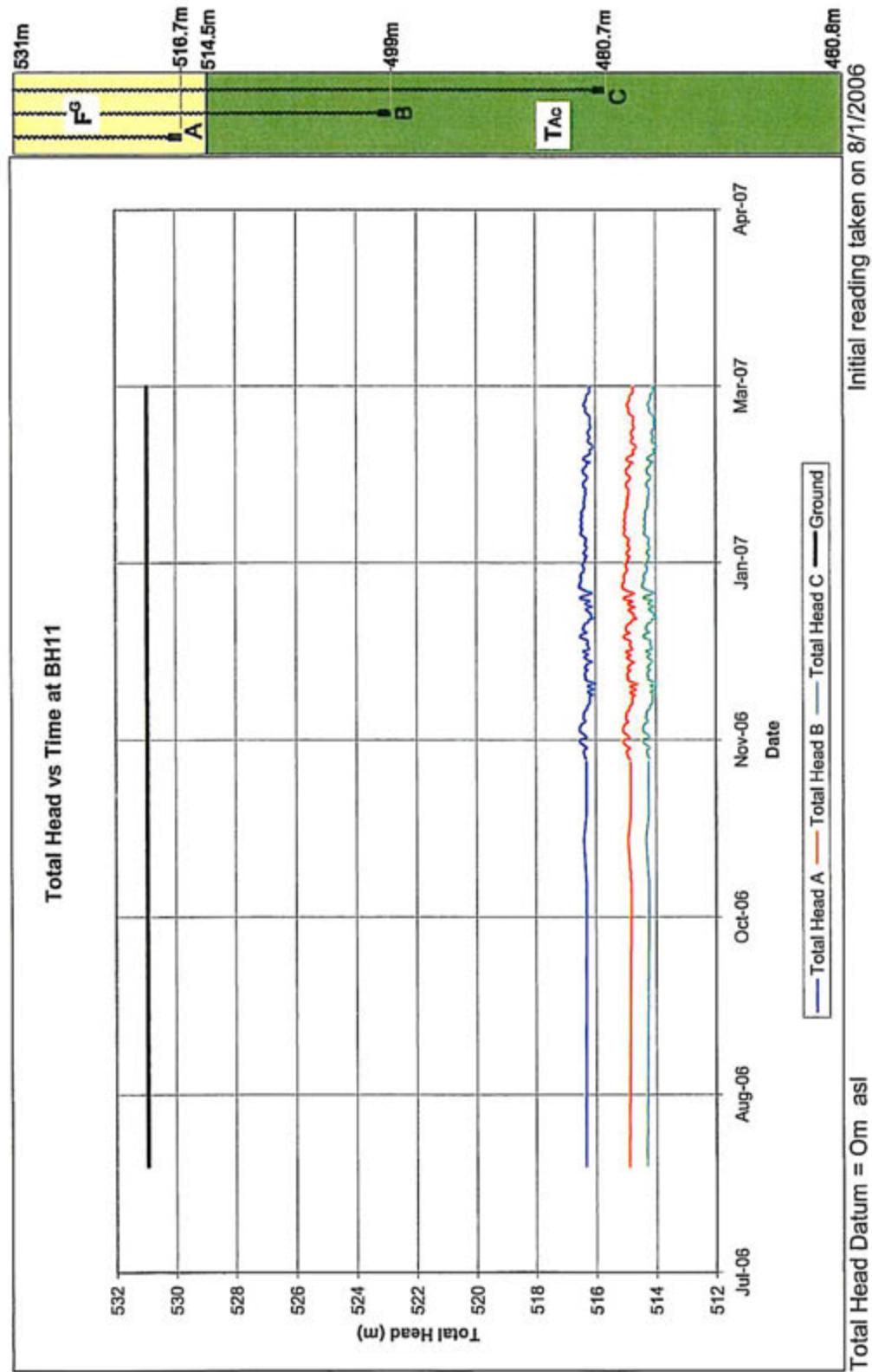


Figure H13

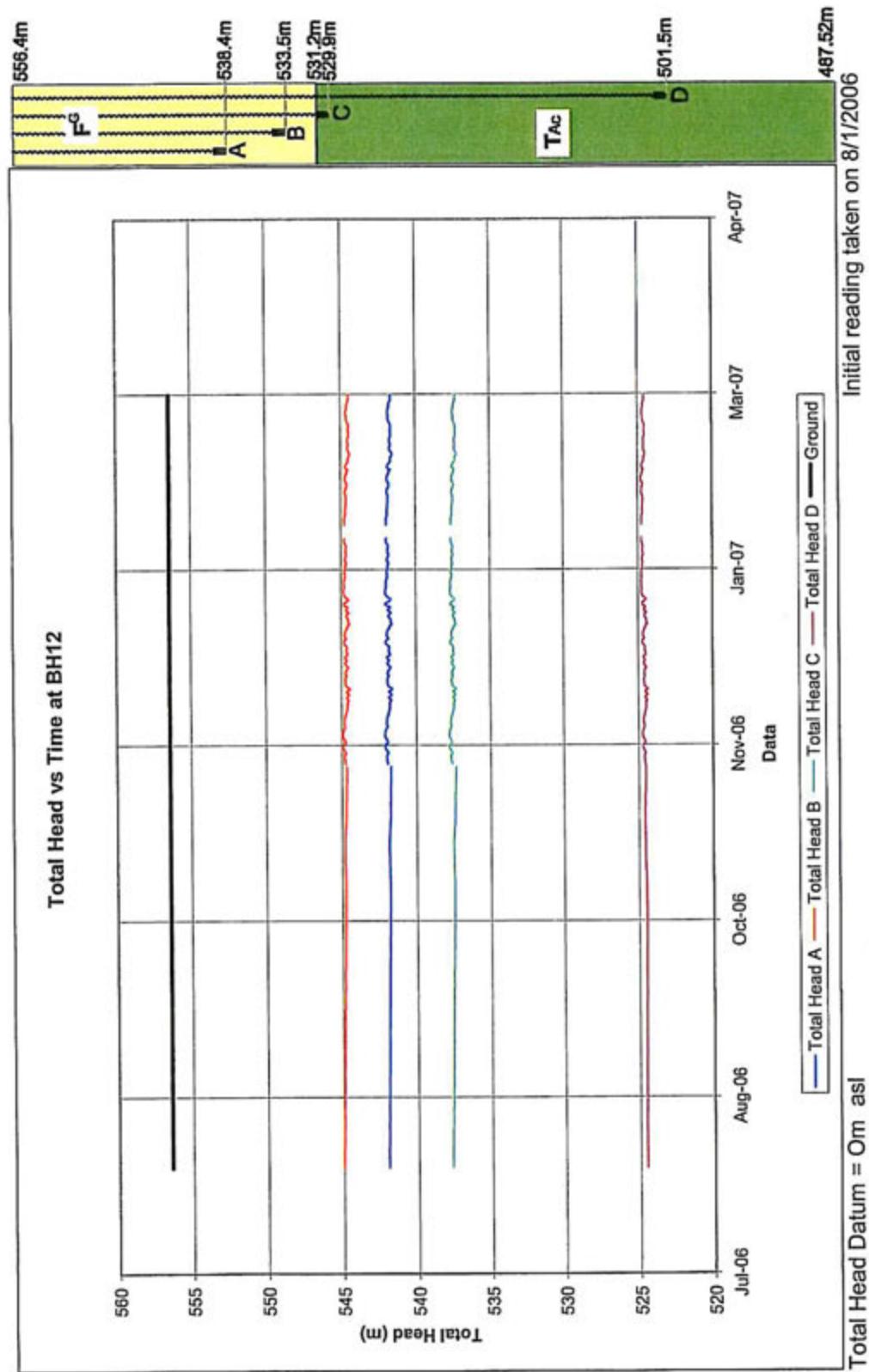


Figure H14

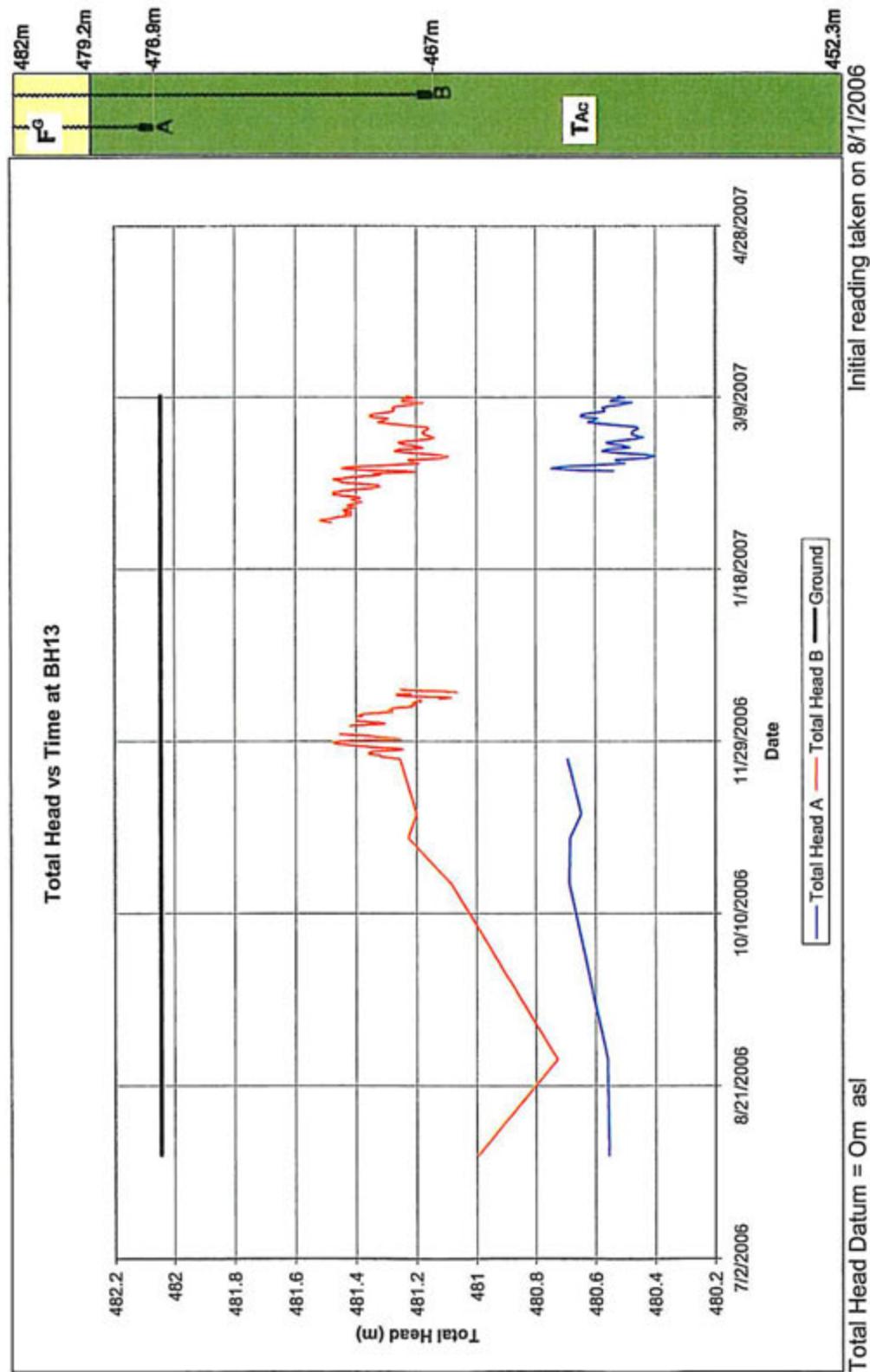


Figure H15

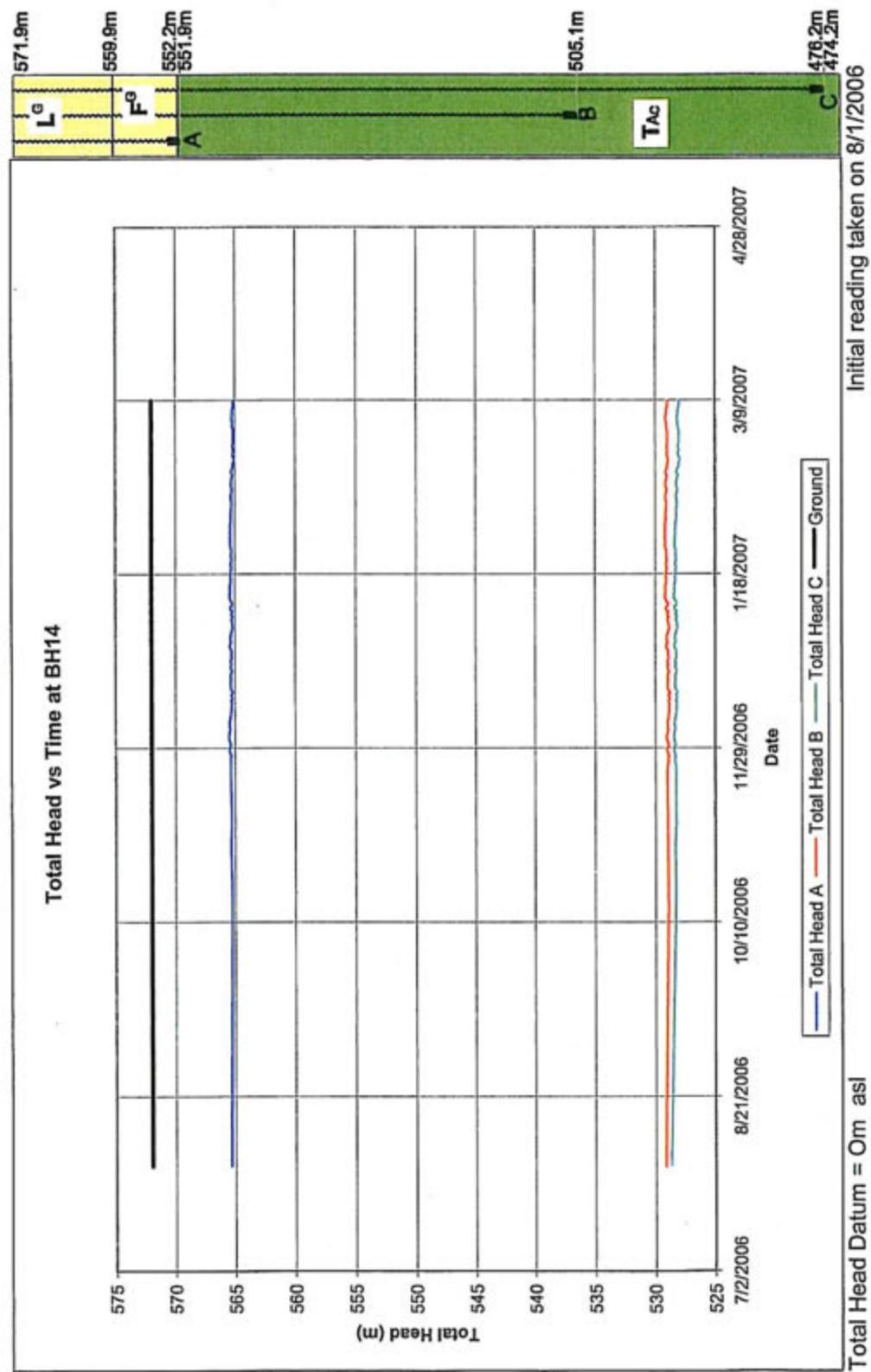


Figure H16

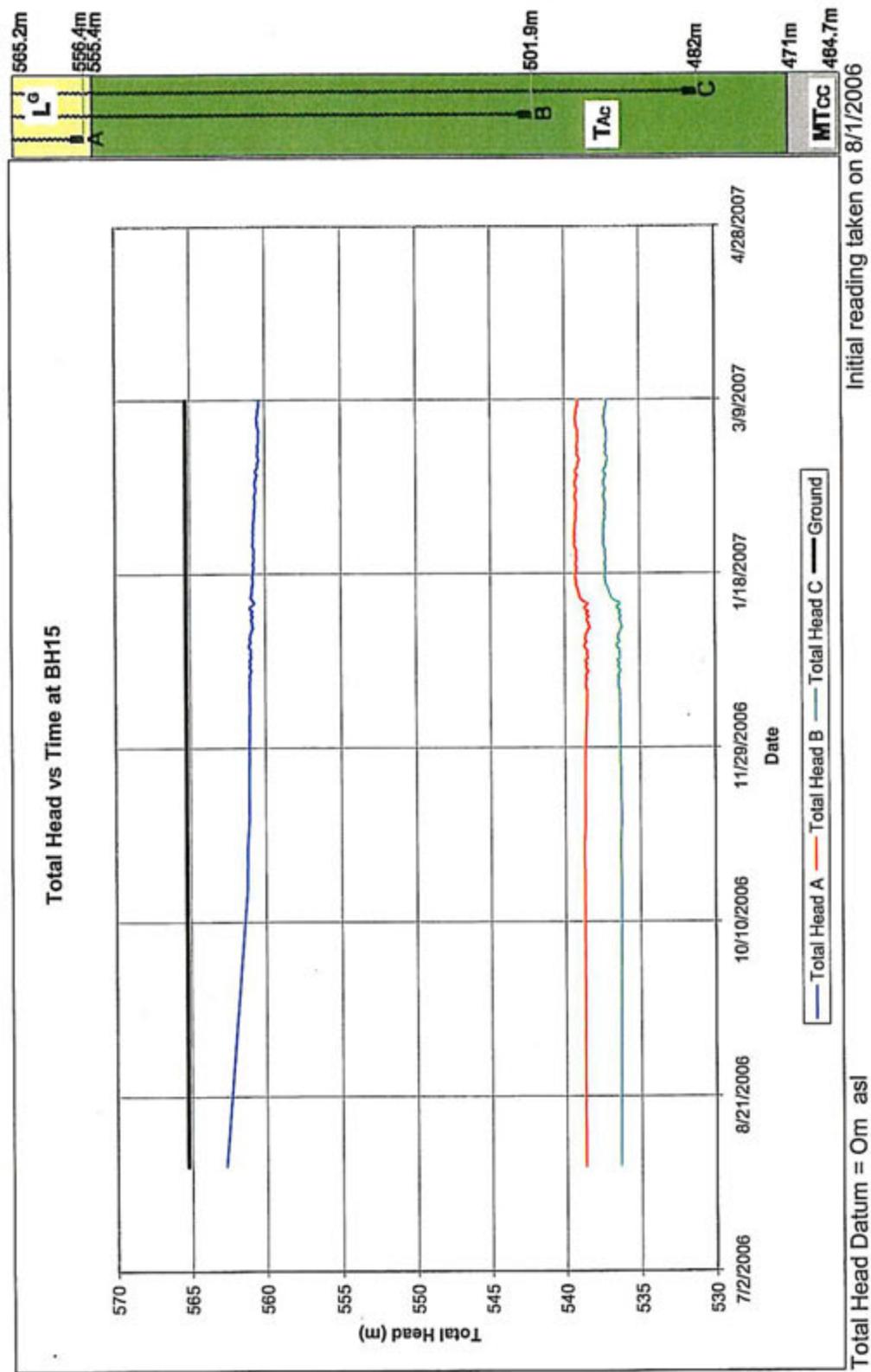


Figure H18

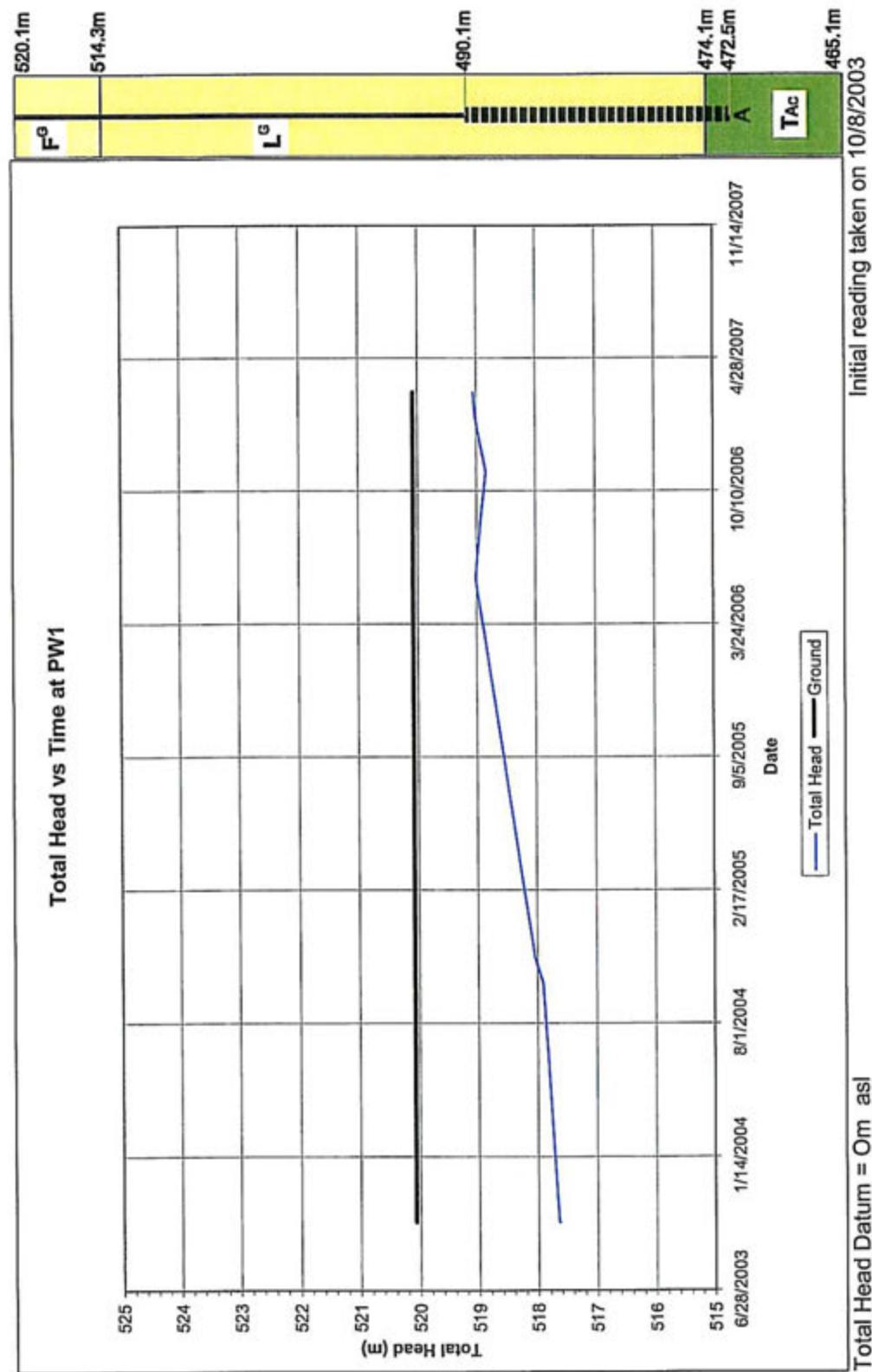


Figure H19

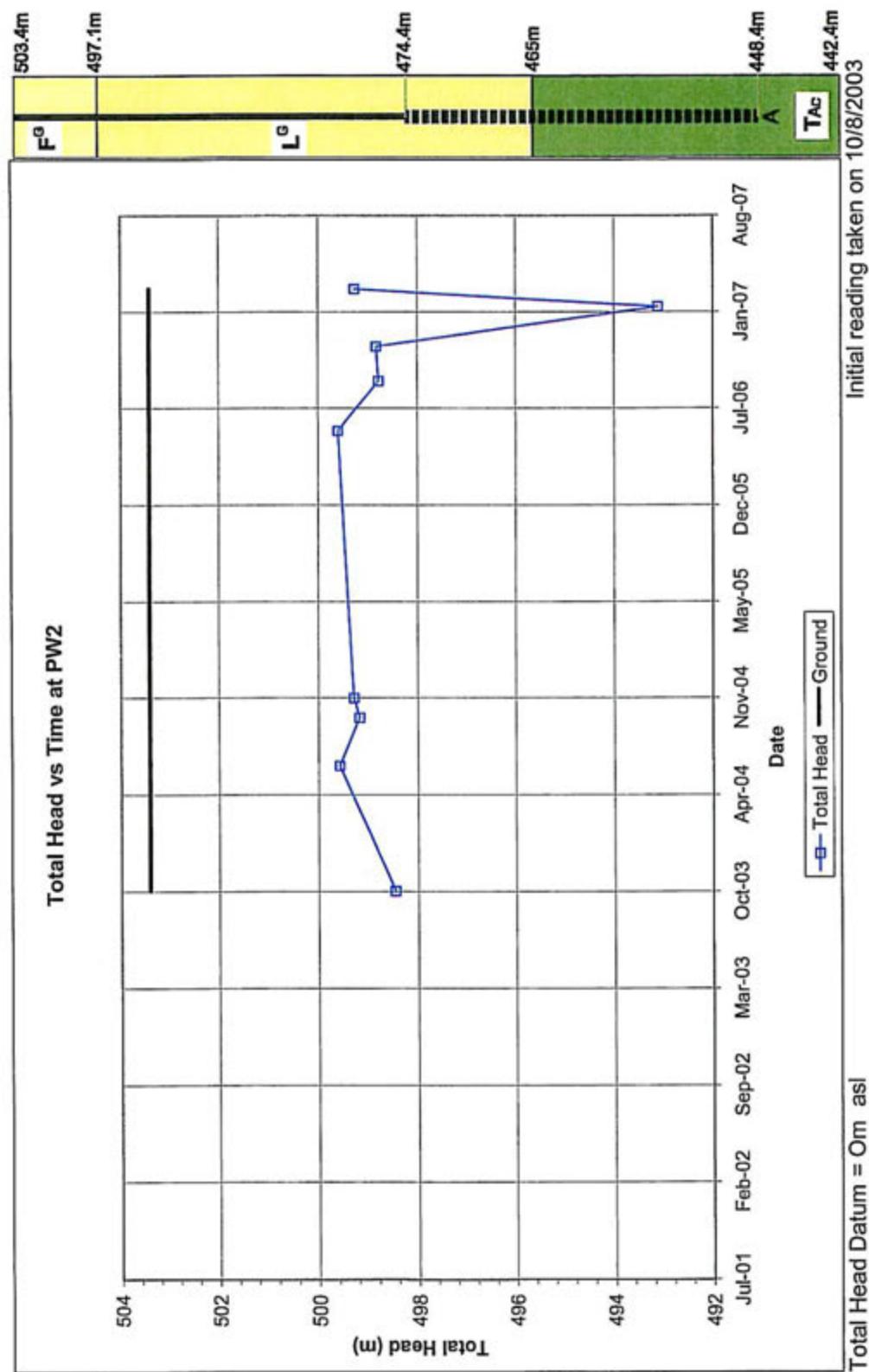


Figure H20

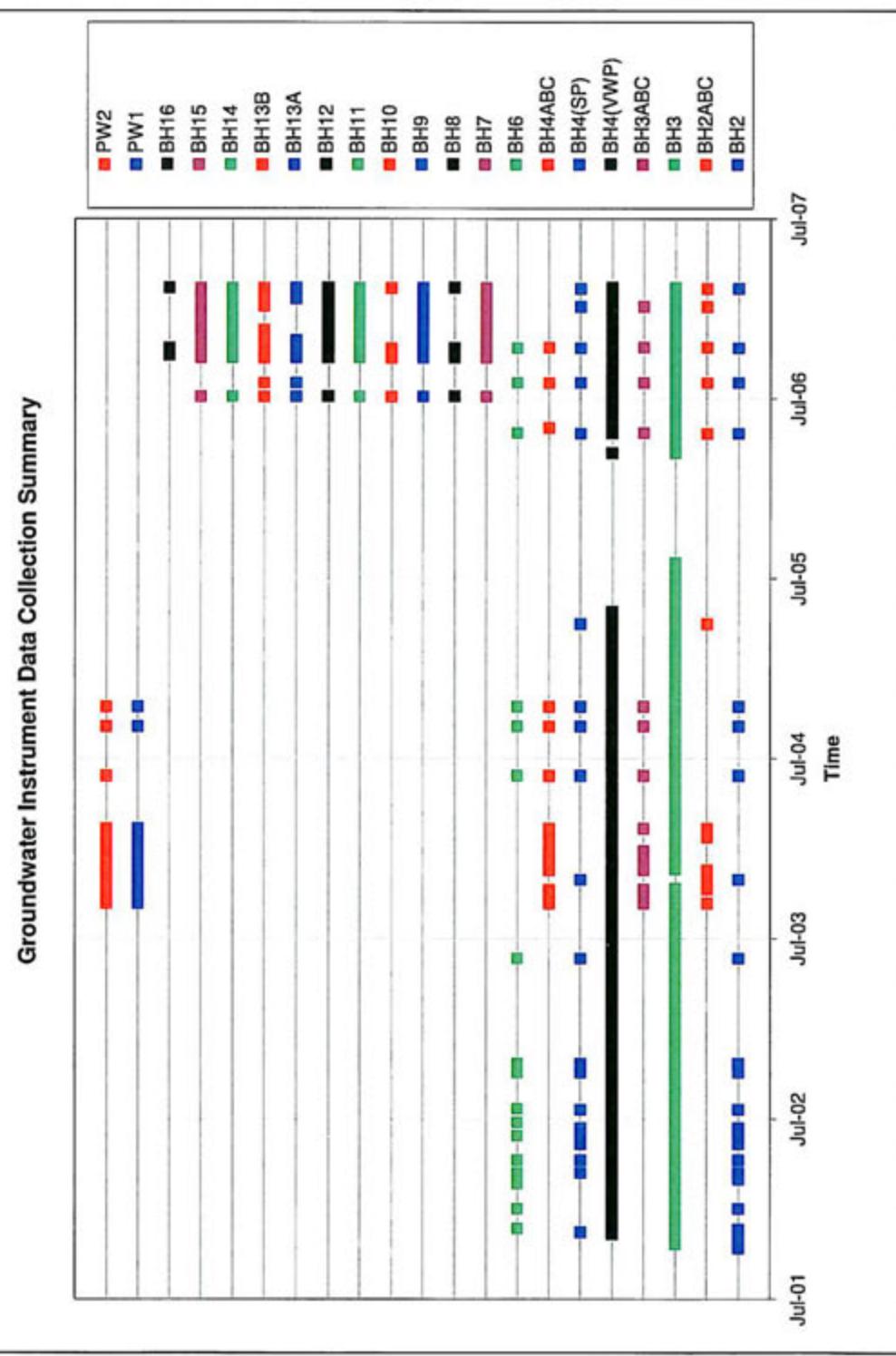




Photo 1: Drilling borehole for installation of vibrating wire piezometer 8A.



Photo 2: View of vibrating wire piezometer prior to installation in borehole.

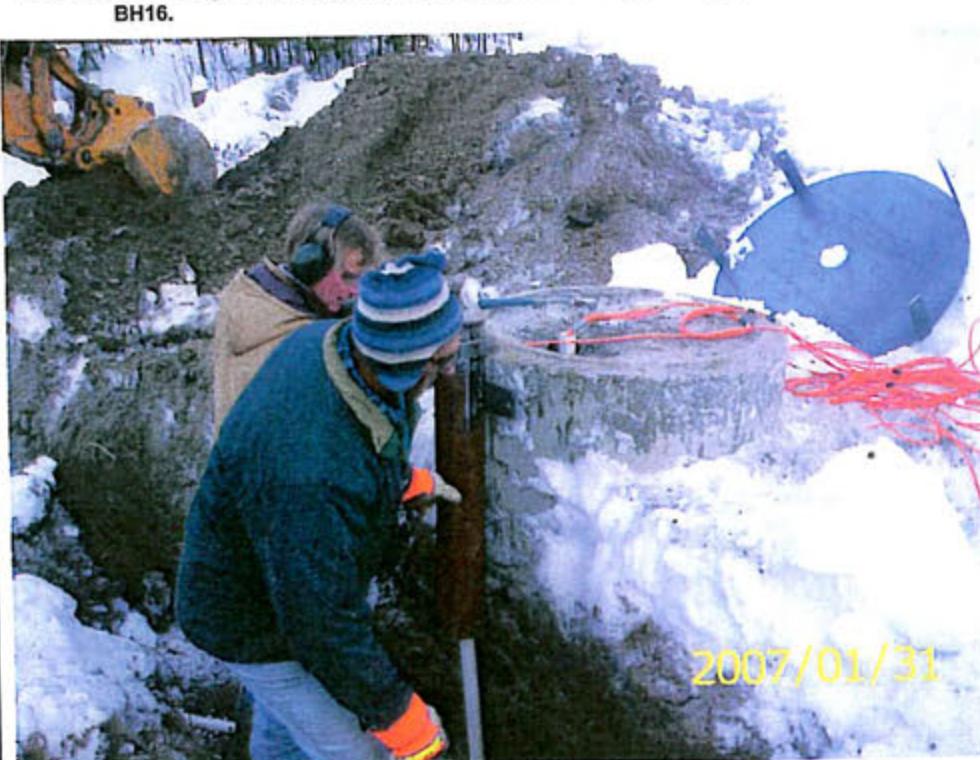


Photo 4: Re-routing of vibrating wire piezometer cable during datalogger installation at BH16.



Photo 3: Trenching between vibrating wire piezometer locations prior to installation of datalogger system at BH8.

Photo 5: Interior view of datalogger system showing battery, datalogger and vibrating wire interface.

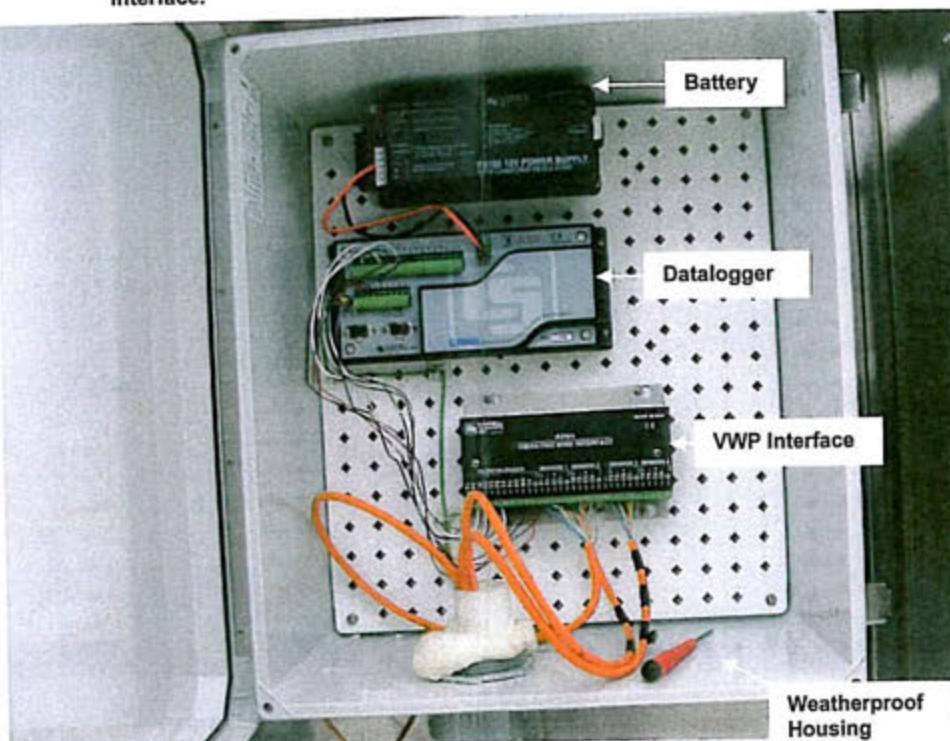


Photo 6: Completed installation of vibrating wire piezometer and datalogger system at BH16.



Photos 1 through 6

Taken: June 2005 to January 2007

X-RAY POWDER DIFFRACTION ANALYSIS OF FIVE SAMPLES.

(Project KX04397-15)

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April 3, 2007

EXPERIMENTAL METHODS

The five samples were reduced to the optimum grain-size range ($<5\text{ }\mu\text{m}$) by grinding under ethanol in a vibratory McCrone Micronising Mill for 7 minutes. Fine grain-size is an important factor in reducing micro-absorption contrast between phases. Step-scan X-ray powder-diffraction data were collected over a range $3\text{-}80^\circ 2\theta$ with $\text{CoK}\alpha$ radiation on a standard Siemens (Bruker) D5000 Bragg-Brentano diffractometer equipped with an Fe monochromator foil, 0.6 mm (0.3°) divergence slit, incident- and diffracted-beam Soller slits and a Vantec-1 strip detector. The long fine-focus Co X-ray tube was operated at 35 kV and 40 mA, using a take-off angle of 6° .

RESULTS

The X-ray diffractograms were analyzed using the International Centre for Diffraction Database PDF-4 using Search-Match software by Siemens (Bruker). Although quantitative phase analysis using the Rietveld method was requested, the abundance of clay minerals with severe stacking disorder, mainly smectite-group (probably montmorillonite-saponite) precludes this. The results of qualitative phase analysis are given in Table 1. Although the abundance of each phase is approximately proportional to peak heights, caution should be used in using this property. The X-ray diffractograms are shown in Figures 1-5.

Table 1. Results of qualitative phase analysis

Mineral	Ideal Formula	BH06-14 at 32.75m	BH06-15 at 57.2m	BH06-15 at 59.7m	BH06-16 at 74.5m	BH06-14 at 87.5m
Quartz	SiO_2	X	X	X	X	X
Kaolinite	$\text{Al}_2\text{Si}_2\text{O}_5(\text{OH})_4$	X	X	X	X	X
Phlogopite/biotite	$\text{K}(\text{Mg},\text{Fe})_3\text{AlSi}_3\text{O}_{10}(\text{OH})_2$	X	X	X	X	X
Smectite group (montmorillonite?)	$(\text{Na},\text{Ca})_{0.3}(\text{Al,Mg})_2\text{Si}_4\text{O}_{10}(\text{OH})_2 \cdot n\text{H}_2\text{O}$	X	X	X	X	X
Lizardite?	$\text{Mg}_3\text{Si}_2\text{O}_5(\text{OH})_4$	X	X	X	X	X
Plagioclase	$\text{NaAlSi}_3\text{O}_8 - \text{CaAl}_2\text{Si}_2\text{O}_8$	X	X	X	X	X
K-feldspar	KAlSi_3O_8	X	X	X	X	X
Alunite ?	$\text{K}_2\text{Al}_6(\text{SO}_4)_4(\text{OH})_{12}$	X	X	X	X	

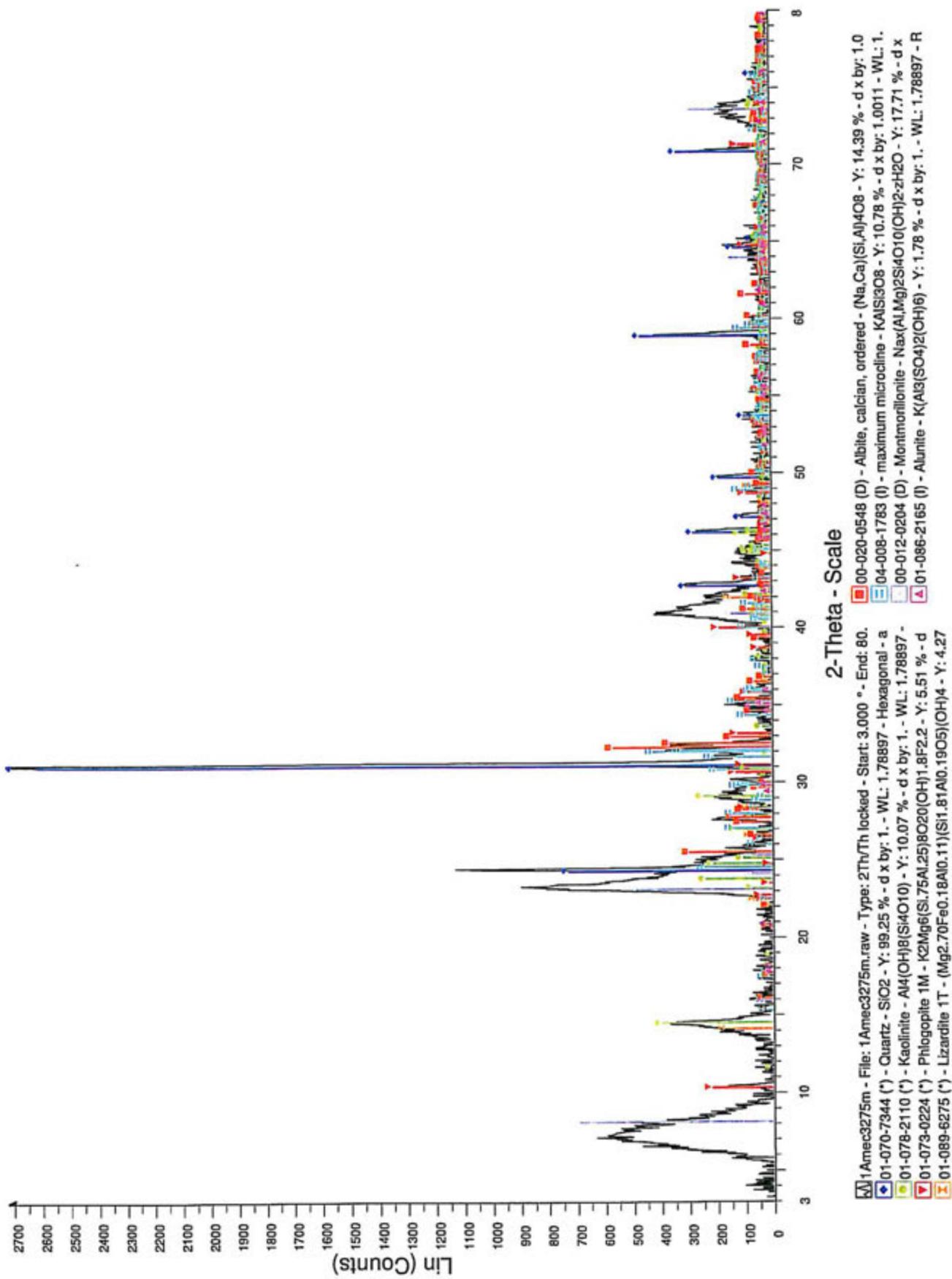


Figure 1. X-ray diffractogram of sample Amec BH06-14 at 32.75m

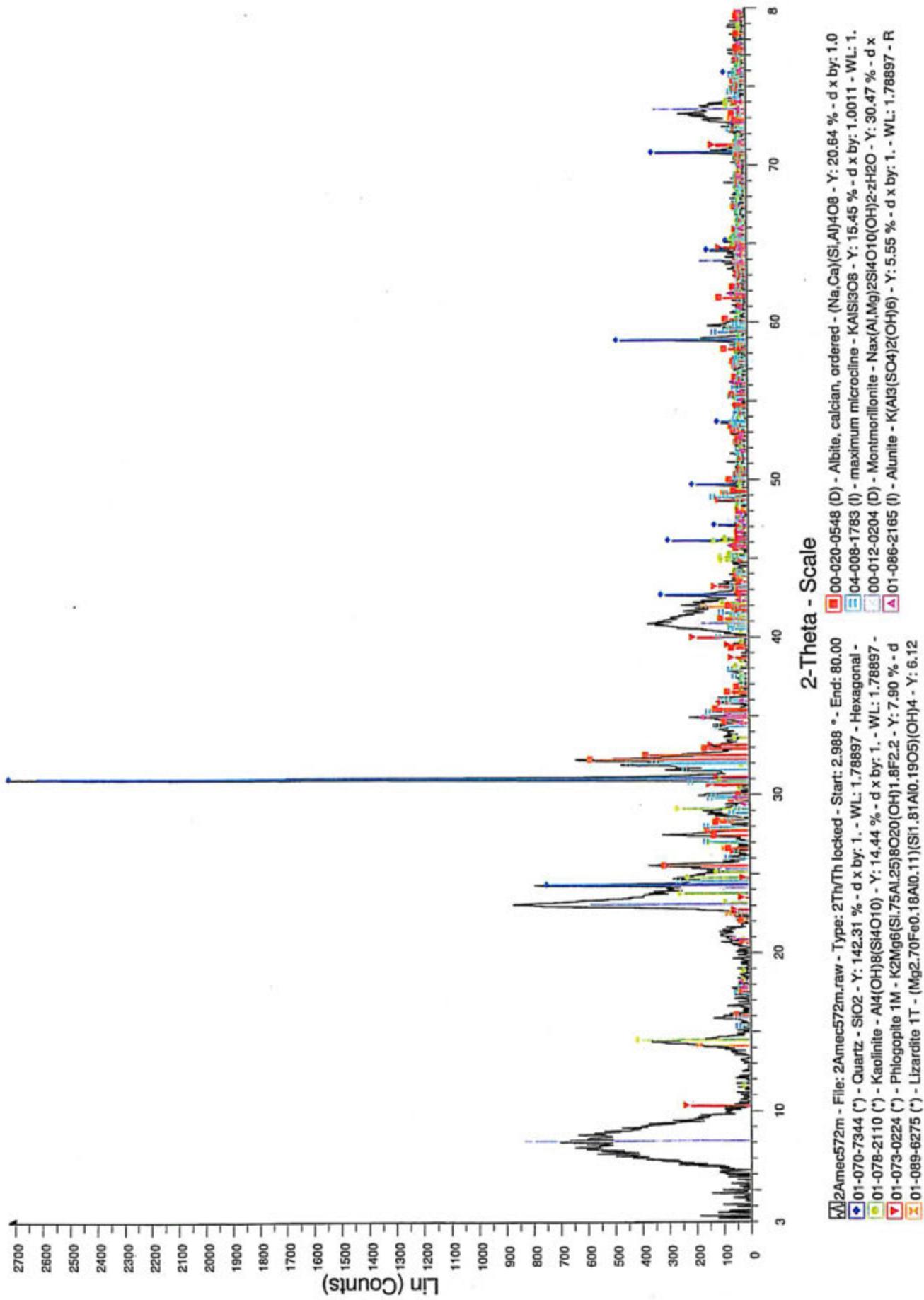


Figure 2. X-ray diffractogram of sample Amec BH06-15 at 57.2m

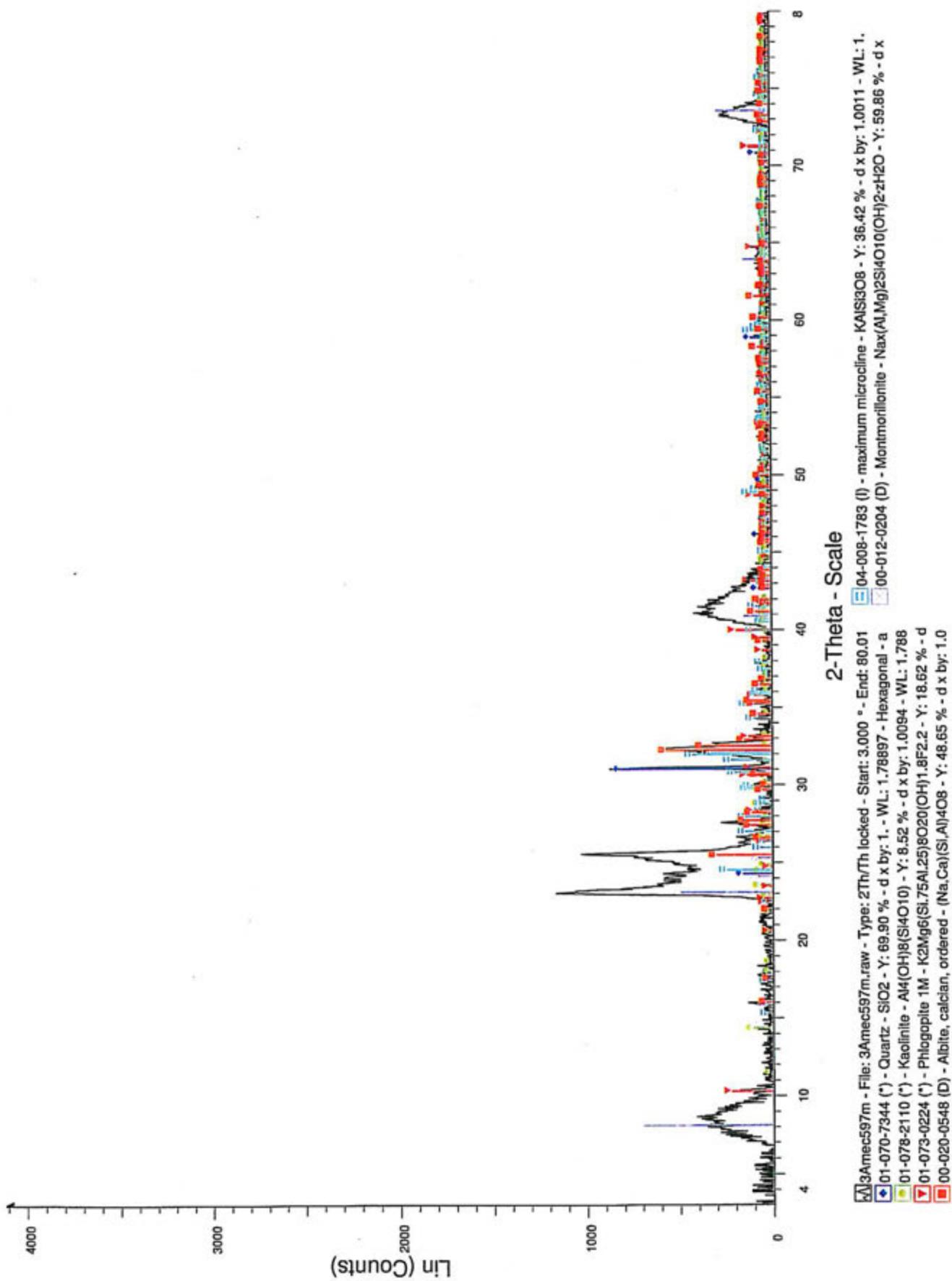


Figure 3. X-ray diffractogram of sample Amec BH06-15 at 59.7m.

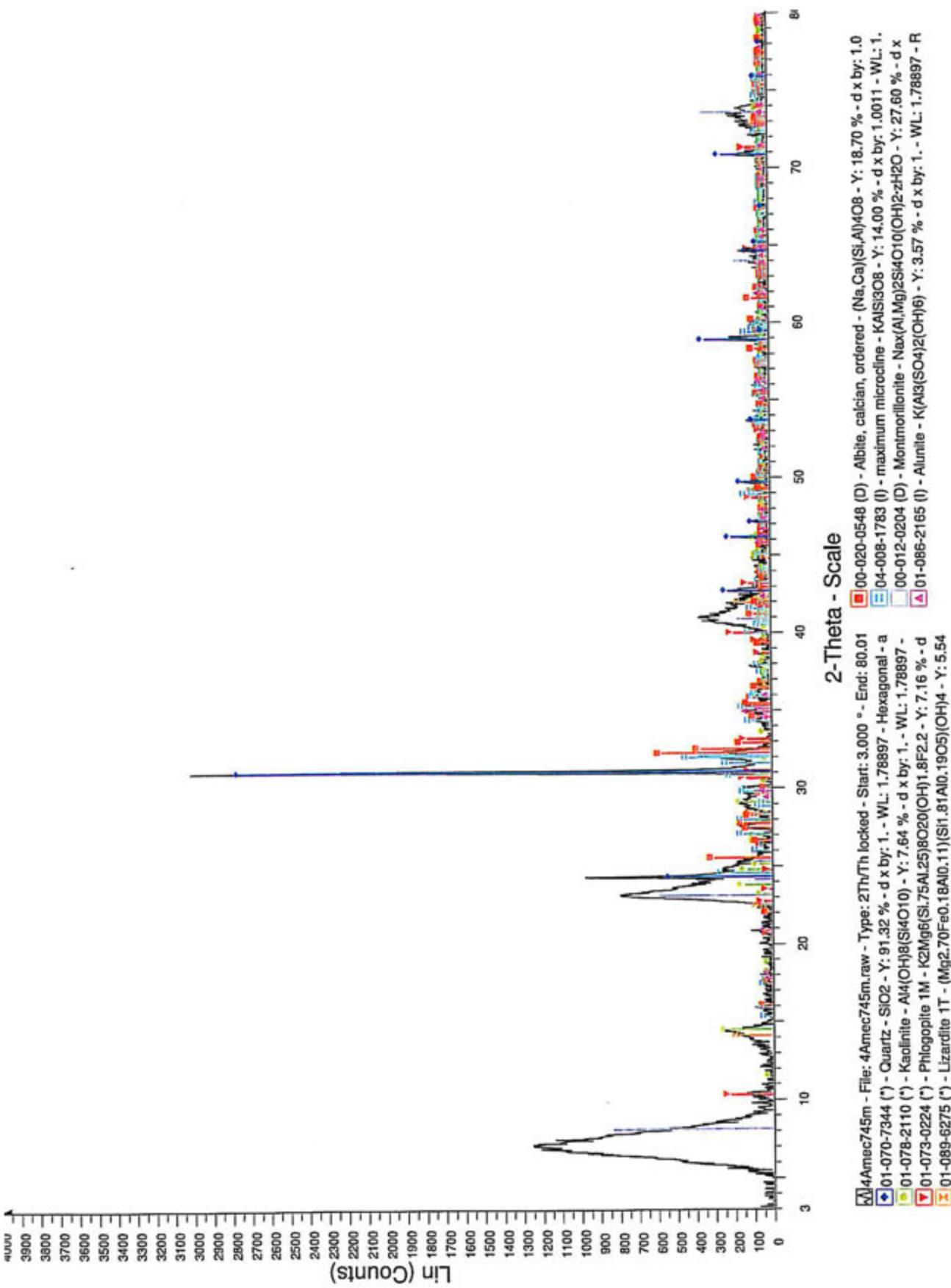


Figure 4. X-ray diffractogram of sample Amec BH06-16 at 74.5m

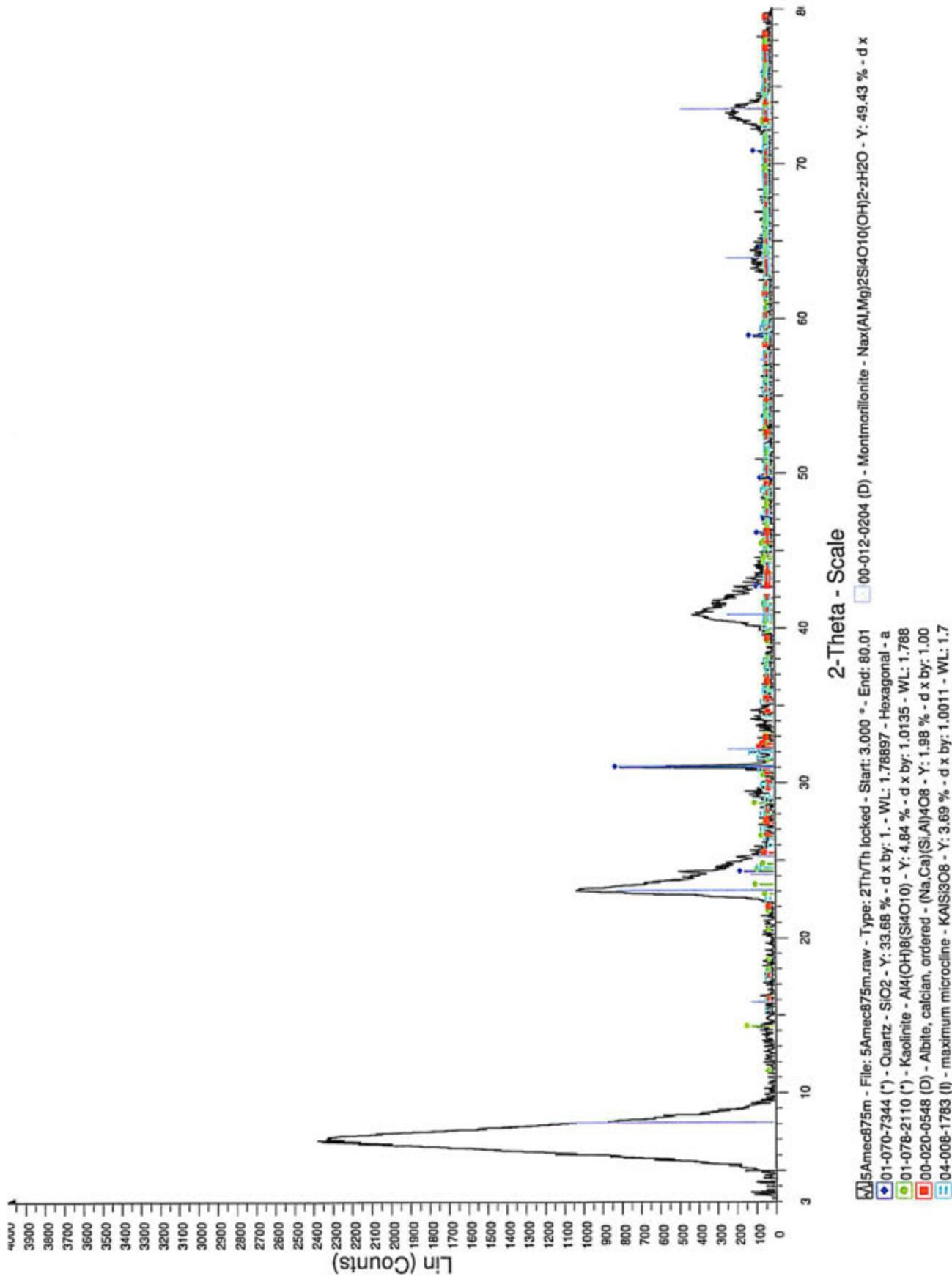


Figure 5. X-ray diffractogram of sample Amec BH06-14 at 87.5m